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Technical Specifications for Buildings

Local Government Engineering Department (LGED)
Japan International Cooperation Agency (JICA)

January, 2005
TECHNICAL SPECIFICATIONS FOR BUILDINGS

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TECHNICAL SPECIFICATIONS FOR BUILDINGS

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<table>
<thead>
<tr>
<th>AAMA</th>
<th>USA Architectural Aluminium Manufacturing Association</th>
</tr>
</thead>
<tbody>
<tr>
<td>AASHTO</td>
<td>American Association of State Highway and Transportation Officials</td>
</tr>
<tr>
<td>ACI</td>
<td>American Concrete Institute</td>
</tr>
<tr>
<td>ANSI</td>
<td>American National Standards Institute</td>
</tr>
<tr>
<td>ASME</td>
<td>American Society for Mechanical Engineers</td>
</tr>
<tr>
<td>ASTM</td>
<td>American Society for Testing and Materials</td>
</tr>
<tr>
<td>AWPA</td>
<td>American Wood Preservers Association</td>
</tr>
<tr>
<td>AWS</td>
<td>American Welding Society</td>
</tr>
<tr>
<td>BDS</td>
<td>Bangladesh Standards</td>
</tr>
<tr>
<td>BIT</td>
<td>Bangladesh Institute of Technology</td>
</tr>
<tr>
<td>BM</td>
<td>Bench Mark</td>
</tr>
<tr>
<td>BNBC</td>
<td>Bangladesh National Building Code</td>
</tr>
<tr>
<td>BOQ</td>
<td>Bill of Quantities</td>
</tr>
<tr>
<td>BS</td>
<td>British Standards</td>
</tr>
<tr>
<td>BUET</td>
<td>Bangladesh University of Engineering and Technology</td>
</tr>
<tr>
<td>BWG</td>
<td>British Wire Gauge</td>
</tr>
<tr>
<td>C.C</td>
<td>Cement Concrete</td>
</tr>
<tr>
<td>C.I</td>
<td>Cast Iron</td>
</tr>
<tr>
<td>CBR</td>
<td>California Bearing Ratio</td>
</tr>
<tr>
<td>cm</td>
<td>Centimeter</td>
</tr>
<tr>
<td>CPT</td>
<td>Cone Penetration Test</td>
</tr>
<tr>
<td>cum</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>FM</td>
<td>Fineness Modulus</td>
</tr>
<tr>
<td>G.I.</td>
<td>Galvanized Iron</td>
</tr>
<tr>
<td>gm</td>
<td>Gram</td>
</tr>
<tr>
<td>HBB</td>
<td>Herring Bone Bond</td>
</tr>
<tr>
<td>IS</td>
<td>Indian Standards</td>
</tr>
<tr>
<td>ISO</td>
<td>International Organization for Standardization</td>
</tr>
<tr>
<td>Kg</td>
<td>Kilogram</td>
</tr>
<tr>
<td>kN</td>
<td>Kilo Newton</td>
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<td>kPa</td>
<td>Kilo Pascal</td>
</tr>
<tr>
<td>LGED</td>
<td>Local Government Engineering Department</td>
</tr>
<tr>
<td>m</td>
<td>Meter</td>
</tr>
<tr>
<td>M.S.</td>
<td>Mild Steel</td>
</tr>
<tr>
<td>MDD</td>
<td>Maximum Dry Density</td>
</tr>
<tr>
<td>mm</td>
<td>Millimeter</td>
</tr>
<tr>
<td>MPa</td>
<td>Mega Pascal</td>
</tr>
<tr>
<td>N</td>
<td>Number of Blows per feet</td>
</tr>
<tr>
<td>PVA</td>
<td>Polyvinyl Acetate</td>
</tr>
<tr>
<td>PVC</td>
<td>Polyvinyl Chloride</td>
</tr>
<tr>
<td>R.C.C.</td>
<td>Reinforced Cement Concrete</td>
</tr>
<tr>
<td>SBR</td>
<td>Styrene Butadiene Rubber</td>
</tr>
<tr>
<td>STP</td>
<td>Standard Laboratory Testing Procedure</td>
</tr>
<tr>
<td>SWG</td>
<td>Standard Wire Gauge</td>
</tr>
<tr>
<td>USA</td>
<td>United States of America</td>
</tr>
<tr>
<td>WPS</td>
<td>Welding Procedure Specification</td>
</tr>
<tr>
<td>WQR</td>
<td>Welder Qualification Requirement</td>
</tr>
</tbody>
</table>
EXORDIUM

It is a matter of satisfaction that the technical specifications for ‘Building’ construction have been published aiming at construction of a document that represents a common and uniform viewpoint of the LGED in this context, the goal obviously being an enrichment of the LGED’s ethos at its various levels. This book is a re-production and compilation of various information and practices that have already been documented in the Standards followed by the United States, United Kingdom, India and the like while constructing ‘Buildings’ of different types and classes. Proper attention has also been paid to induce the procedures and practices that have been prescribed in the “Bangladesh National Building Code, 1993”. Besides, the documents on ‘Building’ construction, prepared and followed earlier by the different projects of the LGED, have also been reviewed, discussed and noted.

It has by this time, been widely acknowledged both in home and abroad the performance of the LGED as a department that works. This is undeniably a coveted identity for LGED, but it obviously does not end there. LGED has been successively striving hard to further improving its professional excellence and good governance. Preparation of this document is a proactive measure in that pursuit.

Through publication of this document I wish to reiterate propagation of this message of preference for construction of cost-effective sustainable infrastructures country-wide within the ambit of the LGED, being fully conducive in respect of optimal utilization of country’s limited resources and alleviating poverty. It equates fully this intent, which has been illustrated by the Government through PRSP.

Technology is continuously advancing and innovations are taking place to cater the needs of the changing time, so is this document. This document is certainly not preemiptory and may require periodic revisions, updating and refinement. Constructive and meaningful suggestions from any quarter to that respect will profoundly be acknowledged.

This document is a comprehensive one and has contemplated wide areas of ‘Building’ construction works that may not be fully needed to construct ‘Buildings’ of smaller type. It may as such be necessary that in the respective case this document is commodiously tailored in due proportion to the need so as to avoid any excess. Conversely, supplemental specifications may be required for any unusual type.

I find myself of high spontaneity in expressing my pleasure to stamping this publication for LGED’s use of highest order with regard to ‘Building’ construction works that would hopefully bring out the best in all of us.

(Md. Shahidul Hassan)
The Superintending Engineer  
Local Government Engineering Department

PROLOGUE

LGED is at present implementing quite a large number of projects in the sectors of different identities. Many of these projects comprise a component on construction of ‘Buildings’, be they are situated in the rural markets, union complex or in the urban locations. Although international Standards on ‘Building’ construction practices are available, but they are many in number and to some extent vary from one another. Technical specifications so long followed by the different projects of the LGED are different among themselves as well. In this backdrop, the need for developing a uniform standard as regards to the construction procedures as a whole of all infrastructures within the purview of the LGED has been keenly felt and so for the ‘Building’ works.

This document has featured almost every item related to ‘Building’ construction describing the item itself, materials requirement highlighting their storage aspect, the construction methods, quality control requirements, safety measures to be taken during construction period, the system of taking measurements of the works performed and the constitution of payment. All these are essential requirements in the implementation of any construction programme. This document has been prepared emphasizing all these phenomena corresponding to the international standards, local needs and practices and interfacing the existing social and economic spectrum of the country. It is surely not an iconoclastic imposition upon the country’s construction industries or the LGED’s technical personnel but is a user’s friendly approach towards demonstrating high quality performance in the face of whatever predicaments prevailing in the field.

This document is obviously not a conclusive one and is subject to modification and updating time to time and we sincerely invite any constructive suggestions for its future refinement.

I wish that LGED engineers and the contractors will make proper and ample use of this document and will demonstrate their perfection in constructing sustainable structures in terms of ‘Building’. At the same time I congratulate Mr. Rubaiyat Nurul Hasan, who as a Consultant has given deep thinking and enormous efforts in the compilation process of this document and the Committee, who has reviewed it. Above all the success of these efforts is lying in the proper adherence to the provisions of this document by the construction industries and the professionals of the LGED.

(Md. Wahidur Rahman)  
Converer,  
Standard Specification Committee
# TABLE OF CONTENTS

<table>
<thead>
<tr>
<th>SECTION-1</th>
<th>GENERAL AND SITE FACILITIES</th>
<th>1 - 15</th>
</tr>
</thead>
<tbody>
<tr>
<td>SECTION-2</td>
<td>CONSTRUCTION MATERIALS</td>
<td>16 - 31</td>
</tr>
<tr>
<td>SECTION-3</td>
<td>MATERIAL TESTING</td>
<td>32 - 34</td>
</tr>
<tr>
<td>SECTION-4</td>
<td>OFFICE SPACE AND FACILITIES FOR THE ENGINEER</td>
<td>35 - 38</td>
</tr>
<tr>
<td>SECTION-5</td>
<td>SITE PREPARATION &amp; REMOVAL OF EXISTING STRUCTURE</td>
<td>39 - 44</td>
</tr>
<tr>
<td>SECTION-6</td>
<td>EXCAVATION AND BACK-FILL FOR STRUCTURES</td>
<td>45 - 51</td>
</tr>
<tr>
<td>SECTION-7</td>
<td>DE-WATERING SYSTEM</td>
<td>52 - 54</td>
</tr>
<tr>
<td>SECTION-8</td>
<td>EARTH FILLING AND SAND FILLING</td>
<td>55 - 62</td>
</tr>
<tr>
<td>SECTION-9</td>
<td>BRICK MASONRY AND BRICK WORKS</td>
<td>63 - 75</td>
</tr>
<tr>
<td>SECTION-10</td>
<td>CONCRETE WORK</td>
<td>76 - 117</td>
</tr>
<tr>
<td>SECTION-11</td>
<td>JOINTS IN CONCRETE</td>
<td>118 - 123</td>
</tr>
<tr>
<td>SECTION-12</td>
<td>REPAIR OF EXISTING CONCRETE STRUCTURES</td>
<td>124 - 125</td>
</tr>
<tr>
<td>SECTION-13</td>
<td>REINFORCING STEEL</td>
<td>126 - 145</td>
</tr>
<tr>
<td>SECTION-14</td>
<td>SUB-SOIL BORING AND TESTING</td>
<td>146 - 149</td>
</tr>
<tr>
<td>SECTION-15</td>
<td>FOUNDATION PILES</td>
<td>150 - 177</td>
</tr>
<tr>
<td>SECTION-16</td>
<td>PILE LOAD TESTING</td>
<td>178 - 186</td>
</tr>
<tr>
<td>SECTION-17</td>
<td>FLOOR</td>
<td>187 - 193</td>
</tr>
<tr>
<td>SECTION-18</td>
<td>CEMENT PLASTER, POINTING AND WALL TILES</td>
<td>194 - 210</td>
</tr>
<tr>
<td>SECTION-19</td>
<td>LIME TERRACING AND DAMP PROOF COURSE</td>
<td>211 - 215</td>
</tr>
<tr>
<td>SECTION-20</td>
<td>PAINTING AND WHITENASH</td>
<td>216 - 244</td>
</tr>
<tr>
<td>SECTION-21</td>
<td>DOORS, WINDOWS AND CEILINGS</td>
<td>245 - 259</td>
</tr>
<tr>
<td>SECTION-22</td>
<td>STAIR RAILINGS</td>
<td>260 - 266</td>
</tr>
<tr>
<td>SECTION-23</td>
<td>MISCELLANEOUS METAL WORK</td>
<td>267 - 273</td>
</tr>
</tbody>
</table>
SECTION-1
GENERAL AND SITE FACILITIES
# GENERAL AND SITE FACILITIES

## CONTENTS

<table>
<thead>
<tr>
<th>SECTION-1</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>1.1</td>
<td>Introduction</td>
</tr>
<tr>
<td>1.2</td>
<td>Definitions</td>
</tr>
<tr>
<td>1.3</td>
<td>Scope of work</td>
</tr>
<tr>
<td>1.4</td>
<td>Submittal</td>
</tr>
<tr>
<td></td>
<td>Construction programme</td>
</tr>
<tr>
<td></td>
<td>Notice of operation</td>
</tr>
<tr>
<td></td>
<td>As-built drawings</td>
</tr>
<tr>
<td></td>
<td>Shop drawings</td>
</tr>
<tr>
<td>1.5</td>
<td>Taking over possession of Site</td>
</tr>
<tr>
<td>1.6</td>
<td>Mobilization</td>
</tr>
<tr>
<td>1.7</td>
<td>Monitoring progress</td>
</tr>
<tr>
<td></td>
<td>Monthly reports</td>
</tr>
<tr>
<td></td>
<td>Attendance at Site meetings</td>
</tr>
<tr>
<td></td>
<td>Receiving visitors</td>
</tr>
<tr>
<td>1.8</td>
<td>Contractor’s Site facilities</td>
</tr>
<tr>
<td>1.9</td>
<td>Materials, plant, equipment and tools</td>
</tr>
<tr>
<td></td>
<td>Products</td>
</tr>
<tr>
<td></td>
<td>Equal products and equivalents</td>
</tr>
<tr>
<td></td>
<td>Additional costs related to substitutions</td>
</tr>
<tr>
<td></td>
<td>Failure of equal products</td>
</tr>
<tr>
<td></td>
<td>Plant, equipment and tools</td>
</tr>
<tr>
<td>1.10</td>
<td>Sufficiency of means employed</td>
</tr>
<tr>
<td>1.11</td>
<td>Protection and safety</td>
</tr>
<tr>
<td></td>
<td>General</td>
</tr>
<tr>
<td></td>
<td>Safety of workmen</td>
</tr>
<tr>
<td></td>
<td>Site precautions</td>
</tr>
<tr>
<td>1.12</td>
<td>Care of works</td>
</tr>
<tr>
<td></td>
<td>Movement of transport and plant</td>
</tr>
<tr>
<td></td>
<td>Keeping works free from atmospheric condition</td>
</tr>
<tr>
<td></td>
<td>Materials on and under the Site</td>
</tr>
<tr>
<td>1.13</td>
<td>Survey works</td>
</tr>
<tr>
<td></td>
<td>Permanent Bench Mark</td>
</tr>
<tr>
<td></td>
<td>Reference line pillars</td>
</tr>
<tr>
<td>1.14</td>
<td>Fabricated items incorporated in the work</td>
</tr>
<tr>
<td>1.15</td>
<td>Inspection/tests at fabricator’s workshop</td>
</tr>
<tr>
<td></td>
<td>General</td>
</tr>
<tr>
<td></td>
<td>Tests and inspection record</td>
</tr>
<tr>
<td></td>
<td>Notice of works off Site</td>
</tr>
<tr>
<td></td>
<td>Standards</td>
</tr>
<tr>
<td></td>
<td>Proprietary products</td>
</tr>
<tr>
<td></td>
<td>Materials to be new</td>
</tr>
<tr>
<td></td>
<td>Orders for materials</td>
</tr>
<tr>
<td></td>
<td>Samples</td>
</tr>
<tr>
<td></td>
<td>Certificates</td>
</tr>
<tr>
<td>Section</td>
<td>Description</td>
</tr>
<tr>
<td>---------</td>
<td>---------------------------</td>
</tr>
<tr>
<td>1.16</td>
<td>Tolerances</td>
</tr>
<tr>
<td>1.17</td>
<td>Recording of measurement</td>
</tr>
<tr>
<td>1.18</td>
<td>Payment</td>
</tr>
</tbody>
</table>
SECTION-1
GENERAL AND SITE FACILITIES

1.1 Introduction

These Specifications shall apply to all such works to be executed involving construction of a building and its allied works under the Contract or otherwise directed by the Engineer. In every case, the Work shall be carried out to the satisfaction of the Engineer and conform to the location, lines, dimensions, cross-sections, etc shown on the Drawings or in the Bill of Quantities (BOQ) or as indicated by the Engineer. The quality of materials, processing of materials as may be needed at the site, salient features of the construction work and quality of finished works shall comply with the requirements set forth in the succeeding Sections and Sub-sections. Where the Drawings and Specifications describe a portion of the work in only general terms and not in complete detail, it shall be understood that only the best general practices are to prevail, materials and workmanship of the best quality are to be employed and instructions of the Engineer are to be fully complied with.

Words importing the singular also mean the plural and vice versa where the context so demands. Similarly, words importing the male also mean female or neuter and vice versa where the context so requires. Words have their normal meaning under the English language unless specifically defined.

1.2 Definitions

The following words and expressions shall have the meaning hereby assigned to them, except where the context otherwise require. However, in the case of any conflict with the stipulations of the Conditions of the Contract, the expressions and meaning of the Conditions of Contract shall prevail.

‘The Employer’ is the Local Government Engineering Department (LGED) representative by its Chief Engineer through the Project Director, ......................... Project and the Executive Engineer, LGED, ......................... District or any other representative appointed from time to time by the Chief Engineer and notified in writing to the Contractor to act as the LGED’s representative for the purpose of this Work.

‘The Engineer’ shall mean the engineer designated or any other engineer appointed from time to time by the Chief Engineer and notified in writing to the Contractor to act as ‘the Engineer’ for the purpose of the Contract.

‘The Contractor’ shall mean any person or corporate body who is pre-qualified under the Project/ enlisted with the LGED and whose Tender to carry out the Work has been accepted by the Employer and the legal successors in title to such person, but not (except with the consent of the Employer) any assignee of such person.

A ‘Sub-Contractor’ shall mean any person or corporate body named in the Contract as a Sub-contractor for a part of the Work or any person or corporate body to whom a part of the Work has been subcontracted with the consent of the Engineer and the legal successors in title to such person or corporate body, but not any assignee of any such person or corporate body.

‘The Contract’ is the contract between the Employer and the Contractor to execute, complete and maintain the Work.

The expression of ‘Work’ or ‘Works’ are what the Contract requires by the Contractor to construct, install and hand over to the Employer, as defined in the Tender Documents. Unless there be something either
in the subject or context repugnant to such construction it shall be construed and taken to mean the works by or by virtue of Contract to be executed, whether temporary or permanent and whether original, altered, substituted or additional.

‘Site’ means the places provided by the Employer where the Works are to be executed and any other places as may be specifically designated in the Contract as forming part of the Site.

‘Tender’ means the Contractor’s priced offer to the Employer for the execution and completion of the Work and the remedying of any defects therein in accordance with the provisions of the Contract, as accepted by the Letter of Acceptance.

‘Letter of Acceptance’ means the formal acceptance by the Employer of the Tender.

1.3 Scope of work

The Work to be carried out under the Contract shall consist of the various items as generally described in the Tender Documents as well as in the BOQ furnished in the Tender Documents.

The Work to be performed shall also include all general works preparatory to the construction of a building and all other related works. The Work shall include works of any kind necessary for the due and satisfactory construction, completion and maintenance of the works to the intent and meaning of the Drawings, BOQ and these Specifications and further Drawings and Orders as may be issued by the Engineer from time to time. Whether specifically mentioned or not in the various Sections of this Specification, the Scope of Work shall include compliance by the Contractor with all conditions of the Contract, all materials, apparatus, plant, equipment, tools, fuel, water strutting, timbering, transport, offices, stores, workshop, staff, labour and the provision for proper and sufficient protective works, temporary fencing and lighting, etc. It shall also include safety of workers, first-aid equipment, suitable accommodation for the staff and workmen with adequate sanitary arrangements, the effecting and maintenance of all insurance, the payment of all wages, salaries, fees, royalties, duties or other charges arising from the erection of works and the regular clearance of rubbish, reinstating and clearing the site as may be required on completion of the Work, safety of the public and protection of the Work and the adjoining land.

The Contractor shall ensure that all actions are taken to have a built-in quality assurance in the planning and execution of the Work. The quality assurance shall cover all stages of works such as setting out, selection of materials, selection of construction methods, selection of equipment and plant, deployment of personnel and supervisory staff, quality control testing, etc. The work of built-in quality assurance shall be deemed to be covered in the Scope of Work.

1.4 Submittal

The submittal by the Contractor shall include construction programme, all Shop Drawings, reports, samples, test results etc. to conform with all applicable provisions of the General Conditions of the Contract and as required under the various Sections of these Specifications. The purpose of the submittal required herein is to assure that items furnished and installed are, in all matters of consequence, equivalent to the specified items and that proper records are maintained of the changes made in the Specifications, Drawings or in materials used or any deviations made in the construction process.

The Contractor shall forward all submittal to the Engineer under a cover letter stating that the submittal have been carefully reviewed by the Contractor and that on-site conditions or dimensions where necessary and correctness have been verified and checked.
The submittal shall be reviewed by the Engineer to verify that the Contractor’s obligations are fulfilled as per the turn intention of the Contract. In checking and approving submittal, the Employer does not relieve the Contractor from responsibilities for construction errors or omissions, which may occur, even though executed in accordance with the approved Shop Drawings. Any such errors or omissions, as is discovered later on, should be corrected by the Contractor irrespective of any approval by the Employer at no additional cost to the Employer. This does not apply to modifications approved as specified herein.

The Contractor shall make submittal of construction requirements at least 10 days prior to actual construction of the component to allow time for checking and re-checking, if necessary. Any works fabricated or installed by the Contractor prior to approval of the Shop Drawings or other required submittal shall be done at his own risk.

Construction programme

Within 10 days of the Formal Work Order being issued, the Contractor shall submit to the Engineer for his approval a Bar Chart/Gantt Chart showing the programme sequence in which works have been proposed to be carried out including the procurement and delivery of equipment and materials.

The Contractor shall, whenever required by the Engineer, also provide in writing a general description of the arrangements and methods, which would be adopted for the execution of the Work.

If at any time it would appear to the Engineer that the actual progress of work does not conform to the approved programme, the Contractor shall be obliged to produce for the approval of the Engineer the reasons for any changes with a revised programme showing the modifications to the previously approved programme necessary to complete the Work on schedule. Submission to and approval by the Engineer of such programmes or furnishing of such particulars shall neither relieve the Contractor from any of his duties and responsibilities under the Contract nor it shall prejudice the ‘Liquidated Damages’ Clause of the Contract.

Notice of operation

The Contractor shall give full and complete written notice of all the important operations, including setting out, to the Engineer sufficiently in advance (not less than 10 days) to enable the Engineer to make such arrangements as the Engineer may consider necessary for inspection and for any other purposes. The Contractor shall not start any important operation without the written approval of the Engineer.

As-built drawings

Before the expiry of the period of maintenance, the Contractor shall submit the full sets of As-Built Drawings of the completed works to the Employer. The sets shall comprise the negatives of Drawings prepared with high quality reproducible polyester transparent “Mylar” film (or similar material) from which clear copy can be re-produced, three clearly printed Drawings and a CD.

The As-Built Drawing shall clearly show the lines and dimensions of the permanent construction actually made based on the changes to the original design from time to time as ordered by the Engineer or proposed by the Contractor and approved by the Engineer.

The original transparent negatives of the Tender Drawings and the Design Drawings will be lent free of charge to the Contractor on request free of charge for his making further prints or reproducing additional number of negatives of Drawings.
Shop drawings

The Contractor shall prepare the Shop Drawings at his own costs showing clearly all elements of construction that are required to assure proper shop fabrication or job installation of items requiring Shop Drawings shall be clearly shown. All material quality, finishes, construction details as specifically related to the project must be shown on the Shop Drawings.

1.5 Taking over possession of Site

The Contractor shall, upon receiving the Work Order, immediately take possession of the Site and move his men and materials to prepare the Site in order to create conditions for starting the Work as per terms of the Contract, Drawings and Specifications.

1.6 Mobilization

The work of mobilization shall consist of carrying out the following listed actions together with all other requirements of the Contract with regard to commencing the execution of the Work by the Contractor at his own cost.

(a) Procurement, assembly, repair and make to running condition of all the contractor-owned constructional plant and equipment by the Contractor at any other site as convenient to him.

(b) Transportation of Contractor-owned constructional plant, equipment and materials from the storage site as mentioned above in (a) to the place of construction.

(c) Assembling and installation of all items of constructional plant, equipment, etc. required for the execution of the Work.

(d) Receiving all constructional plant, equipment and materials to be furnished by the Employer, if any, and collect and transport those to the Work site. All materials shall be properly stored, inventoried and protected until used in to the Work and all plant and equipment shall be tested and made ready for use.

(e) Construction of a suitable Site office building or shed for storage of materials and equipment, workshop, other operational buildings and First-Aid Center attended by competent Medical Assistants.

(f) Maintenance of all temporary roads, fences and sanitary facilities, keep all areas used by the Contractor clean, neat, well-kept and in good repair and provide proper drainage to protect the area from surface run-off and flooding.

(g) Provide all the required electric power, water supply and other utility connections to temporary installations at the Site as may be necessary for the execution of the Work.

(h) Obtain all insurance policies, performance bond and payment guarantees as required under this Contract.

(i) Payment of all fees, permits, licenses, etc. as may be required covering the execution of the Contract.
1.7 Monitoring progress

Monthly reports

The Contractor shall furnish the Engineer, without cost to the Employer, at regular monthly interval and in a form and number of copies determined by the Engineer, with the following:

a) Physical progress for the month under report and the estimated progress for the following month.

b) Completion schedules (target and actual) based on the approved construction programme.

c) A tabulation of construction equipment listing the major items and pieces of equipment comprising the construction plant as were utilized for performance of the Work during the month under report.

d) A tabulation of employees countersigned by the Engineer’s representative, showing the supervisory staff and the number of several classes of labour employed by the Contractor in the month under report.

e) Any report which may be specifically requested by the Employer and/or by the Engineer.

Attendance at Site meetings

The Contractor shall attend punctually the progress and other on-site meetings as would be requested by the Engineer.

Receiving visitors

The Contractor shall receive all authorized visitors of the Employer and allow them to visit the Work in the manner as would be requested by the Employer.

1.8 Contractor’s Site facilities

The Contractor shall, at his own expenses, be responsible for the provision, maintenance, operation and subsequent removal of the following and all other necessary temporary facilities and services on Site those are required to accomplish the Work in a safe and orderly manner as per provisions of the Contract:

a) All temporary stores, warehouses and workshops.

b) All temporary buildings for office accommodation for the Contractor’s staff.

c) Living accommodation for staff.

d) Adequate number of toilets necessary for all persons engaged for the Work with separate arrangements for women. All sewage from toilets shall be disposed off by means of septic tank and soak pit or by some other acceptable disposal system.

e) To keep all sanitary facilities clean and their frequent disinfecting.

f) Fencing, lighting and security.

g) Cranes or other appropriate ways and means for off-loading plant and equipment, placing in temporary storage and moving from storage to equipment locations.

h) Site transport for the staff.

i) Electric power for temporary buildings and tools.
j) Provisions for adequate supply of water of acceptable quality at the Site for use in the Work.

k) Raw water from Site Tube-wells and provisions for adequate potable water.

In addition to above, the Contractor shall also make available all other necessary temporary facilities and services on Site those are required to accomplish the Work in a safe and orderly manner as per provisions of the Contract.

The Contractor shall submit for the approval of the Engineer detailed Plans and/or construction Drawings of the temporary buildings, warehouses, workshops and labour camps that he propose to construct or arrange on lease/rent including the proposals for water and power supply and sewerage facilities. These requirements shall be fulfilled by the Contractor within 10 (ten) days from receipt of the Formal Work Order to commence work (Date of commencement of Work). All buildings and facilities shall be of standard and acceptable to the Engineer.

The labour camps shall be at a location approved by the Engineer and conform to all requirements of the local law. It shall be laid and constructed in accordance with a Drawing prepared by the Contractor and approved by the Engineer.

The Contractor shall be responsible for acquiring the land deemed necessary for the Work beyond the Employer's land and for his temporary buildings, warehouses, workshops, staff quarters, labour camps and any temporary access road. The Contractor shall maintain the Site and all working areas in a safe and hygienic condition and in all matters of health and sanitation shall comply with the requirements of the local Medical Officer of Health or other competent Authority.

1.9 Materials, plant, equipment and tools

Products

The Contractor at his own expenses shall provide the materials, products, plant and equipment as shown on the Drawings or as specified in the Contract. Necessary haulage and safe storage of materials, supervision of works etc. shall be provided by the Contractor.

Equal products and equivalents

Except as specifically required otherwise, the mention of any proprietary materials by trade name is intended to establish a standard of quality, appearance, size and durability. The products of other manufacturers may be used subject to the conditions as stated below.

Additional costs related to substitutions

Any additional costs, or any losses or damages, arising from the substitution of any materials or methods from those originally specified shall be borne by the Contractor, unless such substitution was made at the written request or direction of the Employer.

Failure of equal products

Where products are accepted, based on representation of the Contractor, as approved equals, those shall be used subject to the same installation and performance standards as required by the original specification. Approval of a request for substitution shall not modify the Contract requirements except as specifically noted. Subsequent failure of “approved equals” shall be considered first. For any evidence of improper installation or product inequality, the installation shall be repaired or corrected as directed by the Engineer at the full costs of the Contractor.
Plant, equipment and tools

The Contractor shall furnish all constructional plant, equipment and tools for the proper execution of the Work at his own expenses and keep those in proper working condition. The Contractor shall supply the Employer a list of major items of the constructional equipment and tools that he proposes to use in execution of the Work.

1.10 Sufficiency of means employed

The Contractor shall take upon himself the full and entire responsibilities for the sufficiency of his supervisory and other personnel, plant or equipment or tools, scaffolding, timbering and generally for all means used for the fulfillment of the Contract. In the event of any of these means proving insufficient, the Contractor shall remain fully and entirely responsible for the sufficiency of these means notwithstanding any previous approval or recommendation that might have been given by the Engineer.

1.11 Protection and safety

General

The Contractor at all times shall take all necessary measures to the safety of life and property during construction of various parts of a building. International Safety Manuals used in Engineering Construction Project shall be adopted for protection and safety at the construction Site during the period of construction. Nothing stated herein shall be construed to nullify any rules, regulations, safety standards or statutes of the local authority, or those contained in the various Acts of the Government of Bangladesh. The specific rules, regulations and Acts pertaining to the protection of the public or workmen from health and other hazards wherever specified by the local Authority etc. or by the Act/Ordinance of the Government shall take precedence over whatever are specified herein.

Safety of workmen

Helmets conforming to BDS 1265 and BDS 1266 shall be worn by the workmen and other personnel at all times while works are going on.

Safety goggles of accepted standard (BDS 1360) shall be used by individuals engaged in drilling, cutting, welding and all such works which cause hazard to the eye. The welders and gas cutters shall be equipped with proper protective equipment like gloves, safety boots, aprons and hand shields having filter glass of accepted standard and suitable to the eyes of a particular worker.

Site precautions

In absence of boundary walls, construction Site shall be delineated by fences.

Warning signs shall be displayed, where necessary, to indicate hazardous areas like high voltage zone, area of no smoking etc. Hand lamps shall be of low voltage, preferably 24V. All electrically operated hand tools shall be provided with double earthing.

The temporary wells, which shall be provided by the Contractor at the construction Site as a part of the toilet facilities, shall be provided with proper covers.

The toilet facilities shall be located at a corner of the Site so as to avoid any obstruction. Protection from bad weather and falling object and proper privacy shall be provided to the toilet users.

Temporary toilets shall be dismantled, all wells filled up, and the whole area made level, dressed and restored back to proper grade at the end of the project.
The Contractor at all times shall protect the excavation, trenches and building materials from rain water, groundwater, backing up of drains and from water of any origin. He shall provide all pumping arrangements for removal of surplus water, coverings and other materials as required.

All rubbish and debris shall be removed from the Site and disposed off at a safe distance as per direction of the Engineer so as not to create any obstruction to Work or give rise to health hazards.

The Contractor shall take all necessary precautions to ensure against fire during construction. The Contractor must make all necessary arrangements for providing adequate protection against fire hazards at the construction site during the period of execution of the Work.

Timber, coal, paints and similar combustible materials shall be separated from each other. A minimum of two dry chemical powder (DCP) type fire extinguishers shall be provided at both open and covered locations where combustible and inflammable materials are stored.

Inflammable liquids like petrol, thinner etc., shall be stored in conformity with the relevant regulations. Explosives like detonators, gun powder etc. shall be stored in conformity with all fire protection provisions set forth in the Bangladesh National Building Code.

1.12 Care of works

Movement of transport and plant

The Contractor shall exercise diligence and care in the movement of all transports and plant within the Work area so as not to cause injury or damage to life or property. The Contractor shall be responsible for restoring any roadway, bridge, culvert etc. damaged by his transports and plant to the satisfaction of the Engineer or the appropriate Authority.

Keeping works free from atmospheric condition

The Contractor shall construct all temporary works and other works and supply and operate pumping plant and ensures all measures as may be found necessary for the construction of the Work under proper atmospheric condition.

Notwithstanding any approval by the Engineer of the arrangements made, the Contractor shall remain responsible for the sufficiency thereof and shall be liable for keeping the works safe at all time regardless of the climatic condition at his own expenses. Any loss of production, additional overheads or additional costs of any kind that may result from inclement climatic conditions shall be at the Contractor’s risk.

Materials on and under the Site

All soil, turf, gravel, stone, timber, or other materials obtained in the excavations, clearing of the Site of the Work and soil stripping, shall belong to the Employer and must not be removed from the Site without the written permission of the Engineer. Provided the Engineer directs the Contractor, he may use for the construction of the Work, any timber obtained from trees felled at the Site and any of the materials excavated under the Contract, which the Engineer may determine to be fit for such use.

1.13 Survey works

Permanent Bench Mark

Before commencing the work the Contractor shall establish at his own cost, at least 2 (two) permanent Bench Marks (B.M) with permanent pillars at suitable positions as per direction of the Engineer. These B.Ms. shall be incorporated in the Drawings and used for controlling all levels of construction works.
Reference line pillars

The Contractor shall establish permanent Reference Line Pillars (axis pillars, centre line pillars, etc.) at his own cost for all structures before starting of excavation of foundation pits/trenches as per standard practice and or as per direction of the Engineer.

The Contractor shall remain responsible for safeguarding all Survey Monuments, Bench Marks, Beacons, etc. The Contractor, at his own expenses, shall make necessary arrangements to protect the B.M pillars against any disturbances, damages, including their maintenance.

The Engineer will provide the Contractor with the data necessary for setting out of the center line. All dimensions and levels shown on the Drawings or mentioned in the Documents forming part of or issued under the Contract shall be verified by the Contractor on the Site and he shall immediately inform the Engineer of any apparent error or discrepancy, if found by him in such dimensions or levels. The Contractor shall, after or in connection with these staking out of the center line, survey the terrain and shall submit to the Engineer for his approval, a profile as required by the Engineer.

Instruments and equipment for surveys shall be subject to rigorous inspection by both the Contractor and the Engineer and any items found to be defective in the opinion of the Engineer, shall be promptly replaced, repaired or adjusted as per his direction. A qualified Surveyor or Engineer shall supervise all survey works.

The checking of the setting-out of works by the Engineer’s staff shall not relieve the Contractor of any of his liabilities or responsibilities under the Contract.

1.14 Fabricated items incorporated in the work

Whenever required by the Specifications to fabricate or manufacture and furnish equipment for incorporation in the permanent works, the Contractor shall submit to the Engineer for his approval the names of the manufacturers or fabricators the Contractor proposes to use and also his detailed Shop Drawings for approval before proceeding with the Work. All such Drawings shall be adequately and properly checked before being submitted to the Engineer for approval and shall be so designated.

Any fabricating or manufacturing undertaken during or before the approval of the Drawings, will be at the Contractor’s risk. The Engineer shall have the right reserved to ask the Contractor to make any changes in the Design, which may be found necessary in the opinion of the Engineer, for the equipment or component materials to fully meet the requirements and intent of these Specifications without causing any additional costs to the Employer.

Approval of the Contractor’s Drawings shall not relieve the Contractor of any part of his obligation to meet all requirements of these Specifications or of the responsibilities for the correctness of his Drawings. At the time of delivery of the equipment, the Contractor, if requested to do so, shall furnish the Engineer two complete sets of negatives of the final approved Drawings.

1.15 Inspection/tests at fabricator’s workshop

General

All equipment furnished under these Specifications and all works performed thereon will be subject to inspection by the Engineer or his authorized representative. Inspection at the manufacturer’s plant, when located only in Bangladesh, may be made with the intention to determine the meeting of requirements of the Specifications in respect of use of equipment and materials.

The Contractor shall notify the Engineer a minimum of 15 (fifteen) days in advance of the date and place of equipment/materials to be available for inspection. No equipment or materials shall arrive at the Site
until the Engineer’s inspection at the manufacturer’s plant or Contractor’s storage place outside the actual Site has been made, the Engineer’s approval has been given, final Drawings have been furnished by the Contractor and the Contractor’s responsibilities for furnishing equipment and materials meeting the requirements of the Contract Document are fully complied with. All costs of the Engineer’s inspection shall be borne by the Contractor.

**Tests and inspection record**

The record shall identify the Contractor and the Supervision Consultant staff (when applicable) involved, the place, the date and time when the inspection is completed, the sections of the Work and the materials tested or inspected and its state of completion. Reference shall be made to the relevant Working Drawings and the specific aspects or properties, which were checked or measured, shall be recorded.

One copy of each record of inspection shall be submitted to the Engineer and one copy of each record of inspection shall be submitted to the Supervision Consultant (when involved). The Contractor shall maintain records of inspections and tests in an orderly fashion at the Site until the issuance of the Defects Liability Certificate for the whole of the Work, or such earlier time as the Engineer may instruct. The Engineer shall have the rights of access to them at all times.

After the issuance of the Defects Liability Certificate for the whole of the Work, or such earlier time as the Engineer may instruct, the Contractor shall, as instructed by the Engineer, either dispose of the records or deliver them as directed.

**Notice of works off-Site**

The Contractor shall give adequate written notices to the Engineer on the preparation or manufacture at a place not within the Site of any pre-fabricated units or parts of units or materials to be used in the Work. Such notices shall state the place and time of the preparation or manufacture, quarrying or extraction. The notice be given sufficiently in advance as to enable the Engineer to make arrangements which he may deem necessary for inspection before the start and at any stage of the Work and not only at the time when the units or parts are completed. Off-Site works shall not commence without the prior approval of the Engineer.

Any unit or parts, prepared or manufactured without giving such prior notice to the Engineer, may be rejected, if the Engineer considers that his inspection was necessary during the time of preparation or manufacture. No inspection by the Engineer shall relieve the Contractor of any of his responsibilities, duties and liabilities under the Contract.

**Standards**

Except where otherwise specified or authorized by the Engineer, all materials and workmanship shall conform to the latest edition of the relevant Standard Specifications of the ASTM or BNBC.

Materials meeting other internationally accepted equivalent or higher Standards may be accepted subject to review by the Engineer. The Contractor shall submit in English language any such alternative Standards proposed by him, for approval by the Engineer.

The Contractor shall provide the Engineer 3 (three) sets of each of the Standards, Codes and References to be used in the Contract within 45 (forty-five) days of the Date of Commencement of the Work. In addition, he shall supply 3 (three) copies of any other Standards or Codes subsequently specified or alternatively proposed to be used by the Engineer, the Supervision Consultant (when
involved) and the Site Laboratory. All Standards shall be in English. On completion of the Contract, all copies of Standards, Codes and References, so provided, shall become the properties of the Employer.

**Proprietary products**

Where a proprietary or brand name or the name of a supplier or manufacturer is indicated on the Drawings or in the Specifications, this would be in respect of items, which have not otherwise being adequately described by ASTM, BNBC or equivalent recognized Standards. Alternative items based on recognized national Standards of the country of origin may be accepted provided that documented proof in the English language is submitted to the Engineer for his approval sufficiently in advance and showing that the alternative proposal is equal or higher in quality and performance than the specified item.

**Materials to be new**

All materials used in the permanent works shall be new. No materials, incorporated in the permanent works, shall have previously been used in the temporary works.

**Orders for materials**

Before orders are placed for any materials of any description to be used in the permanent works, the Contractor shall submit to the Engineer the names and addresses of the manufacturers or suppliers proposed. Following approval by the Engineer, the Contractor shall submit to him copies of all orders placed for such materials.

**Samples**

In accordance with the provisions of the Contract, the Contractor shall, in the way as directed by the Engineer, supply samples of materials to be incorporated in the Work. The Contractor shall submit the samples required for approval in labeled boxes suitable for storage and with sufficient time for testing. Due allowance shall be kept for the fact that if samples are rejected, further samples and testing will be required. The Engineer shall keep the approved samples with him and will compare the supply with the sample before acceptance. He shall reject any materials not conforming to the character and quality of the approved samples.

**Certificates**

All manufacturer’s certificates of tests, proof sheets, mill sheets etc., showing that the materials have been tested in accordance with the requirements of the relevant ASTM, BNBC or other approved Standard or this Specification, shall be supplied in English language by the Contractor to the Engineer free of charge.

### 1.16 Tolerances

Unless it has been specified in the different Sections otherwise, all works shall be constructed within the tolerances shown in the Table given below.

<table>
<thead>
<tr>
<th>Type of Structure</th>
<th>Item</th>
<th>Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>Concrete Structures</td>
<td>Tolerances from the specified position (Structure)</td>
<td>25mm</td>
</tr>
<tr>
<td></td>
<td>Maximum departure of plan position of structure or element</td>
<td></td>
</tr>
<tr>
<td></td>
<td>Tolerances from the specified dimensions (Structure)</td>
<td></td>
</tr>
<tr>
<td></td>
<td>Maximum departure in thickness or cross sectional dimensions of columns, beams, buttresses, wall</td>
<td>+6mm</td>
</tr>
<tr>
<td>Type of Structure</td>
<td>Item</td>
<td>Tolerance</td>
</tr>
<tr>
<td>-------------------</td>
<td>------</td>
<td>-----------</td>
</tr>
<tr>
<td>footings etc., up to and including 500mm thick (except tunnel and shaft linings)</td>
<td>Ditto – between 500mm and 1000mm thick</td>
<td>-3mm</td>
</tr>
<tr>
<td>Ditto – between 1000mm and 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>+10mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>+10mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>-5mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>+10mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>-8mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>+25mm</td>
</tr>
<tr>
<td>Ditto – over 4000mm thick</td>
<td>Ditto – over 4000mm thick</td>
<td>-10mm</td>
</tr>
</tbody>
</table>

**Tolerances from specified position (Surface)**

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Maximum departure of vertical, sloping or curved surfaces including joint surfaces</td>
<td>25mm</td>
<td></td>
</tr>
<tr>
<td>Maximum departure of horizontal or near-horizontal surfaces including joint surfaces</td>
<td>20mm</td>
<td></td>
</tr>
<tr>
<td>Tolerance on Straightness or Departure from Specified Curve (Surface)</td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

**General Surface**

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Maximum deviation in horizontal or vertical directions (gradual)</td>
<td>12mm in 2m</td>
<td></td>
</tr>
<tr>
<td>Maximum deviation in horizontal or vertical directions (abrupt)</td>
<td>6mm</td>
<td></td>
</tr>
</tbody>
</table>

**Formwork**

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Sectional dimension</td>
<td>+5mm</td>
<td></td>
</tr>
<tr>
<td>Plumb</td>
<td>+1 in 1000 of height</td>
<td></td>
</tr>
<tr>
<td>Levels (before any deflections has taken place)</td>
<td>+3mm</td>
<td></td>
</tr>
</tbody>
</table>

**Reinforcement**

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>Length of splice</td>
<td>-25mm</td>
<td></td>
</tr>
<tr>
<td>Variation of protective cover</td>
<td>+5mm</td>
<td></td>
</tr>
<tr>
<td>Variation in indicated position or reinforcement:</td>
<td>One bar diameter</td>
<td></td>
</tr>
<tr>
<td>• Starter bars</td>
<td></td>
<td></td>
</tr>
<tr>
<td>• Slabs and Walls</td>
<td>0.25 times the indicated spacing</td>
<td></td>
</tr>
<tr>
<td>• Beams and columns</td>
<td>+5mm</td>
<td></td>
</tr>
<tr>
<td>Dimension of bent bars:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>• Stirrups and ties</td>
<td>+5mm</td>
<td></td>
</tr>
<tr>
<td>• Other bars</td>
<td>+10mm</td>
<td></td>
</tr>
</tbody>
</table>

**R.C.C. Piles**

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td>1. Pre-cast driven pile:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a) Verticallity for vertical pile</td>
<td>1 in 50</td>
<td></td>
</tr>
<tr>
<td>b) Verticallity for raker pile</td>
<td>1 in 25</td>
<td></td>
</tr>
<tr>
<td>c) Deviation from position shown on the plan for vertical and raker piles after driving</td>
<td>1/4th of least dimension or 75mm whichever is greater</td>
<td></td>
</tr>
<tr>
<td>Concrete piles casting tolerances:</td>
<td></td>
<td></td>
</tr>
<tr>
<td>a) Maximum departure in thickness or cross sectional dimensions</td>
<td>+6mm</td>
<td></td>
</tr>
<tr>
<td></td>
<td>- 0.00</td>
<td></td>
</tr>
</tbody>
</table>
### Table of Tolerances

<table>
<thead>
<tr>
<th>Type of Structure</th>
<th>Item</th>
<th>Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>b) Deviation of pile face</td>
<td>6mm in 3m</td>
</tr>
<tr>
<td></td>
<td>c) Deviation of cross-section centroid from straight line connecting the centroid of the end faces of the pile</td>
<td>10mm</td>
</tr>
<tr>
<td>2. Bored and Cast-in-situ pile:</td>
<td>a) Verticality for vertical pile</td>
<td>1 in 75</td>
</tr>
<tr>
<td></td>
<td>b) Verticality for raker pile</td>
<td>1 in 25</td>
</tr>
<tr>
<td></td>
<td>c) Deviation from position shown on the plan for vertical and raker pile shaft</td>
<td>Maximum 75mm in any direction</td>
</tr>
<tr>
<td>Timber Piles</td>
<td>Deviation of cross-sectional dimension</td>
<td>-6mm</td>
</tr>
<tr>
<td></td>
<td>Deviation of cross-section centroid from straight line joining end face centroid</td>
<td>40mm</td>
</tr>
<tr>
<td></td>
<td>Level of top pile</td>
<td>+12mm</td>
</tr>
</tbody>
</table>

* In addition to above, other tolerances have also been specified in the different Sections and Sub-sections in the relevant portions.

#### 1.17 Recording of measurement

Conditions of the Contract, Technical Specifications and Contract Drawings are to be read in conjunction with the Bill of Quantities (BOQ).

General directions and descriptions of works and materials are not necessarily be repeated nor summarized in the BOQ. References to the relevant Sections of the Contract documents shall be made before entering the Tender’s rate.

The quantities given in the BOQ are only approximate and provisional and are given to provide a common basis for tendering. It does neither expressly nor by implication prescribed that the actual volume of work to be performed will exactly correspond therewith.

Any clarification regarding the BOQ and the Method of Measurement shall be adjudged by the Engineer in accordance with this Standard Specification, its Sub-sections, BOQ and other Tender Documents.

The works, executed fully complying the Drawings and instructions of the Engineer, will be measured for payment in accordance with the method adopted in the BOQ and the item therein set forth, notwithstanding any custom to the contrary. The net quantities of the finished works in place will always be taken except where otherwise specified.

No allowance shall be made for waste, laps, cuttings, etc. and no deduction will be made for grout nicks, joggle holes or rounded arises and sinkage or for fitting iron works, etc.

#### 1.18 Payment

Full account shall be taken of all information contained in the Tender Documents and made available during the tender period as affects, inter-alia, working methods, haulage requirements and sequence of operations. Full allowance shall be made for all these provisions in the rates and sums entered against the various items in the BOQ of the Contract.

The specified payment Sections/Sub-sections of the Contract shall apply to any additional or varied works, which may be required to execute under the Contract except where specifically varied therein.
The basis of payment will be the actual quantities of works ordered and carried out, as measured by the Engineer (based on the As-Built Drawing, BOQ or otherwise as directed by the Engineer) and valued at the rates and prices of the Tender, where applicable, or otherwise at such rates and prices as (in case of non-tendered items) the Engineer may fix within the Terms of the Contract.

No payment will be made on account of the anticipated profit for work covered by the Contract, which is not performed. No adjustment will also be made in the unit rates set out in the Bill of Quantities because of an increase or decrease in the actual quantities from the Estimated quantities indicated therein, unless otherwise stated in the Conditions of Contract.

Notwithstanding any limit, which may be implied by the wording of the individual item and or the explanations in this Section, it is to be clearly understood that the Tender price is for the works finished and completed in every respect. Full account of all requirements and obligations have to be taken, whether expressed or implied covered by all parts of the Contract. The Tender price shall, therefore, include all incidental and contingent expenses (including all taxes and VATs) and risks of every kind necessary to construct, complete and maintain the whole of the Work in accordance with the Contract. Full allowance is to be made in the Tender price for all costs involved in the following, inter-alia, which are referred to and/or specified herein:

- All setting-out and survey works.
- Temporary access unless separately billed, fencing, guarding, lighting, and all temporary works including their removal on completion.
- Paying fees and giving notices to the Authorities.
- Reinstatement of the Site.
- Safety precautions and all measures to prevent and suppress fire and other hazards.
- Interference to the works by persons or vehicles being legitimate users of the facilities on or in the vicinity of the Site.
- Protection and safety of adjacent structures so far as they may be affected by the works or temporary works.
- Supplying, maintaining and removing the Contractor’s own housing for staff and labour, offices, workshop, plant yard, transport, welfare, services in connection therewith and other facilities required by the Contractor on completion of work unless separately billed.
- Working in the dry condition except where otherwise permitted by the Specification.
- Supplying, inspection and testing of materials intended for use in the works including the provision and use of equipment.
- Maintaining public roads and footpaths.
- Opening quarries and borrow pits including all surveys, site investigations, removal and disposal of overburden, trimming of quarry or borrow pit faces and floors and all measures necessary to render quarries or pits safe and free for draining on completion.
- Providing and transporting to Site all equipment necessary for the execution of the Work, setting to works, operating (including all fuel and consumable stores), removal from the Site all construction equipment upon completion of the Work, costs of all tests and other requirements in respect of such plant and equipment.
The requirements and all incidental costs and expenses involved to provide all necessary skilled and unskilled labours and supervision.

Protection of all completed works following operations making good damages to any completed works due to any cause whatsoever, clearing all rubbish as they accumulate and leaving the Site in a tidy condition.

All costs associated with the provision and submission of Progress Reports, Records, Photographs, preparation of the necessary Shop and Working Drawings etc. except those provided in the Bill of Quantities.

Workmen’s compensation and Owner’s liability insurance.

Payments under the item for hiring of land (if there be any) in addition to the Employer’s land for temporary works shall be made in accordance with the receipts obtained from the land owners within the limitation of quoted rate only if such provision is made in the BOQ of the Contract.

Payment of royalties for fill materials obtained from privately owned land/carried earth shall remain included within the rates of the relevant items of the Contract. The volume of borrowed materials shall be calculated on the basis of pre-work and post-work measurements. Finished sections as per Drawings will be the basis for post-work measurement while the Work is complete as per Specifications.

Payment shall mean gross payable amount on the rates of the BOQ including the Performance Security.

With regard to the Sub-section on ‘Contractor’s Site Facilities’, payment will be made for hiring land for the Contractor’s temporary works outside the Employer’s property, only if such provisions are kept in the BOQ of the Contract.

The cost of keeping the works free from water will only be paid for, if referred to in the BOQ of the Contract Documents.

No payment shall be made for any tests required under the Specification unless specifically referred to in the BOQ. If the Engineer requires any tests outside the BOQ, the cost of such test shall be agreed with the Engineer before execution and paid for as a supplementary item.

No direct payment shall be made for works required under other Sub-sections. The costs for such works shall be deemed included in the related items of the BOQ.
SECTION-2
CONSTRUCTION MATERIALS
## CONSTRUCTION MATERIALS

### CONTENTS

#### SECTION-2

<table>
<thead>
<tr>
<th>2.1 Bricks</th>
<th>16 - 18</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>16</td>
</tr>
<tr>
<td>First class bricks</td>
<td>16</td>
</tr>
<tr>
<td>Picked jhama bricks</td>
<td>16</td>
</tr>
<tr>
<td>First class machine made bricks</td>
<td>17</td>
</tr>
<tr>
<td>Perforated bricks</td>
<td>17</td>
</tr>
<tr>
<td>Clinker bricks</td>
<td>18</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.2 Aggregates</th>
<th>18-22</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>18</td>
</tr>
<tr>
<td>Coarse aggregate</td>
<td>19-21</td>
</tr>
<tr>
<td>General</td>
<td>19-20</td>
</tr>
<tr>
<td>Stone aggregate</td>
<td>20</td>
</tr>
<tr>
<td>Brick aggregate</td>
<td>20</td>
</tr>
<tr>
<td>Storage of coarse aggregate</td>
<td>20-21</td>
</tr>
<tr>
<td>Fine aggregate</td>
<td>21-22</td>
</tr>
<tr>
<td>General</td>
<td>21</td>
</tr>
<tr>
<td>Impurities</td>
<td>21</td>
</tr>
<tr>
<td>Grading</td>
<td>21-22</td>
</tr>
<tr>
<td>Sand fill</td>
<td>22</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.3 Cement</th>
<th>22-24</th>
</tr>
</thead>
<tbody>
<tr>
<td>White cement</td>
<td>23</td>
</tr>
<tr>
<td>Rejection of cement</td>
<td>23</td>
</tr>
<tr>
<td>Storage of cement</td>
<td>23-24</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.4 Admixture</th>
<th>24</th>
</tr>
</thead>
</table>

<table>
<thead>
<tr>
<th>2.5 Reinforcement</th>
<th>24-25</th>
</tr>
</thead>
<tbody>
<tr>
<td>Mild steel bar</td>
<td>24</td>
</tr>
<tr>
<td>High strength deformed rod</td>
<td>24</td>
</tr>
<tr>
<td>Cleaning and storage</td>
<td>24</td>
</tr>
<tr>
<td>Pre-stressing steel and anchorage</td>
<td>25</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.6 Rustless tying wire</th>
<th>25</th>
</tr>
</thead>
</table>

<table>
<thead>
<tr>
<th>2.7 Lime</th>
<th>25</th>
</tr>
</thead>
<tbody>
<tr>
<td>Storage and handling of lime</td>
<td>25</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.8 Water</th>
<th>25-26</th>
</tr>
</thead>
</table>

<table>
<thead>
<tr>
<th>2.9 Fill</th>
<th>26</th>
</tr>
</thead>
</table>

<table>
<thead>
<tr>
<th>2.10 Timber</th>
<th>26-27</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>26</td>
</tr>
<tr>
<td>Inspection</td>
<td>26</td>
</tr>
<tr>
<td>Wrought faces and allowances on joiner’s work</td>
<td>26</td>
</tr>
<tr>
<td>Timber piles</td>
<td>27</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.11 Geo-textile</th>
<th>27-28</th>
</tr>
</thead>
<tbody>
<tr>
<td>General</td>
<td>27</td>
</tr>
<tr>
<td>Geo-textile bags</td>
<td>27-28</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>2.12 Marble</th>
<th>28</th>
</tr>
</thead>
<tbody>
<tr>
<td>Marble stone</td>
<td>28</td>
</tr>
<tr>
<td>Marble chips</td>
<td>28</td>
</tr>
</tbody>
</table>
2.13 Glass
- Glass general
- Flat glass
- Wired glass
- Storage and handling of glass

2.14 Wire gauge
- Wire gauge general
- Other materials

2.15 Paints and protective materials
- Knotting
- Red lead paint
- Linseed oil putty
- Varnishes/wood polish
- White wash
- Colour wash
- Oil bound distemper
- Emulsion paints
- Creosote
- Storage and handling of paint, varnishes, etc

2.16 Aluminium members

2.17 Structural steel for doors and windows

2.18 Pipes
- M.S. pipe
- PVC pipe
- Storage and handling of pipe

2.19 Gunny bags
SECTION-2
CONSTRUCTION MATERIALS

2.1 Bricks

General

Bricks shall be manufactured from clay or shale or a combination of these materials and shall be uniformly burnt throughout. They shall be hard and sound and give a clear metallic ring when struck with a small hammer or another brick and should not break when dropped to the earth from a height of 1.5m with one brick above another in the formation of a ‘T’. The surface should be too hard to be scratched with the fingernail.

Bricks shall be stacked on dry firm ground in regular tiers. Each stack shall comprise 50 bricks in length and 10 bricks in height, the bricks being placed on edge. The width of each stack shall be formed with two bricks. Clear distance between adjacent stacks shall be not less than 800mm.

Bricks shall be loaded or un-loaded with care, and shall not be thrown or dumped. They shall be carried from the stack to the Site of placement in small batches as and when necessary.

First class bricks

First Class Bricks shall comply with the following requirements:

appearance. Sound, hard and well burnt, uniform in size, shape and colour, homogeneous in texture and shall have plane rectangular faces with parallel sides and sharp straight right-angled edges. This shall be of uniform colour (generally deep red or copper), homogeneous in texture and free from cracks, flaws and nodules of free lime. A fractured surface shall show a uniform compact structure free from holes, lumps or grits. Shall emit clear metallic sound when struck. When scratched by steel or nails, there should be no permanent mark on the surface.

Unit Weight 2000 kg/m³

Crushing strength 170 kg/cm² (average) but not less than 140 kg/cm² in any individual brick

Maximum water absorption 20% of dry weight

Efflorescence Nil

Dimensions (±3mm) 240mm x 115mm x 70mm

Picked jhama bricks

Picked Jhama Bricks shall be over-brunt first class bricks, uniformly vitrified throughout with good shape, hard, slightly black in colour and without cracks or spongy areas.

Water absorption, as a percentage of the dry weight, shall not exceed 15%.

Crushing strength should be on average 210 kg/cm², but not less than 170 kg/cm² in any individual bricks

All other requirements for First Class Bricks shall also apply to Picked Jhama Bricks.
**First class machine made bricks**

First Class Machine Made Bricks shall be thoroughly burnt and shall have plane rectangular faces with parallel sides and sharp straight right-angled edges. They shall be of uniform colour (generally deep red or copper), homogeneous in texture and free from cracks, flaws and nodules of free lime. A fractured surface shall show a uniform compact structure free from limps and grits of holes.

Other requirements of the First Class Machine Made Bricks shall comply with the following requirements:

- **Minimum crushing strength**: 210 kg/cm².
- **Maximum water absorption**: 10% of dry weight.
- **Efflorescence**: Nil.
- **Dimensions (± 5mm)**: 200mm x 100mm x 50mm.

**Perforated bricks**

Perforated bricks shall meet the following specifications:

- **Minimum compressive strength on gross area for**
  - Multi-core brick: 70 kg/cm².
  - 10-hole engineering bricks: 210 kg/cm².
- **Maximum size of perforation**: 645 mm².
- **Minimum number of perforation**
  - Along the width of brick: 2.
  - Along the length of brick: 6.
- **Minimum wall thickness**
  - Between brick edge and perforation: 16 mm.
  - Between adjacent perforation: 10 mm.
- **Maximum water absorption**: 12% of dry weight.
- **Efflorescence**: Nil.
- **Dimension**: 241mmx114mm x 70mm.

The perforations may be of any regular shape in cross-section. In case of a rectangular section, the larger dimension shall be parallel to the longer side of brick. Dimension of perforation measured parallel to the plane of the shorter side shall not be more than 16 mm except in case of circular shape of the perforation in which case it may be allowed up to 20mm. Total area of perforation shall not exceed 45% of the total area of corresponding faces of the brick.

In all other respect the perforated bricks shall conform to the specifications of the First Class Machine Made Bricks.
Clinker bricks

Clinker bricks shall meet the following specifications:

- Minimum compressive strength: 562 kg/cm²
- Minimum modulus of rupture: 42 kg/cm²
- Maximum water absorption: 12% - 15% of dry weight.
- Efflorescence: Nil.
- Dimension: 203mm x 102mm x 51mm.

Clinker bricks shall be manufactured by dry process and burnt to a higher temperature and shall be uniformly vitrified to a dark copper tone.

Arises shall be square, straight and sharply defined.

2.2 Aggregates

General

Aggregates shall be hard, strong, durable, dense and free from injurious amount of adherent coatings, clay, lumps, dust, soft or flaky particles, shell, mica, alkali, organic matter and other deleterious substances. The various sizes of particles of which an aggregate is composed of shall be uniformly distributed throughout the mass.

Testing of aggregates shall be in accordance with BS 812 or ASTM C-136.

Approval of a source of aggregate by the Engineer shall not be construed as constituting the approval of all materials to be taken from that source and the Contractor shall be responsible for the specified quantity and quality of all such materials used in the Work. Aggregates shall not be obtained from sources, which have not been approved by the Engineer.

The Contractor shall provide means of storing aggregates at each point where concrete is made such that:

- aggregates shall be stored on a hard and dry patch of ground covered with a 50mm thick layer of lean concrete
- each nominal size of coarse aggregate and the fine aggregate shall be kept separated at all times
- contamination of the aggregates by the ground or other foreign materials shall be effectively prevented at all times
- each heap of aggregate shall be capable of draining freely
- the aggregates shall be handled so as to avoid segregation

The Contractor shall make available to the Engineer such samples of the aggregate as he may require. Such samples shall be collected at the point of discharge of aggregate to the batching plant/mixer machine. If any such sample does not conform with the Specifications, the aggregate shall promptly be removed from the Site and the Contractor shall carry out such modifications to the supply and storage arrangements as may be necessary to secure compliance with the Specifications.
Coarse aggregate

General

Coarse aggregate shall be obtained from breaking hard durable rock or gravel or Picked Jhama Bricks, which conform to the requirements of AASHTO Standard Specifications M-80. Coarse aggregate shall be clean, free from dust and other deleterious materials. The grading of the coarse aggregate shall be such that when combined with the approved fine aggregate and cement, it shall produce a workable concrete of maximum density.

Aggregate pieces shall be angular in shape and have granular or crystalline or smooth, but not glossy non-powdery surfaces.

Maximum allowable limits of deleterious substances that shall not be exceeded for coarse aggregate are shown in the following table:

<table>
<thead>
<tr>
<th>Material</th>
<th>Mass Percent</th>
</tr>
</thead>
<tbody>
<tr>
<td>Soft fragments</td>
<td>2.00</td>
</tr>
<tr>
<td>Clay lumps</td>
<td>0.25</td>
</tr>
<tr>
<td>Material passing the 0.075mm sieve</td>
<td>0.50 for clay</td>
</tr>
<tr>
<td></td>
<td>1.50 for fracture dust</td>
</tr>
<tr>
<td>Thin or elongated pieces: Flakiness index (STP T 7.13) less than</td>
<td>30.00</td>
</tr>
</tbody>
</table>

The Aggregate Crushing Value (STP T 7.7) shall be less than 30% or the Ten percent Fine Value (STP T 7.8) shall be greater than 150 kn.

Grading for nominal size coarse aggregate shall comply with the following ASTM C-33 standard gradations:

20mm nominal size Coarse Aggregate

<table>
<thead>
<tr>
<th>Sieve Size (mm)</th>
<th>% Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>25</td>
<td>100</td>
</tr>
<tr>
<td>19</td>
<td>90-100</td>
</tr>
<tr>
<td>12.50</td>
<td>20-55</td>
</tr>
<tr>
<td>9.50</td>
<td>0-15</td>
</tr>
<tr>
<td>4.75</td>
<td>0-5</td>
</tr>
</tbody>
</table>

40mm nominal size Coarse Aggregate

<table>
<thead>
<tr>
<th>Sieve Size (mm)</th>
<th>% Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>50</td>
<td>100</td>
</tr>
<tr>
<td>37.5</td>
<td>95-100</td>
</tr>
<tr>
<td>19</td>
<td>35-70</td>
</tr>
<tr>
<td>9.5</td>
<td>10-30</td>
</tr>
<tr>
<td>4.75</td>
<td>0-5</td>
</tr>
</tbody>
</table>

Coarse aggregate subject to five cycles of the Soundness Test, specified in ASTM C88, shall not show a loss exceeding 10% when magnesium sulphate solution is used except where otherwise approved.

The flakiness and elongation indices of the predominant size fractions in each single sized coarse aggregate, determined in accordance with BS 812, shall not exceed 20% and 35% by weight respectively.
Aggregate for use in concrete which is subject to abrasion and impact shall comply with the Test requirements of BS 812 and the Specification of BS 63 Part 1 and BS 63 Part 2 and BS 882 respectively.

Coarse aggregate shall be tested for drying shrinkage characteristics in accordance with BRS Digest No. 35.

Coarse aggregate shall be stored at Site in such a manner that it is not contaminated by fine aggregate, earth or other foreign matter. Adequate precautions shall be taken to prevent segregation of the coarse aggregate while it is being transported and stacked.

**Stone aggregate**

The boulders to be used as coarse aggregate in concrete shall be composed of limestone, sandstone, granite, trap rock or rock of similar nature and shall have the following properties:

<table>
<thead>
<tr>
<th>Property</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Compressive strength (minimum)</td>
<td>490 kg/cm²</td>
</tr>
<tr>
<td>Specific gravity</td>
<td>2.4 – 2.7</td>
</tr>
<tr>
<td>Unit-weight</td>
<td>2245 – 2566 kg/cum</td>
</tr>
<tr>
<td>Porosity</td>
<td>2 – 6%</td>
</tr>
<tr>
<td>Water absorption</td>
<td>1.5 – 5% by weight</td>
</tr>
</tbody>
</table>

The boulder shall be of uniform light colour as approved and shall be free from thin lamination, adherent coatings and deleterious substances. The wear loss of coarse aggregate of all types shall not exceed 35% by weight when tested by the Los Angeles Abrasion Test.

The boulders shall be supplied in sizes that can be handled manually by one person. Stock piling shall be such as to permit ready identification of the materials and shall be approved by the Engineer. Site for stockpiles shall be clean prior to storing materials. The stockpiles shall be built up in layers not to exceed 1.22m in height and each layer shall be inspected before the next layer is started. The crushed boulder chips shall be stacked in accordance with the specified sizes in different stacks as directed by the Engineer. Height of each stack should not exceed 33% of the minimum base dimension of the stack.

**Brick aggregate**

Brick aggregate shall be as far practically as possible of uniform specific gravity. Blown bricks or unevenly burnt bricks shall not be crushed for the purpose of providing aggregates. Best possible first class picked jhama bricks of selected quality only shall be allowed for crushing.

Brick aggregate shall consist of first class Picked Jhama Brick chips graded as stated above under the Sub-section "General". All brick aggregates shall be screened and washed at Contractor’s own costs and shall consist of clean, well shaped cubical particles, free from splintered or flaky particles, soil, organic matter or any deleterious materials.

**Storage of coarse aggregate**

Aggregate of different sizes or grades and from different sources of supply shall not be mixed. All aggregate shall be stored separately free from contact with earth and other deleterious matter. The coarse aggregate should be stockpiled in different stacks, according to the sieve sizes.

All precautions shall be taken during transport and stockpiling of coarse aggregate to prevent segregation. Segregated aggregate shall not be used until they have been thoroughly re-mixed and the resulting stack is of uniform and acceptable gradation.
Aggregate shall be stock-piled at least 7 (seven) days prior to their anticipated use to permit the Engineer to sample each stock-pile to determine the acceptability of the material for the intended use.

**Fine aggregate**

**General**

Fine aggregates for use in the concrete and masonry work shall be non-saline clean natural sand and have a Specific Gravity not less than 2.6 and conform to the requirements of ASTM C 144. It shall be angular (gritty to touch), hard and durable, free from clay, mica and soft flaky pieces. All sands must be well washed and clean before use.

A well graded sand should be used for cement work as it adds to the density of the mortars and concretes. Sand required for brick work needs to be finer than that for stone work.

Sand which contains 90% of particles of size greater than 0.06mm and less than 0.2mm is fine sand. On the other hand, sand which contains 90% of particles of size greater than 0.6mm and less than 2mm is coarse sand.

Supply methods and stock piling of sand shall be such, as to permit ready identification of the material delivered and shall be approved by the Engineer.

**Impurities**

Sand shall be clean and free from injurious amount of organic impurities. Deleterious substances shall not exceed the following percentage by weight.

<table>
<thead>
<tr>
<th>Material</th>
<th>Percentage by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>Material passing No. 200 sieve</td>
<td>2.0</td>
</tr>
<tr>
<td>Shale, coat, soft or flaky fragments</td>
<td>1.0</td>
</tr>
<tr>
<td>Sulphur compounds</td>
<td>0.3</td>
</tr>
<tr>
<td>Clay Lumps (wet, on No. 4 sieve)</td>
<td>0.00</td>
</tr>
</tbody>
</table>

Fine aggregate subject to five cycles of the soundness test, specified in ASTM C88 shall not show a loss exceeding 10 mass percent when magnesium sulphate solution is used except where otherwise approved.

**Grading**

Sand shall be well graded from coarse to fine within the limits given below or shall conform to the specified Fineness Modulus.

<table>
<thead>
<tr>
<th>Fine aggregate for concrete</th>
<th>% Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sieve No.</td>
<td></td>
</tr>
<tr>
<td>9.5mm</td>
<td>100</td>
</tr>
<tr>
<td>4</td>
<td>95-100</td>
</tr>
<tr>
<td>16</td>
<td>45-80</td>
</tr>
<tr>
<td>50</td>
<td>10-30</td>
</tr>
<tr>
<td>100</td>
<td>2-10</td>
</tr>
</tbody>
</table>
### Fine aggregate for masonry

<table>
<thead>
<tr>
<th>Sieve No.</th>
<th>% Passing by Weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>4</td>
<td>100</td>
</tr>
<tr>
<td>8</td>
<td>95-100</td>
</tr>
<tr>
<td>16</td>
<td>70-100</td>
</tr>
<tr>
<td>30</td>
<td>40-75</td>
</tr>
<tr>
<td>50</td>
<td>10-35</td>
</tr>
<tr>
<td>100</td>
<td>2-15</td>
</tr>
</tbody>
</table>

**Sand fill**

Sand for sand fill shall consist of hard, dense, durable materials free from injurious amounts of clay lumps, light weight materials or other deleterious substances.

Unless otherwise specified on the Drawings, sand fill with gunny bags shall have Fineness Modulus not less than 0.8.

Sand fill for the Geo-textile bags shall, unless otherwise approved by the Engineer, comply with the following grading:

<table>
<thead>
<tr>
<th>mm</th>
<th>0.60 to 0.30</th>
</tr>
</thead>
<tbody>
<tr>
<td>d90</td>
<td>0.50 to 0.25</td>
</tr>
<tr>
<td>d86</td>
<td>0.40 to 0.20</td>
</tr>
<tr>
<td>d60</td>
<td>0.35 to 0.20</td>
</tr>
<tr>
<td>d50</td>
<td>0.20 to 0.05</td>
</tr>
</tbody>
</table>

#### 2.3 Cement

Cement used in the works shall be obtained from manufacturers, approved in writing by the Engineer and shall be Ordinary Portland Cement complying with the requirements of ASTM C150 Type 1 or BS 12 or BDS 232 or equivalent standard. Special cements shall conform to the requirements provided in writing by the Engineer.

A certificate showing the place of manufacture and the results of standard tests carried out on the bulk supply from which the cement was extracted must accompany each consignment of cement delivered to the Site.

The Engineer may make any tests, which he considers advisable or necessary to ascertain, if the cement has deteriorated in any manner during transit or storage. Any cement which, in the opinion of the Engineer, is of doubtful quality shall not be used in the Work until it has been re-tested and test result sheets, showing that it complies in all respects with the relevant standard, have been delivered to and accepted by the Engineer.

Cement that becomes lumpy or otherwise deteriorated in transit or storage shall not be used for brick masonry or concrete works. All cement, found unsuitable for use, shall be removed from the Site immediately.

The Engineer shall ask to carry out sampling, inspection and testing of all cement as may consider be necessary. Samples shall be taken as instructed from the Site store or from elsewhere on the Work or from any places where cement is used for incorporation in the Work. The compressive strength and tensile strength of standard cubes and briquettes respectively shall be not less than as follows:
<table>
<thead>
<tr>
<th>Days</th>
<th>Compressive strength (N/mm²)</th>
<th>Tensile strength (N/mm²)</th>
</tr>
</thead>
<tbody>
<tr>
<td>3</td>
<td>12.4</td>
<td>1.0</td>
</tr>
<tr>
<td>7</td>
<td>19.3</td>
<td>1.9</td>
</tr>
<tr>
<td>28</td>
<td>27.6</td>
<td>2.4</td>
</tr>
</tbody>
</table>

Initial setting time shall be not less than 45 minutes and the final setting time shall be not more than 8 hours. Cement, when tested for fineness, shall have a specific surface of not less than 160m²/kg. Cement when tested for soundness shall not have an expansion of more than 10 mm. The unit weight of cement shall be a minimum of 14.16 KN/m³.

**White cement**

White Cement shall be made from pure calcite lime stone and have the same physical properties as those of Portland Cement Type 1, ASTM C-150.

A typical composition of White Cement is as follows:

- CaO: 65%
- SiO₂: 25.5%
- Al₂O₃: 5.9%
- Fe₂O₃: 0.6%
- MgO: 1.1%
- SO₃: 0.1%

**Rejection of cement**

The Engineer may reject any cement as the result of any tests thereof notwithstanding the manufacturer’s certificate. The Engineer may also reject cement, which has deteriorated owing to inadequate protection or from other causes where the cement is not to his satisfaction. The Contractor shall remove at his cost all rejected cement from the Site without delay.

**Storage of cement**

Cement shall be delivered at the Site in sound and properly sealed jute/paper bags, each plainly marked with manufactures name or registered mark. Cement shall be well protected from weather by tarpaulins or other approved cover during transit. Weight of individual bag containing cement shall be 50 kg and weight of all bags shall be uniform. Weight of cement shall be legibly marked on each bag. Bags in broken or damaged condition shall be rejected.

The Contractor shall provide waterproof and well-ventilated godowns at the specified or approved location at the Site having a floor of wood or concrete raised platform at minimum 450mm above the ground so as to protect the cement against moisture from air or from any other source. Sheds shall be large enough to allow a minimum 300mm gap between the stacked cement and the godown walls to store cement in sufficient quantity to ensure continuity of work and to permit each consignment to be stacked separately therein to permit easy access for inspection. All storage facilities shall be subject to approval by the Engineer.

Immediately upon arrival at the Site, cement shall be stored in the godowns with adequate provisions to prevent absorption of moisture. The Contractor shall use the consignments in the order in which they are received. Cement delivered to the Site in drums or bags provided by the supplier or manufacturer, shall be stored in the drums or bags until used in the Work. Any cement in drums or bags, which has been
opened, shall be used immediately on opening. Cement shall not be stored in a godown for more than 3 (three) months if bagged or 6 (six) months, if in bulk or a lesser period as directed by the Engineer. After this period is over, any unused cement shall be removed from the Site.

2.4 Admixture

Admixture shall be used to provide excellent acceleration of gaining strength at early age and major increase in strength at all ages by significantly reducing water demand in a concrete mix, especially suitable for pre-cast concrete and other high early strength requirements. Admixture shall conform to BS 5075 Part 3 and ASTM C 494.

2.5 Reinforcement

Mild steel bar

This is a type of bar plain and round or deformed in shape of a structural or intermediate grade conforming to ASTM Specification A 510 or A 615 with a yield strength of not less than 280 MPa (N/mm²) i.e. 40 grade.

High strength deformed rod

Reinforcing steel under this type comprises Grade-60 Deformed re-bars. The steel shall conform to ASTM Specification A 617M or A 615M of yield strength not less than 420 MPa (N/mm²). The structural grade shall be made from billets. The ends of the bar shall be machine sheared perpendicular to the axis of the bar. The bars shall be free from injurious defects and shall have a workman like finish.

Cleaning and storage

Steel reinforcement bars and structural steel shall be stored in a way to prevent distortion, corrosion, scaling and rusting. Reinforcement bars and structural steel sections shall be coated with cement wash before stacking, especially in humid areas. In the case of long time storage or storage in coastal areas, reinforcement bars and steel sections shall be stacked at least 200mm above the ground level.

Steel sections shall be stacked upon platforms, skids or any other suitable supports. Bars of different sizes and lengths and structural sections shall be stored separately to facilitate issues in required sizes and lengths without cutting from standard lengths. Ends of bars and sections of each type shall be painted with separate designated colours.

Tag line shall be used to control the load in handling reinforcing bars or structural steel when a crane is used. Heavy steel sections and bundles of reinforcing bars shall be lifted and carried with the help of slings and tackles.

All bars, prior to its use, shall be cleaned with wire brush to make them free from nail scale, loose rust, dirt, paint, oil, grease or other foreign substances.

Bars of reduced sectional area to excessive rust shall be rejected.

All reinforcing steel shall be stored properly under shed not to be contaminated by oil, grease, dirt or mud.

All stacking and storing of bars shall be the Contractor’s responsibility and contingent upon his Tender.
Pre-stressing steel and anchorage

Pre-stressing reinforcement shall comprise high strength seven wire strand, high strength steel wire or high strength alloy bars conforming grade and type as shown on the Drawings.

Un-coated seven-wire strand shall conform to the specifications of AASHTO M 203.

Un-coated stress-relieved steel wire shall conform to the specifications of AASHTO M 204.

Un-coated high-strength bars shall conform to the specifications of AASHTO M 275.

2.6 Rustless tying wire

Rustless tying wire of 18 SWG shall be obtained from approved manufacturers and shall, as regards strength, comply with the requirements specified. The Contractor shall, at his own costs, provide binding wires of required specifications.

2.7 Lime

Lime shall be stone lime of good quality high calcium lime containing calcium oxide from 95% upwards. The impurities, insoluble in acids, should not exceed 3% for the quick lime and 1% for the hydrated lime. Limes shall conform to the requirements of ASTM C 5 for quick lime and ASTM C 207 for hydrated lime.

Storage and handling of lime

Quicklime shall be slaked as soon as possible. If not possible, it may be stored in compact heaps having only the minimum of exposed area. The heaps shall be stored on a suitable platform under a roof protected from rain and wind. A minimum space of 300mm shall be provided all round the heaps to avoid bulging of walls.

Un-slaked lime shall be stored in a watertight place and shall be separated from combustible materials.

Hydrated lime shall be supplied either in containers or sacks, such as jute bags lined with polyethylene or high density polyethylene woven bags lined with polyethylene or craft paper bags. It shall be stored in a dry room to protect the lime from dampness and to minimize warehouse deterioration.

When dry slaked lime is to be used within a few days, it shall be stored on a covered platform and protected from rain and wind. It shall be kept in a dry airtight godown when immediate use is not required. However, it shall never be stored for more than two months.

Workmen, handling bulk lime, shall wear protective clothing, respirators and goggles. They shall be instructed for cleanliness as a preventive measure against dermatitis and shall be provided with hand cream, petroleum jelly or similar protectors.

2.8 Water

Water shall be clean, fresh and free from organic or inorganic matter in solution or suspension in such amount that may impair the strength or durability of the concrete. Water shall be obtained from a supply, where possible. However, it may be taken from any other sources, only if approved. No water from excavation shall be used. Only water of approved quality shall be used for washing shuttering, curing of concrete and similar other purposes.
Water to be used in construction shall be stored in tanks, bottom and the sides of which shall be constructed with brick or concrete. Contact with any organic impurities shall be prevented.

The tank shall be so located as to facilitate easy storage and filling in, and supply for construction works and other purposes.

2.9 Fill

Materials for filling shall be uniform in character throughout and free from substances that by decay or otherwise may cause the formation of hollows or cavities or otherwise affect the stability of the filling.

Earth filling shall be of selected materials obtained from the excavation or carted fine sand as approved by the Engineer. No soft chalk or clay or earth with a predominating clay content shall be used. Hard core shall be selected hard clean gravel, broken brick, broken concrete, broken or crushed stone, quarry waste or similar approved materials. Concrete for filling shall be to the proportions specified.

2.10 Timber

General

All timbers for temporary or permanent works shall be of best quality, sound, straight and well seasoned. They shall be free from sap, defects, radial cracks, cup-shakes, large/loose/dead knots, or other imperfections and shall show a clean surface with cut.

Timber shall be stored in stacks on well treated and even surfaced beams, sleepers or brick pillars so as to be at least 200mm above the ground level. Members shall be stored separately in layers according to the lengths.

A space of 25mm shall be kept between the members. The longer pieces shall be placed in the bottom layers and the shorter pieces in the top layers. At least one end of the stack shall be in true vertical alignment.

The recommended width and height of a stack are 1.5m and 2.0m respectively. Minimum distance between two stacks shall be 800mm.

The stacks of the timbers shall be protected from hot dry wind, direct sun and rain. Weights may be placed on top of the stacks to prevent wrapping of timber. Nails, metal straps, etc. attached to used timber shall be removed before stacking.

Inspection

All timbers shall be subject to inspection at Site piece by piece and shall be to the approval of the Engineer who may reject such timber as is considered by him to be under-specified. In the case of timber specified to be creosoted, the Engineer may reject such timber before or after creosoting, if specifications are not correctly followed. The Contractor shall provide all necessary labour for handling the timber during inspection free of charge.

Wrought faces and allowances on joiner's work

All joiner’s works shall be wrought and finished with a clean, even and smooth face. Thickness shall be given to include 2mm for each wrought face in soft-wood and 1.5mm for hard wood.
Timber piles

Timber piles shall be made of Sal, Sundari, Gajari or any other approved hard wood. They shall be matured, straight and free from large or loose knots, cracks and other defects.

Piles shall have a minimum diameter of 100mm measured at one-third point from the thickest end (butt) without bark. Piles should be straight and a straight line drawn from the center of the butt to the center of the tip shall be contained entirely within the pile.

Timber piles exposed permanently above water shall be treated with a water repellent preservative such as creosote for a minimum period of 24 hours in accordance with BS 5268, Para 5, 1977.

2.11 Geo-textile

General

All geo-textiles shall be manufactured and supplied by a firm or firms of reputable geo-textile manufacturers. The Engineer shall approve the quality of geo-textile and the manufacturer as well.

Before placing an order for any quantity of geo-textile, the Contractor shall submit samples and test reports to the Engineer for approval for each type of geo-textile from an independent testing laboratory, approved by the Engineer.

The geo-textiles to be incorporated within the works shall comply with the appropriate Codes and Standards including the following:

- ASTM D4491 Standard test methods for water permeability of geo-textile by permittivity.
- DIN 53936 (pt1) Determination of the water permeability coefficient $k_{v1}$ normal to the geo-textile plane with constant head.
- ISO 9073-1 Determination of mass per unit area for non-woven textiles.
- ISO 9073-2 Determination of thickness of non-woven textiles.
- ISO 9073-3 Determination of tensile strength and elongation of non-woven textiles.

The filter effective opening size, $O_{90}$, defined as being the grain size of a standard sand corresponding to 90% retention by weight on a sample of the geo-textile in a vibrating sieve apparatus, shall be measured in a wet apparatus using the BAW (Bundesanstalt fur Wasserbau – German Federal Institute for Waterways Engineering) method.

All geo-textiles shall be clearly and uniformly marked on the upper face. The marking shall take the form of an indelible repeat roll imprint at the edge of each geo-textile roll recurring at least every 1.5m.

Geo-textile bags

Geo-textile bags shall be manufactured from short staple non-woven geo-textile weighing not less than 0.8 kg/m², and with $O_{90}$ not greater than 0.07mm or similar material approved by the Engineer.

Geo-textile bags shall be manufactured to the dimensions and capacity specified on the Drawings and filled with sand which complies with the requirements stated in the preceding Sub-section.
Each bag shall be double stitched along all edges except for the opening at the top of each bag, which shall be wide enough to allow the filling of the bag. The minimum tensile strength of the seam shall be not less than 90% of the tensile strength of the geo-textile. The top of each bag shall have a flap, which shall be closed tightly after filling and then double stitched.

The bags shall be stored under cover, well covered from direct sunlight and to prevent the ingress of dust or mud. They shall be protected from damage by insects or rodents.

2.12 Marble

Marble stone

Marble shall be of Italian origin or equivalent and size as per Drawing having approved colour and texture.

Marble chips

Marble chips shall be white in approved colour and shall be of size # 2-3 (retained on screens 6mm and 19mm mesh). The chips shall be of uniform colour and texture and shall be made from white marble stone, a calcareous metamorphic rock, which is capable of being polished and have following properties:

- Compressive strength: 562 – 844 kg/cm²
- Specific gravity: 2.72
- Unit weight: 2563 – 2724 kg/m³

Marble dust

Marble dust shall consist of finely grounded white marble stone and 90% shall pass sieve # 100.

2.13 Glass

Glass general

All glass shall be obtained from an approved manufacturer and be free from blemishes of all kinds and descriptions, whether surface or internal.

Flat glass

Flat glass shall be provided where specified or directed in the following grades:

- 24 oz. flat drawn clear sheet glass.
- 32 oz. clear sheet glass.
- 6mm thick ‘Georgian’ rough cast wired glass.
- 6mm thick polished glass.

Wired glass

Wired glass shall be 6mm thick with wire reinforcements inside and shall be obtained from an approved manufacturer and shall be subject to the approval of the Engineer.
Storage and handling of glass

All glass sheets shall be kept dry and stored in a covered place. Glass sheets shall be lifted and stored upright on their long edges and put in to stacks of not more than 25 sheets. They shall be supported at two points at about 300mm from each end by fillets of wood.

The bottom of each stack shall be about 25mm clear from the base of the wall and other support against which the stack rests. The whole stack shall be as close to upright as possible. Smooth floors shall be covered with gunny bags.

Workmen handling glass sheets, remnants and waste glass pieces and fibre-glass shall be provided with gloves, jelly and other suitable hand protections. In removing glass sheets from crates, great cares shall be taken to avoid damages and breakage. Glass edges shall be covered or protected to prevent injuries to workmen.

2.14 Wire gauge

Wire gauge general

Gauge for fly proofing shall be of the quality uniformly woven webbing of 23 meshes per square centimeter. The wire for the gauge shall be of best quality 22 SWG brass or copper wire or any other approved materials.

Other materials

Gauge known as “plastic gauge” may also be used as and when required by the Engineer.

2.15 Paints and protective materials

Knotting

Knotting shall be uniform dispersion of lac or suitable resin (natural or synthetic) in a suitable solvent.

White lead paint shall be made from pure white lead in accordance with BS 239, mixed with fine boiled linseed oil, turpentine, dryers and pigments and strained free from skins and all extraneous matter before being pigments. If so used, the quantity shall not exceed 8% of the paint mixed ready for the brush. No other ingredient except the colouring matter will be allowed and the colour shall be produced by using the least required amount of colouring matter. The proportions of the ingredients for the various coats shall be subject to the approval of the Engineer.

Red lead paint

Red lead paint shall be made from non-setting red lead in accordance with BS 217, thoroughly ground and well and freely mixed with approximately 15% of boiled linseed oil to give a paint with good covering power, body and adhesion. It shall be determined by tests to be made by the Contractor to the satisfaction of the Engineer. The Engineer may select samples of the paint for analysis after a sufficient quantity of the work about to be painted has been mixed.

Linseed oil putty

Putty for stopping and glazing shall consist of whiting/chalk powder thoroughly ground with linseed oil to form a smooth paste, and shall conform BS 544.
Varnishes/wood polish

The material is required to be clear and transparent and when applied shall on drying, give a glossy coating free from fun and specks. The composition of the varnish shall conform to the requirements of BS 274.

White wash

White wash shall be made from pure flat lime brought to the work in an unslaked condition. Water shall be added to this lime in a tub until the mixture is of the consistency of cream and shall be allowed to rest for a period of 48 hours. The mixture shall then be strained through an approved cloth strainer and 4 kg of gum boiled with 12 kg of rice and a suitable quantity of blue shall be added per cubic meter of the mixture.

Colour wash

Colour wash, where not of an approved proprietary brand, shall be made from pure selected fat lime as described above for white wash, to which shall be added and intimately mixed the necessary pigment to produce the tint specified. The pigment shall be to the approval of the Engineer.

Oil bound distemper

Oil bound distemper shall comply with BS 1053 Type-1 and shall be obtained from an approved manufacturer.

Emulsion paints

Robbialac/Berger/Elite Emulsion Paints shall preferably be used but the Engineer may allow any other brands of equivalent standard subject to the production of appropriate test certificates and guarantees.

Creosote

The Creosote is a paint used for preservation of timber. It shall be pure tar distillate of the best quality as obtained and sold under the trade name ‘SOLIGNUM’. The ‘SOLIGNUM’ shall be clear so as not to mar the timber. Other brands equivalent to ‘SOLIGNUM’ may also be used, if only approved by the Engineer.

Storage and handling of paint, varnishes, etc.

Paints, varnishes, lacquers and thinners shall be kept in properly sealed or closed containers. The containers shall be kept in a well ventilated location, free from excessive heat, smoke, sparks or flames. The floor of the paint store shall have at least 100 mm thick loose sand on it.

Temporary electrical wiring and fittings shall not be installed in a paint store. When electrical lights, switches or electrical equipment are necessary to be stored or used in the same room, the room shall be designed in a way to reduce explosion risks.

Buckets containing sand shall be kept ready for use. A five-kilogram dry powder fire extinguisher conforming to accepted standards shall be kept at an easily accessible position close to the paint store.

2.16 Aluminium members

Aluminium doors, windows, curtain walls, etc. shall be of approved standard conforming to the U.S. Architectural Aluminium Manufacturing Association (AAMA) or equivalent specifications. The frames and sash members shall be of extruded shape made of 6063 – T5 high quality aluminium alloy having a
minimum section thickness of 2mm unless otherwise shown on the Drawings or indicated in the BOQ and shall conform to the U.S. Aluminium Association or equivalent standard.

2.17 **Structural steel for doors and windows**

All steels used in doors and windows shall be the products of reputable manufacturer and shall conform to the American Standard Specifications. The sections, sizes and profiles shall be as per the requirements for a specific work as shown on the Drawings.

2.18 **Pipes**

**M.S. pipe**

M.S. Pipe shall be made from low carbon steel conforming to the requirements of ASTM A 53 and physical requirements as specified therein.

**PVC pipe**

PVC pipe shall be of unplasticized polyvinyl chloride and shall conform to BS 3500: 1968/3506:1969 or equivalent. The pipes shall be laid and jointed in accordance with the manufacturer’s instructions and to the Engineer’s satisfaction.

**Storage and handling of pipe**

Pipes shall be stored in stacks with stoppers provided at the bottom layer to keep the pipe stack stable. The stack, particularly of smaller diameter pipes, shall be in a pyramid shape. Pipes shall not be stacked more than 1.5m height.

Each stack shall have pipes of the same type and size only. Removal of pipes shall start from the top layer and by pulling from one end. A pipe shall not be stored inside another pipe. The pipes may also be placed alternately length and crosswise.

PVC pipes shall be stored in a shaded area. The ends of pipe, particularly those especially prepared for jointing, shall be protected from abrasion. Damaged portion of a pipe shall be cut out completely.

Pipes of conducting materials shall be stacked on solid level sills and contained in a manner to prevent spreading or rolling of the pipe. For storage in large quantity, suitable packing shall be placed between the layers. During transportation, the pipes shall be so secured as to prevent displacement/rolling.

2.19 **Gunny bags**

The gunny bags used in the permanent works shall be new, 50/75 kg capacity bags similar to those normally used. The Contractor shall submit sample bags to the Engineer for his approval.
SECTION-3
MATERIAL TESTING
### MATERIAL TESTING

#### CONTENTS

#### SECTION-3

<table>
<thead>
<tr>
<th>Section</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>3.1 General</td>
<td>32</td>
</tr>
<tr>
<td>3.2 Tests</td>
<td>32-34</td>
</tr>
<tr>
<td>Bricks</td>
<td>32</td>
</tr>
<tr>
<td>Coarse aggregate</td>
<td>32</td>
</tr>
<tr>
<td>Fine aggregate</td>
<td>32</td>
</tr>
<tr>
<td>Cement</td>
<td>33</td>
</tr>
<tr>
<td>Reinforcement</td>
<td>33</td>
</tr>
<tr>
<td>Test for water</td>
<td>33</td>
</tr>
<tr>
<td>Workability test for concrete</td>
<td>33</td>
</tr>
<tr>
<td>Strength test for concrete</td>
<td>33-34</td>
</tr>
<tr>
<td>Testing geo-textiles</td>
<td>34</td>
</tr>
<tr>
<td>3.3 Expenses for tests</td>
<td>34</td>
</tr>
</tbody>
</table>
SECTION-3
MATERIAL TESTING

3.1 General
Notwithstanding the requirements stated in the detailed specifications for individual items, the following minimum tests shall be carried out in the LGED specific laboratories and in the field. In the cases the testing facilities are not available in the LGED laboratories, the tests shall be performed elsewhere as directed by the Engineer.

Contractor’s Materials Engineer will be responsible for liaison and coordination with the Site laboratory, the Engineer, field sampling/testing staff and off-Site laboratories to ensure that all sampling, specified tests and inspections are carried out in a timely manner.

No inspection or approval by the Engineer shall relieve the Contractor of any of his duties and obligations under the Contract.

All test types and quantities described in the following Sub-sections are considered “Normal Testing” and anything beyond that in type and quantity is considered as “Special Testing”. The Engineer may increase the frequency of testing as per requirement.

3.2 Tests

Bricks
For each consignment not exceeding 100,000 bricks, minimum 6 (six) bricks shall be tested to ascertain:
- Dimensions and unit weight
- Compressive strength
- Water absorption
- Efflorescence

Coarse aggregate
The tests mentioned below shall be carried out for each day’s casting or per 15 cubic meter of concrete whichever provides the greater number of tests.
- Gradation
- Unit weight
- Water absorption
- Specific gravity
- Abrasion loss/Crushing loss

Fine aggregate
The tests mentioned below shall be carried out for each day’s casting or per 15 cubic meter of concrete whichever provides the greater number of tests.
- Gradation
- Fineness Modulus (F.M.).
- Specific Gravity
- Water absorption
- Surface moisture
Cement

For each consignment of a particular brand not exceeding 25 tons, at least 3 (three) samples collected random shall be tested prior to the cement be incorporated in to the works to ascertain:

- Consistency
- Setting time
- Compressive strength
- Fineness

Reinforcement

For each consignment not exceeding 10 (ten) tons or as directed, 3 (three) representative samples of each size of M.S. bar shall be tested for:

- Cross sectional area
- Unit weight
- Measurement of deformation
- Yield strength
- Tensile strength
- Elongation
- Bending

Only Test Certificates issued by BUET or the concerned regional University of Engineering and Technology shall be accepted by the Engineer when the requisite test facilities are not available with the LGED Laboratories.

Test for water

Water will be tested to ensure that it remains free of oil, salt, acid, alkali, sugar, vegetable or other injurious substances.

Workability test for concrete

The Slump Test shall be carried out as frequently as required by the Engineer and not less than one per hour during placing of concrete.

Strength test for concrete

The compressive strength of the concrete shall be determined by Cylinder Test. The Cylinder moulds shall be 150mm in diameter and 300mm long. Each class of concrete shall be represented by at least six Cylinders. Not less than one group of six test Cylinders shall be made for each 30 cubic meter of structural concrete, but there shall be at least one group of six test Cylinders for each day’s concrete work. For columns and girders, one set of test Cylinders would be made from each batch of concrete not exceeding one cubic meter. Samples from which compression test specimen are moulded, shall be obtained in accordance with the Method of Sampling Fresh Concrete (ASTM C 172). The concrete samples would be collected from a point just before final placement or as directed by the Engineer. Cylinders may be collected from any batch (load) including the first. Specimens made to check the adequacy of the proportions for strength of concrete or as a basis for acceptance of concrete shall be made and cured in accordance with methods and curing, concrete compression and flexure test specimens in the field (ASTM C 31 or equal). Strength tests shall be made in accordance with the method of test for compressive strength of moulded concrete cylinders (ASTM C 39 or equal).
Six Cylinders would form a set of sample for strength determination. Three Cylinders shall be tested at seven days and three cylinders shall be tested at twenty-eight days. Each and every twenty-eight days Cylinders shall attain the minimum specified compressive strength. The Contractor shall perform trial mix of his own to determine the characteristic strength or mean strength that has to be attained.

The twenty-eight days strength tests shall be used as a basis for acceptance of the concrete. Seven days tests are made to obtain advance information on the adequacy of strength development. Age-strength relationships shall be pre-established for the materials and proportion used.

**Testing geo-textiles**

Tests of mass per unit area, thickness and tensile strength in accordance with the Standards listed under the Section on ‘Construction Materials’ shall be carried out by an approved testing laboratory on samples taken from each quantity of 10,000 m² of geo-textile fabric supplied. The k and O₉₀ values shall be tested on samples taken from every 50,000 m² of geo-textile fabric supplied. Seams shall be tested for tensile strength every 10,000 m of seam.

The geo-technical test results of the underlying embankment soil together with the manufacturer’s specification and installation instructions for the proposed cloth, including permeability and porosity (with methods of testing) and a sample of the cloth shall be submitted for the approval of the Engineer.

The sample size for the fabric shall be 2 square meter and shall be marked to indicate its upper side, longitudinal and transverse directions, type of geo-textile and the date that the sample was taken. Seam samples shall be at least 1m in length and the ends of the threads are to be firmly tied by the Contractor or Supplier at the time the samples are taken. Each test shall be carried out on at least five samples.

The Contractor shall bear the expenses of all routine tests. Notwithstanding the submission of reports to the effect that the geo-textile conforms to the Specifications, the Engineer shall at all times be entitled to have additional samples of geo-textile tested, if he is of the opinion that the geo-textile does not conform to the Specifications. The Engineer shall only select samples from ends of geo-textile rolls or geo-textile, which has been cut already.

A geo-textile will be regarded as defective, if any of the specified values is not achieved other than those of unit weight and effective opening size, for which the following tolerances will be permitted:

(a) **Single layered geo-textiles:**

<table>
<thead>
<tr>
<th>Property</th>
<th>Tolerance</th>
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</thead>
<tbody>
<tr>
<td>Unit weight</td>
<td>minus 10%</td>
</tr>
<tr>
<td>O₉₀</td>
<td>plus or minus 20%</td>
</tr>
</tbody>
</table>

(b) **Composite geo-textiles:**

<table>
<thead>
<tr>
<th>Property</th>
<th>Tolerance</th>
</tr>
</thead>
<tbody>
<tr>
<td>Total weight</td>
<td>minus 15%</td>
</tr>
<tr>
<td>Single layer weight</td>
<td>minus 20%</td>
</tr>
<tr>
<td>O₉₀</td>
<td>plus or minus 20%</td>
</tr>
</tbody>
</table>

### 3.3 Expenses for tests

All expenses for the tests as stated in the above Sub-sections would be borne by the Contractor unless otherwise provisions are made in the Tender Documents.

Any tests instructed by the Engineer both in type and quantity beyond those specified above shall be paid to the Contractor, if not specific instructions are there under the concerned items of the Tender Documents.
SECTION-4
OFFICE SPACE AND FACILITIES
FOR THE ENGINEER
## OFFICE SPACE AND FACILITIES FOR THE ENGINEER

**CONTENTS**

### SECTION-4

<p>| | | |</p>
<table>
<thead>
<tr>
<th></th>
<th></th>
<th></th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>4.1</strong></td>
<td>Field Office</td>
<td><strong>35</strong></td>
</tr>
<tr>
<td><strong>4.2</strong></td>
<td>Office equipment and stationary articles</td>
<td><strong>36</strong></td>
</tr>
<tr>
<td><strong>4.3</strong></td>
<td>Survey equipment</td>
<td><strong>36</strong></td>
</tr>
<tr>
<td><strong>4.4</strong></td>
<td>Offices and equipment</td>
<td><strong>36-37</strong></td>
</tr>
<tr>
<td><strong>4.5</strong></td>
<td>Signboards</td>
<td><strong>37</strong></td>
</tr>
<tr>
<td><strong>4.6</strong></td>
<td>Progress in photographs and videos</td>
<td><strong>37</strong></td>
</tr>
<tr>
<td><strong>4.7</strong></td>
<td>Measurement and payment</td>
<td><strong>37-38</strong></td>
</tr>
</tbody>
</table>
SECTION-4
OFFICE SPACE AND FACILITIES FOR THE ENGINEER

4.1 Field Office

In addition to the office space required for his own use, the Contractor shall provide and maintain Field Office with toilet facilities, furniture and office equipment for the use of the Engineer and his staff.

Field Office for the Engineer shall mean a building having a minimum 15 square meter net clear internal floor area exclusive of walls and partitions, staircase and toilet and have number of rooms as required by the Engineer. It shall be constructed in 250mm thick brick wall in appropriate cement mortar with C.I. sheet roofing and a protective ceiling made of hard board and timber to the satisfaction of the Engineer. The floor shall be 75mm thick lean concrete with 30mm thick mortar on the top with a neat cement finish to give a smooth look. The foundation of this building shall be sound to the satisfaction of the Engineer. The building shall have required number of doors and windows. Uninterrupted power supply facility, if necessary, shall be made available by means of arranging a stand-by generator.

Access road to the Field Office, sufficient parking accommodation and hard standing sheds for vehicles along with boundary fencing shall be constructed by the Contractor.

The Contractor shall provide, for each office, one office table and four chairs of standard, approved by the Engineer. Safety helmets in adequate numbers be always made available for use of the staff and the visitors.

Offices shall be maintained watertight and shall be provided with ventilation. All doors shall be fitted with approved locks. Windows shall be provided with separate screens and blinds and shall have interior locking devices too.

All offices, complete with furnishings, fittings, access roads and hard standings, shall be ready, for occupation by the Engineer within four weeks of the date when the Contractor first occupies the Site.

All offices shall be regularly and properly cleaned as long as they are in use.

All access roads and hard standings shall be maintained in a convenient trafficable condition throughout the Contract period.

The general location of the Field Office shall be decided by the Engineer in consideration of the Contractor’s Work Plans. The Field Office shall be situated at locations that shall be free from flooding.

The Contractor shall submit for the approval of the Engineer, along with the Tender, Plans and Drawings showing the details for the building including plans and designs for foundations, access roads, sheds, etc. Plans shall also be submitted showing architectural and structural details and the proposed layout of electrical and running water supply, roads and hard standings thereto. The Engineer may require revision of the said plan prior to the approval for construction.

Prior to the occupation of the office, the Engineer may specify to the Contractor the defects in the work whereupon he may occupy the office and withhold payment for the work in this item until the Contractor remedies and makes good the said defects to the satisfaction of the Engineer.

On completion of the Contract the Field Office including furnishings shall become the property of the Employer.
4.2 Office equipment and stationary articles

The Contractor shall require to purchase and supply the following Office equipment and consumables to the Engineer:

(i) One Computer (English) of approved brand with printer and Auto CAD facilities

(ii) One Photocopy Machine (A3 size)

(iii) Minor items of field office equipment such as file trays, punches, staplers etc. in reasonable number/quantities as requested by the Engineer.

(iv) Consumables such as papers, pens, files etc. in reasonable number/quantities as requested from time to time by the Engineer.

Upon completion of the Contract, the office equipment listed above shall remain the property of the Employer.

4.3 Survey equipment

As per requirement of the program, survey equipment shall be provided on each contract Site for use by the staff of the Contractor and the Engineer. A tentative list of such survey equipment is given below:

Optical square 1 no.
Spirit level (metal 1m long) 1 no.
Steel measuring tape 25m long 1 no.
Steel measuring tape 5m long 1 no.
Leveling staff 3m long 1 no.
Ranging poles 5 nos.
Surveyor's plumb bob 1 no.
Wild T-1A Theodolite with tripod (or equivalent) 1 no.
Wild NA-2 Automatic level with tripod (or equivalent) 1 no.
Traversing targets with tripods 1 no.
Magnetic Compass 1 no.

Miscellaneous tools and minor items of survey equipment such as umbrellas, hammers, knives etc. shall be made available at Site in reasonable numbers at all times for use by the staff of the Contractor and the Engineer.

Consumables such as pegs, stakes, string lines, paint, marking crayons, etc., shall be made available at Site in reasonable numbers and quantities at all times for use by the staff of the Contractor and the Engineer.

Upon completion of the Contract, the survey equipment listed above shall remain the property of the Contractor.

4.4 Offices and equipment

The Contractor shall provide and maintain an inventory of all furnishings and equipment and shall replace any equipment, which is lost or irreparably damaged subject to the condition that the Engineer shall ensure his staff to take all reasonable precautions in the handling, operation and transportation of such equipment.
The Contractor shall pay all expenses in respect of water, electricity (where available), garbage cleaning etc. necessary for running the Office and maintaining conducive environment.

The Contractor shall place all necessary support staff such as office boys, cleaners, messengers, road-men, chain-men etc. in required number to the Engineer and his personnel in smooth performing of his responsibilities.

4.5 Signboards

The Contractor shall supply, erect and maintain in good condition at least two Identification Signboards of sizes to be specified by the Engineer to be fixed one at each end of the Work at a place clearly visible to the public. The Signboards shall be mounted on steel pipe frames with the required sizes at a height 2m above the ground and shall be sufficiently strong to withstand the wind forces. The board shall be fabricated from steel angle and plates and painted with suitable colours and written in English and/or Bengali as per direction of the Engineer.

Each board shall display:

- The name of the Project
- The name of the Work
- The name of the Employer
- Contract value
- Date of commencement of work
- Date of completion of work
- Other particulars, which will be asked by the Engineer.

4.6 Progress in photographs and videos

Photographs and videos showing the progress of works and special photographs showing particular features or other matters of interest in connection with the Work or their surroundings shall be taken every month by an approved qualified photographer/cameraman to the choice of the Engineer. Number of photographs/video clips will not exceed 10 (ten) per month.

Four colour un-mounted prints of a size 250mm on approved photographic paper of every such photograph inscribed with its serial number, date of shooting and a short title shall be furnished to the Engineer every month.

All negatives and video clips shall be numbered, filed and retained at the Site. On completion of the Contract, those shall become the properties of the Employer and shall be handed over to the Employer by the Contractor.

6 (six) complete sets of colour prints of the finished permanent Work, not exceeding 20 (twenty) photographs in number, shall be taken when and as directed by the Engineer prior to finally granting the Contractor the Certificate of Completion and shall be suitably mounted, titled and supplied to the Engineer.

4.7 Measurement and payment

Provisions for Office space and facilities for the Engineer shall not be measured.

Payment for all the items as stated below shall be for the full period of the Contract including any extension, if allowed.
Payment for all equipment, signboards, photographs, video clips, services etc. of the Field Office detailed in this Sub-section shall be made as described below, where price and payment shall be the full compensation for complying with this Section of the Specification and the Conditions of the Contract.

Payment of rates for the pay items shall be the full compensation for supplying, erecting and maintaining the Field Office for the Engineer including all furniture, fixtures and fittings, access roads, office equipment, signboards, photographs, video clips etc. all in full compliance with the requirements of this Section.

No separate payment shall be made to the Contractor for providing the requisite tools, minor items and the consumables. Compensation for these items shall be deemed to be included in the other pay items of the BOQ.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Supply, erection and maintenance of Signboards</td>
<td>Lump sum</td>
</tr>
<tr>
<td>Providing, erection and maintenance of Office for the Engineer including all office equipment and consumables</td>
<td>Lump sum</td>
</tr>
<tr>
<td>Providing photographs</td>
<td>Lump sum</td>
</tr>
</tbody>
</table>
SECTION-5

SITE PREPARATION & REMOVAL
OF EXISTING STRUCTURE
# SITE PREPARATION & REMOVAL OF EXISTING STRUCTURE

## CONTENTS

### SECTION-5

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>5.1</td>
<td>Site preparation</td>
<td>39-40</td>
</tr>
<tr>
<td>5.1.1</td>
<td>Description</td>
<td>39</td>
</tr>
<tr>
<td>5.1.2</td>
<td>Commencement</td>
<td>39</td>
</tr>
<tr>
<td>5.1.3</td>
<td>Drawings</td>
<td>39</td>
</tr>
<tr>
<td>5.1.4</td>
<td>Setting Out</td>
<td>39</td>
</tr>
<tr>
<td>5.1.5</td>
<td>Earthworks, general</td>
<td>39</td>
</tr>
<tr>
<td>5.1.6</td>
<td>Clearing of Site</td>
<td>39</td>
</tr>
<tr>
<td>5.1.7</td>
<td>Measurement</td>
<td>40</td>
</tr>
<tr>
<td>5.1.8</td>
<td>Payment</td>
<td>40</td>
</tr>
<tr>
<td>5.2</td>
<td>Demolition and removal of existing structure</td>
<td>41-44</td>
</tr>
<tr>
<td>5.2.1</td>
<td>Description</td>
<td>41</td>
</tr>
<tr>
<td>5.2.2</td>
<td>Planning</td>
<td>41</td>
</tr>
<tr>
<td>5.2.3</td>
<td>Precautions and protection</td>
<td>41-42</td>
</tr>
<tr>
<td></td>
<td>Protection of adjoining properties</td>
<td>41</td>
</tr>
<tr>
<td></td>
<td>Protection of public</td>
<td>41</td>
</tr>
<tr>
<td></td>
<td>Precautions prior to demolition</td>
<td>41-42</td>
</tr>
<tr>
<td></td>
<td>Precautions during demolition</td>
<td>42</td>
</tr>
<tr>
<td>5.2.4</td>
<td>Performance methods</td>
<td>42-43</td>
</tr>
<tr>
<td></td>
<td>Sequence of demolition operation</td>
<td>42-43</td>
</tr>
<tr>
<td>5.2.5</td>
<td>Disposal of demolished materials</td>
<td>43</td>
</tr>
<tr>
<td>5.2.6</td>
<td>Materials</td>
<td>43</td>
</tr>
<tr>
<td>5.2.7</td>
<td>Measurement</td>
<td>44</td>
</tr>
<tr>
<td>5.2.8</td>
<td>Payment</td>
<td>44</td>
</tr>
</tbody>
</table>
SECTION-5
SITE PREPARATION & REMOVAL OF EXISTING STRUCTURE

5.1 Site preparation

5.1.1 Description
This item of work shall consist of clearing the Site, undertaking general type of earthworks, setting out, etc. as shown on the Drawings, stated in the BOQ and/or as instructed by the Engineer.

5.1.2 Commencement
The Contractor shall give the Engineer at least 7 (seven) days written notice of his intention to commence work on any part of the Site. Works shall not be commenced until written approval has been received by the Contractor from the Engineer.

5.1.3 Drawings
The works are to be carried out in accordance with the Drawings and as directed by the Engineer. It may become necessary or desirable, during the progress of the Work, to change any feature shown on the Drawings in accordance with the actual field conditions. Whenever this may occur, the Contractor shall perform the required works to the revised dimensions in accordance with the written instructions of the Engineer.

5.1.4 Setting out
Prior to the commencement of the Work, the Contractor shall study the Drawings and fully understand all aspects of the Work and co-relate the same with the dimensions shown on the Structural Drawings and shall fix up the alignment, set the Bench Mark (B.M) pillars, levels, pegs etc.

The Contractor shall check all the vital measurements of the layout plan of the building and submit a report to the Engineer the deviation, if required any from the dimensions shown in the approved Drawings for the building before starting construction works. In case of any deviation of unacceptable amount, the Engineer will inform the Contractor of the remedial measures, which may be necessary under a particular situation.

Cutting or filling charts, prepared by the Engineer, will be given to the Contractor to sign as a token of his agreement.

5.1.5 Earthworks, general
Earthwork shall be undertaken to the lines and levels shown on the Drawings unless directed otherwise by the Engineer. In carrying out the earthworks, the Contractor shall take all necessary precautions to avoid damage to or deterioration of the earthwork materials and existing ground.

5.1.6 Clearing of Site
The Site shall be cleared as required to remove all stumps, roots, vegetable and other objectionable materials specifically within the areas for construction of structure, appurtenance and any other facilities indicated on the Drawings or designated by the Engineer. The cleared materials shall be deposited on the approved off-Site areas or burnt as directed by the Engineer.
5.1.7 Measurement

The works on Site preparation shall not be measured.

5.1.8 Payment

No direct payment shall be made for works required under this Section. Costs for such works shall be deemed included in the related items of the BOQ.
5.2 Demolition and removal of existing structure

5.2.1 Description
This item of work shall consist of satisfactory dismantling, removal and disposal of salvage, wholly or in part, of all existing structures and sub-structures within the Site as indicated on the Drawings or in the BOQ and/or as instructed by the Engineer. This item of Work shall also include taking all safety and precautionary measures so as to protect the adjoining properties and the public from any possible accidents.

5.2.2 Planning
The Contractor shall engage one experienced Engineer who shall carry out a detailed survey work and a study of the structures to be demolished and the structures in their surroundings before commencing the demolition work. He shall then plan the sequence of operations. The plan shall be approved by the Engineer before any works starts.

The Engineer shall notify the neighbours and the public well about the intended demolition through media. The extent, duration and time of the demolition shall be clearly stated in the notification.

5.2.3 Precautions and protection

Protection of adjoining properties
Notification shall be made in writing to the owner of each potentially affected plot, building or structure at least a week before the commencement of works.

The Contractor shall under all circumstances preserve and protect the adjoining plots, buildings or structures from any damages or injuries. This shall be done at his own expenses.

When damage to the adjoining property is imminent, the demolition operation shall be stopped forthwith and shall not be re-started until the necessary measures to prevent such damage have been taken.

Protection of public
The Contractor shall make safe distances and posts prominent signs. He shall either close or protect every sidewalk and road adjacent to the Site. All public roads shall be kept open and unobstructed at all times unless unavoidable circumstances arise.

If a covered walk is considered not necessary, the sidewalk shall be blocked and diversion roads or alternate protection shall be provided.

Precautions prior to demolition
The Contractor shall ensure taking the following precautionary measures prior to any demolition works starts:

- Demolition of any building shall not commence until the required pedestrian protection structures have been built. Dilapidated buildings or structures shall be protected from collapse by way of bracing, shoring, etc. before demolition starts.

- Danger sign shall be posted round the property. All entrances shall be barricaded or manned. Warning lights shall be placed above all barricades during night and dark hours. Watchmen shall be employed to prevent unauthorized entry of the public in the danger zone.
• All utility lines shall be disconnected upon the approval of the relevant Authorities. Temporary service connections for the demolition work shall be taken separately.

• The Site shall be thoroughly cleaned of all combustible materials. All materials of fragile in nature like glass, sanitary fixtures, etc. shall also be removed from the Site.

• Workmen shall be provided with all necessary safety appliances prior to the start of works. Safety precautions for fire shall be provided.

Precautions during demolition

The Contractor shall provide protection against all damages or loss to life and property during demolition. The Engineer shall depute an experienced and competent Representative to provide constant supervision during the entire period when the demolition works will be carried on.

The Contractor shall make all suitable arrangements to control dust. He shall ensure necessary stacking of all materials and debris in a way as have been stated in the relevant Section/Sub-sections of this Specification.

The demolition Site shall be provided with natural and artificial lighting and ventilation arrangements, which shall be the responsibilities of the Contractor.

The Contractor shall well protect all existing features required during demolition operations with substantial covering to the entire satisfaction of the rules and regulations of the undertakings or they shall be temporarily relocated.

For a building or structure of more than 8m or two storeys in height, all windows and exterior wall openings that are within 6m of floor opening and being used as the passage for debris, shall be solidly boarded. Openings in floors below the level of demolition, not used for removal of materials or debris, shall be barricaded or covered by planks.

5.2.4 Performance methods

Where a structure is to be replaced, the existing structure shall be demolished to a level up to the bottom of the foundation of the new structure or as directed by the Engineer.

Sequence of demolition operation

In case of a structure to be demolished becomes a building, the following steps and procedures shall be followed:

• The demolition shall proceed in descending order and storey by storey. All works in the upper floor shall be completed and approved by the Engineer prior to disturbing any supporting member on the lower floor. Demolition of the structure in sections may be permitted in exceptional cases only if necessary precautions are ensured.

• Walls shall be removed part by part in reasonably level courses. No wall or any part of the structure shall be left in a condition that may collapse or be toppled by wind, vibration etc.

Fall of the demolished wall in large chunks, which endangers the adjoining property or exceeds the safe load capacity of the floor below, shall be avoided. Debris shall be removed at frequent intervals to avoid piling up and overloading of any structural member.
Platforms shall be provided for demolition of walls less than one and half brick thick. Lateral bracing shall be provided for sections of walls having a height more than 22 times its thickness, or otherwise considered unsound. No workman shall stand on any wall to remove materials. Staging or scaffold shall be provided at a maximum of 3.5m below the top of the wall.

At the end of each day work, all walls shall be left stable to avoid any danger of getting overturned. Foundation walls, which serve as retaining walls shall not be demolished until the adjoining structure has been underpinned or braced and the earth removed.

- Support/centering shall be provided prior to removal of masonry or concrete floor. Planks of sufficient strength shall be used in shuttering. No person shall be allowed to work in an area underneath a floor being removed; such areas shall be barricaded.

The total area of a hole cut in any intermediate floor for dropping debris shall not exceed 25% of that floors area. No barricades or rails for guarding the floor hole shall be removed until the storey immediately above has been demolished down to the floor line and all debris cleared from the floor.

In cutting holes in a floor, which spans in one direction, a maximum 300mm wide slit shall be cut at first along the entire length of the slab. The slit shall be increased gradually thereafter.

- Use of explosives will not be permitted except directed by the Engineer. When explosives are required for blasting, requisite procedures are to be followed and adequate safety measures are to be insured.

- The Contractor shall have to arrange at the Site all plant, machinery and equipment at his own initiative together with making necessary techniques, arrangements and methods.

5.2.5 Disposal of demolished materials

Demolished materials shall be disposed off according to their salvage value.

If the salvaged superstructure or any portion of the structure is deemed fit for re-use elsewhere, the usable portions shall be marked and removed without any damages and stockpiled neatly at an accessible point above the highest water level within the Site or at a place as directed by the Engineer. All other usable materials shall be stockpiled within the Site.

The Contractor shall remove all materials, classed as waste and not considered of value by the Engineer, from the Site but within the limits of the Site at his own expenses. Debris shall be remove from the Site as soon as possible. Priority shall be given to dispose off combustible materials immediately. Materials like glass, nails shall not be strewn about. Standard precautions shall be taken to prevent fire from debris.

5.2.6 Materials

All materials so removed/salvaged shall be the property of the Employer unless otherwise specifically stated in the Contract. Any materials, not required by the Employer, shall be classed as waste.
5.2.7 Measurement

All activities as described above under this item of work are the responsibilities of the Contractor except otherwise stated.

The Work shall be measured according to the types of structures and shall be measured in cubic meter/ metric ton.

5.2.8 Payment

Demolition and removal of existing structures as measured above shall be paid at the Contract unit price per cubic meter or metric ton depending upon the type of works and the price quoted in the Contract. The payment shall be the full compensation for all dismantling, removal and disposal of all debris with carriage, stacking of usable materials, taking all safety measures, all allied items, all labour, equipment, tools and incidentals necessary to complete the work strictly in accordance with the stated Specifications.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Demolition and removal of existing structure</td>
<td>Cubic meter</td>
</tr>
<tr>
<td>(C/C, R.C.C, Brick work, Timber)</td>
<td></td>
</tr>
<tr>
<td>Demolition and removal of existing steel structure</td>
<td>Metric ton</td>
</tr>
</tbody>
</table>
SECTION-6
EXCAVATION AND BACK-FILL FOR STRUCTURES
# EXCAVATION AND BACK-FILL FOR STRUCTURES

## CONTENTS

### SECTION-6

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>6.1</td>
<td>Description</td>
<td>45</td>
</tr>
<tr>
<td>6.2</td>
<td>Materials</td>
<td>45-46</td>
</tr>
<tr>
<td></td>
<td>Excavated material</td>
<td>45</td>
</tr>
<tr>
<td></td>
<td>Ordinary fill</td>
<td>45</td>
</tr>
<tr>
<td></td>
<td>Sand</td>
<td>45</td>
</tr>
<tr>
<td></td>
<td>Blinding concrete</td>
<td>45-46</td>
</tr>
<tr>
<td></td>
<td>Cement</td>
<td>45-46</td>
</tr>
<tr>
<td></td>
<td>Coarse aggregate</td>
<td>46</td>
</tr>
<tr>
<td></td>
<td>Fine aggregate</td>
<td>46</td>
</tr>
<tr>
<td></td>
<td>Water</td>
<td>46</td>
</tr>
<tr>
<td>6.3</td>
<td>Construction methods</td>
<td>46-50</td>
</tr>
<tr>
<td></td>
<td>Excavation</td>
<td>46-47</td>
</tr>
<tr>
<td></td>
<td>Poor foundation material</td>
<td>47</td>
</tr>
<tr>
<td></td>
<td>Disposal of excavated material</td>
<td>47-48</td>
</tr>
<tr>
<td></td>
<td>Pumping and bailing</td>
<td>48</td>
</tr>
<tr>
<td></td>
<td>Back filling</td>
<td>48-49</td>
</tr>
<tr>
<td></td>
<td>Cofferdams</td>
<td>49-50</td>
</tr>
<tr>
<td>6.4</td>
<td>Measurement</td>
<td>50</td>
</tr>
<tr>
<td>6.5</td>
<td>Payment</td>
<td>50-51</td>
</tr>
</tbody>
</table>
SECTION-6
EXCAVATION AND BACK-FILL FOR STRUCTURES

6.1 Description

This item of work shall consist of excavation in any type of soil/material for the foundation of structures, disposal of excavated materials, construction and removal of cofferdams, sheeting and other temporary works in protecting the stability and safety of the excavated foundations, pumping, de-watering/bailing water from foundations, back-filling of completed structures with suitable back-fill.

No separate payment shall be made for the excavation and back-fill for structures when the works will involve use of cofferdams. The costs of this temporary work shall be deemed included as part of the Tender sum.

The Work shall be carried out at the locations and according to the lines, levels, grades and dimensions shown on the Drawings, stated in the BOQ and/or as directed by the Engineer.

6.2 Materials

Excavated material

The Engineer shall classify all excavated materials either as suitable for fill or as waste.

Approved suitable excavated materials free from large lumps, wood or other objectionable materials shall be placed as back-fill above the level of pile except where other materials are shown on the Drawings, stated in the BOQ and/or required by the Engineer.

Ordinary fill

Ordinary fill consists of earth having Liquid Limit not exceeding 50 (STP T3.2) and Plasticity Index not exceeding 20 (STP T3.2) and shall be used as back-fill material above the level of pile caps and areas except where other materials are shown on the Drawings, stated in the BOQ and/or required by the Engineer.

Sand

Unless otherwise stated on the Drawings or in the BOQ or ordered by the Engineer, back-fill material below the top level of pile caps shall consist of sand free from chemical contamination with not more than 10% of the material passing the No. 200 sieve (U.S. size). All other specifications should conform to what have been illustrated under the relevant Sub-section of this Specification. The sand to be used shall be approved by the Engineer prior to placing.

Blinding concrete

Blinding concrete shall be placed as backfill as shown on the Drawings, stated in the BOQ and/or ordered by the Engineer. The material shall conform to the specifications stated below:

Cement

Cement shall conform to the requirements of ASTM specification C 150 Type 1 or similar approved standard for normal Portland cement.
Cement shall be free from any hardened lumps and foreign matter. It shall have a minimum of 90% of particles by weight passing the 75 micron sieve, an initial setting time in excess of 45 minutes and a final setting time of not more than 375 minutes.

All other specifications should conform to what have been illustrated under the relevant Sub-sections of this Specification.

**Coarse aggregate**

Except otherwise stated, coarse aggregate shall consist of hard, durable angular fragments of crushed stone and/or crushed natural gravel conforming all other specifications illustrated under the relevant Sub-section of this Specification.

**Fine aggregate**

All specifications should conform to what have been illustrated under the relevant Sub-section of this Specification.

**Water**

Water shall be subject to the approval of the Engineer and shall be reasonably clear, free from oil, alkali, salts, acid and organic substances and other deleterious materials or objectionable quantities of suspended materials. All other specifications shall be in accordance with the requirements illustrated under the relevant Sub-section of this Specification.

### 6.3 Construction methods

**Excavation**

The Contractor shall notify the Engineer before commencing excavation of the foundation trenches so that the cross-section, elevations and measurements of the undisturbed ground may be taken. The natural ground adjacent to the structure shall not be disturbed without taking any permission from the Engineer.

Trenches and foundation pits for structures shall be excavated to the lines, grades and elevations as shown on the Drawings or as directed by the Engineer. The elevations of the bottom of the foundations shown on the Drawings are approximate only and the Engineer may order such changes as deemed necessary to provide a secured foundation.

Where unstable soil is encountered at the bed level, it should be brought to the notice of the Engineer and all such unstable soil shall be removed as directed and replaced with suitable materials to provide adequate support for the structure.

On acceptance of the materials forming the bottom of any excavation by the Engineer subsequently becoming unacceptable to him due to exposure to weather condition or due to flooding or have become puddled, soft or loose during the work process, the Contractor shall remove such damaged, soft, or loose materials and make additional excavation as per requirement. Such additional excavation shall be held as excess excavation and the cost of the excess excavation and subsequent replacement with a suitable back-fill shall be at the expenses of the Contractor.

Any erroneous excavation or excess excavation for the conveniences of the Contractor, or over excavation performed by the Contractor for any purpose or reasons shall be at the expenses of the Contractor. If the excavation for foundations exceeds the depths specified, the Contractor shall brought it
back to the specified levels with sand, mass concrete or other approved materials conforming Standard Specifications at the Contractor’s own expenses.

Excavation shall be sufficiently large to provide necessary working space, shuttering and any other Temporary Works required during construction.

Boulders, roots and any other objectionable materials encountered in excavation, shall be removed. The excavated foundation shall be cleared of all loose materials and cut to a firm surface.

When the footing is to rest on the ground and not on piles, special cares shall be taken not to disturb the bottom of the excavation and excavation to final grade shall be deferred until immediately before the footing is placed. If foundation fill material is required, it shall be placed and compacted in layers not more than 150mm thick or as directed by the Engineer. The dry density on compaction within 300mm below the top level shall not be less than 100% maximum dry density as determined in accordance with STP T4.5 (standard compaction).

In excavating foundation trenches, the last 150mm layer shall not be excavated until immediately before commencing the construction work except that the Engineer shall instruct otherwise. Any damages to the work due to the Contractor’s operation shall be repaired at the expenses of the Contractor.

The Contractor shall be solely responsible for the safety and stability of the excavation and shall provide all protective supports, bracing, sheet piles, shoring etc. as required. Shoring should be adequate to provide enough safety to all the adjacent structures and land.

Excavated materials, classified as suitable for fill, shall be stockpiled. Waste materials and suitable fill materials in excess of requirement, shall be disposed of by the Contractor outside the limits of the Site.

The foundation material shall be cleared of all loose and displaced materials and cut to a firm surface, either leveled, stepped or serrated, as specified or shown on the Drawing or directed by the Engineer leaving a smooth solid bed to receive foundation.

No footing, bedding material or structure shall be placed on any foundation until the Engineer has inspected and approved the depth of excavation and the foundation materials.

**Poor foundation material**

When, in the opinion of the Engineer, the bottom of any excavated foundation is of soft or otherwise unsuitable material, the Contractor shall remove the unsuitable material and fill with sand or blinding concrete at the direction of the Engineer. The sand or concrete shall be placed following the procedures specified for back-filling. Sand shall be clear, all passing a No.4 sieve (U.S. size).

When the ground between the piles is too soft to support the green concrete, the Contractor shall submit his proposal for a bottom form to the Engineer for his approval. Extra excavation and foundation-fill or concrete-fill in such case will not be paid separately.

If the bottom form is carried out by strengthening the ground in the aforementioned way, the Contractor shall, if requested, submit calculations showing that the pile cap will not be harmed during hardening due to differential settlement between the piles and the strengthened ground.

**Disposal of excavated material**

All excavated materials, so far accepted by the Engineer as suitable, shall be utilized as back-fill or embankment-fill. The surplus materials shall be termed as waste.
Excavated materials, suitable for use as back-fill, shall be deposited by the Contractor in spoil heaps at points convenient for re-handling of the materials during the back-filling operations.

Excavated materials shall be deposited in such places and in such a manner as not to cause damage to roads, services or properties either within or outside the project area and so as to cause no impediment to the drainage of the Site or surrounding areas. The location of spoil heaps shall be subject to the approval of the Engineer.

Waste materials shall be disposed of in accordance with the instruction of the Engineer.

**Pumping and bailing**

The foundation shall be kept free from water at all times during the construction period. The ground water level shall be maintained at a minimum of 0.9m below the lowest designed excavation level.

Pumping and bailing from any foundation shall be done so as to preclude the possibility of the movement of water through or alongside any concrete being placed. No pumping or bailing will be permitted during the placing of concrete and for at least 24 hours thereafter, unless it is done from a suitable sump separated from the concrete work by a watertight wall or from well points.

The Contractor shall be solely responsible and include in his rates all costs in designing the de-watering system, providing all equipment and accessories required for de-watering. The rates shall also include cost for transportation, furnishing, installation, safe operation and maintaining of the system including operators, mechanics, the supply of power, fuel, lubricants, spares, repairing, etc. throughout and the removal of the equipment at the end of the construction period under this Contract.

Excavations shall be as dry as possible prior to and during placing concrete. Placing of concrete under water will only be permitted if indicated on the Drawings or approved by the Engineer.

**Back-filling**

All excavated spaces shall be back-filled around the permanent structure to original ground level. Prior to placing back-fill, all trash, metal, debris, lumber, bricks, soft materials and similar objectionable foreign materials shall be removed from the area to be back-filled. No back-fill shall be placed against any structure without the prior permission of the Engineer.

Any protective support, bracing or shoring shall be removed, as the back-filling progresses in such a manner as to prevent caving-in.

Back-fill shall be of approved materials that will produce a dense and well-compacted filling. The material shall be free from large lumps, organic or extraneous materials.

Ordinary fill placed as back-fill shall be laid and compacted. The moisture content of the fill materials, before compaction, shall be within + 5% of the Optimum Moisture Content. Each layer of materials shall be compacted uniformly using approved compaction equipment and procedures. The materials shall be compacted to achieve not less than 90% Maximum Dry Density (STD) beneath the bottom level. The dry density, after compaction within 300mm below the top level, shall not be less than 95% Maximum Dry Density as determined in accordance with STP and soaked CBR (4 days) should be greater than 4% at 95% Maximum Dry Density. The compacted layer shall be approved by the Engineer before the Contractor can commence a new layer.
Sand back-fill shall be placed and thoroughly compacted in layers of not more than 150mm. Sand should be clear, all passing a No. 4 U.S. Standard Sieve and conforming generally to ASTM C 144 for fine aggregate with F.M. not less than 1.2 or as required by the Engineer.

Layers of filling shall be tested as directed by the Engineer. Each compacted layer shall not be covered until the Engineer is satisfied that the specified degree of compaction has been achieved.

In placing back-fill, the materials shall be placed in, as far as possible, to approximately the same height on each side of the structure. If conditions require appreciable higher back-filling on one side, the additional materials shall not be placed until permission is given by the Engineer on being satisfied by himself that the structure has enough strength to withstand any created pressure.

In general, no structure shall be subject to the pressure of back-filling until 3 (three) days on expiry of the period designated for removal of forms. This period shall be extended if abnormal curing conditions exist.

Adequate provisions shall be made for drainage during placing back-fill.

**Cofferdam**

The term “cofferdam” denotes any temporary or removable structure, constructed to hold the surrounding earth, water or both, out of the excavation whether such structure is constructed of earth, timber, steel, concrete or any combination of these. The term includes earth dikes, timber cribs, sheet piling, removable steel shells and all bracings and it shall be understood to include excavation enclosed by pumping wells and well points.

Cofferdams shall be constructed so as to control water to preclude sliding and caving-in of the walls of the excavation.

The interior dimensions of cofferdams shall be such as to give sufficient clearance for the construction and removal of any required forms and the inspection of the interior and to permit pumping.

If possible, cofferdams shall be so designed that no cross bracing shall be left in place. If this is not possible, bracing left in place shall be of structural steel. The end of such structural members that would be exposed when the structure is completed shall be boxed back at least 50mm behind the face. The resulting holes shall be completely filled with concrete.

In general, sheet-piling cofferdams shall extend well below the bottom of the footings and shall be well braced and made maximum watertight.

When conditions are encountered which, in the opinion of the Engineer, render it impossible to de-water the foundation before placing of brickwork or concrete, the Engineer may require the construction of a concrete foundation or seal. This shall be placed as directed by the Engineer. The foundation shall then be de-watered and the footing placed.

When foundation piles are to be driven inside a cofferdam and it is judged impossible to de-water the cofferdam before placing concrete, the excavation may be extended below the design level to a depth sufficient to allow for swell of the materials during pile driving operations. Any materials that rise above the design level shall be removed.

Where it is possible to de-water the cofferdam, the foundation materials shall be removed to exact grade after the foundation piles are driven.
The natural streambed adjacent to the cofferdam shall not be disturbed without the permission of the Engineer. Any excavation adjacent to the cofferdam shall be back-filled to the original ground level to the satisfaction of the Engineer.

Unless otherwise provided, cofferdams shall be removed on completion of the structure without disturbing or marring the finished work. The Engineer may order the Contractor to leave any part or the whole of the cofferdam in place and this shall not entitle the Contractor to claim for any additional payments.

The Contractor shall submit Drawings showing his proposed methods of cofferdam construction. However, the Contractor shall remain fully responsible for the adequacy of the design for strength and stability and for the safety of the people working therein.

6.4 Measurement

The volume of excavation and back-fill shall be measured in cubic meter.

The quantity of excavation for structures to be measured for payment shall include excavation for all structures.

Back-filling with previously excavated materials shall not be measured or paid for separately but shall be deemed included within the rate for excavation.

Volumes to be excavated for blinding concrete shall not be measured and the price for the excavation thereof shall be included in the above measured item for excavation and back-fill.

Back-fill with concrete or sand, where directed by the Engineer, including concrete seals shall be measured separately as the volume within the plan outline and top and bottom surfaces. Concrete or sand, placed to back-fill excavation beyond the excavation required, will not be measured for payment.

If sand fill is ordered over top level of pile cap, the fill shall be the specified filling volume measured on the Drawings up to the profiles agreed upon in writing by the Engineer.

Removal of cofferdams, slides, silting or filling, if required, shall neither be measured nor paid for.

6.5 Payment

The work measured shall be paid for at the Contract unit prices per cubic meter as shown in the Bill of Quantities. The payment shall be the full compensation for all excavations and back-filling for structures including supply of all materials, labour, equipment, tools and incidentals necessary to the successful completion of the work. The payment shall also be the full compensation for excavation and subsequent back-filling of working space around the foundation structure for shoring and other protective supports, for construction and removal of cofferdams, for de-watering and for disposal of surplus excavated materials by hauling to any distance at approved locations.

Should it be necessary, in the opinion of the Engineer, to lower the footings to an elevation below the level shown on the Drawings, payment for the excavation and backfill for structures required below plan level down to and including an elevation 1.5m below plan level for any individual footing will be made at a unit price equal to 115% of the Contract unit price and payment for the excavation from an elevation greater than 1.5m below plan level down to and including an elevation 3m below plan level will be made at a unit price equal to 125% of the Contract unit price for “Excavation and Back-filling for Structures”. No additional extra compensation will be allowed for any required cofferdam adjustments arising from such lowering of footings.
In case where the extra depth required for any footing or footings exceeds 3m, a supplementary agreement shall be made covering the quantities recovered from depths in excess of 3m below the plan grade.

Payment for Back-filling shall be included in the pay item for “Excavation and Back fill for Structures” except for sand fill and concrete fill. These fill types shall be measured as provided above and paid for at the concerned Contract unit prices. However, no compensation shall be made for less Back-filling with excavated materials or more surplus to waste in the pay item of “Excavation and Back-filling for Structures”.

All payments for the Back-filling and compaction of those areas, which were removed as structural excavation shall be included in the appropriate unit rates as shown below.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Excavation and back-fill for structures</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Concrete back-fill for structures</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Sand back-fill for structures</td>
<td>Cubic Meter</td>
</tr>
</tbody>
</table>
SECTION-7
DE-WATERING SYSTEM
<table>
<thead>
<tr>
<th>SECTION-7</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>7.1</td>
<td>Description of de-watering system</td>
<td>52</td>
</tr>
<tr>
<td>7.2</td>
<td>Types of de-watering system</td>
<td>52</td>
</tr>
<tr>
<td>7.3</td>
<td>Contractor's responsibilities</td>
<td>52</td>
</tr>
<tr>
<td>7.4</td>
<td>Site information</td>
<td>53</td>
</tr>
<tr>
<td>7.5</td>
<td>De-watering by sub-surface water control system</td>
<td>53-54</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>53</td>
</tr>
<tr>
<td></td>
<td>Precautionary measures</td>
<td>53</td>
</tr>
<tr>
<td></td>
<td>Operation</td>
<td>53</td>
</tr>
<tr>
<td></td>
<td>Monitoring wells</td>
<td>53-54</td>
</tr>
<tr>
<td></td>
<td>Removal of systems</td>
<td>54</td>
</tr>
<tr>
<td>7.6</td>
<td>De-watering by surface water control system</td>
<td>54</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>54</td>
</tr>
<tr>
<td></td>
<td>Operation of de-watering system</td>
<td>54</td>
</tr>
<tr>
<td></td>
<td>Removal of system</td>
<td>54</td>
</tr>
<tr>
<td>7.7</td>
<td>Measurement</td>
<td>54</td>
</tr>
<tr>
<td>7.8</td>
<td>Payment</td>
<td>54</td>
</tr>
</tbody>
</table>
SECTION-7
DE-WATERING SYSTEM

7.1 Description
This item of work shall consist of draining out of surface water and/or controlling water accumulated from any source that may require the use of pumps, other mechanical devices or the use of well point or tube-well system. All works to be carried out as per the stipulation of the BOQ and/or as instructed by the Engineer.

7.2 Types of de-watering system
One or both of the following de-watering systems shall be adopted considering the actual field conditions and requirements for proper execution of work.

- De-watering by Sub-surface Water Control System
- De-watering by Surface Water Control System

7.3 Contractor's responsibilities
The Contractor shall be solely responsible and include in his rates for the following tasks:

- The design of the de-watering system including the collection of the requisite data, preparation of Plans and Drawings of the necessary de-watering system.
- Providing all equipment and accessories required for de-watering by the Surface Water Control System and Sub-surface Water Control System for satisfactory execution of the work.
- Transportation, furnishing, installation, safe operation and maintaining of the system including operators, mechanics, supply of power, fuel, lubricants, spares, repairing, etc. throughout and the removal of the equipment at the end of the construction period under this Contract.

The Contractor shall provide continuous supervision of the system by the persons competent to recognize adverse conditions as they develop and take immediate corrective measures. The supervisor whose name and hours of duty duly furnished to the Engineer by the Contractor, shall have thorough knowledge of the system including the ability to make minor emergency repairs.

The control of water throughout the time of this Contract shall be the full responsibilities of the Contractor. The ground water table shall be maintained at minimum of 0.9m below the lowest designed excavation level. Control methods shall be subject to the approval of the Engineer including the Contractor's equipment, plans, methods, installation and operation procedures, etc.

The control methods adopted by the Contractor shall be subject to the approval of the Engineer including equipment, plans, methods, installation, operation, monitoring, maintenance procedures and precautions against the failure of any part of the system. The precautions shall include sufficient standby pumping plant and essential spare parts. The standby pumping plant shall comprise at least one pump and the standby pumping capacity shall be at least 10% of the total working capacity.
7.4 Site information

Any sub-soil investigation conducted by the LGED will be made available for the Contractor's review. The LGED assumes no responsibility regarding the correctness of these data. It is the responsibility of the Contractor to verify all sub-surface conditions prior to submitting his tender.

7.5 De-watering by sub-surface water control system

General

De-watering by Sub-surface Water Control System is defined as controlling water accumulated from any source requiring the use of well point or tube-well system.

Works to be performed under this Section include furnishing, installing, maintaining, operating and removing the sub-surface water control system including observation wells, so that the required excavation can be safely and properly performed and the structure built and back-filled to the elevation as shown on the Drawings.

Precautionary measures

Excavation shall not be made below a level 1m above the ground water level shown to exist by the water level in the observation wells. If the distance to the ground water table becomes less than 1m or the Engineer has any reason to believe that rising ground water is likely to endanger either the open excavation or the structure, back-filling may be ordered by the Engineer as a precaution against failure.

If for any reason, ground water control is lost and ground water appears in any portion of the excavation, the Contractor shall take immediate action to control and confine the flow. Any portion of the final grade which, in the opinion of the Engineer, has been damaged by the action of the ground water, shall be excavated as directed by the Engineer and back-filled in accordance with the Specifications at no extra cost to the Contract.

If it becomes necessary for any reason to stop the sub-surface de-watering operations before the construction of sub-structure is complete, the Engineer may order the Site to be flooded up to the surrounding ground water level as de-watering is discontinued. Under no circumstance shall the Site be flooded by allowing the ground water to rise through the soil. If it becomes necessary to flood the Site as described above, all equipment that can be damaged shall be removed to safety/a safe place.

The cost of all such back-filling, flooding and subsequent draining and re-excavation shall be included in the lump sum price for de-watering and no extra payment beyond the Contract price will be allowed.

Operation

The sub-surface De-watering System shall be operated 24 hours of a day on all days of a week during the period that de-watering is required. The Contractor shall take prior precautions against failure of any part of the system.

Monitoring wells

Observation wells of 40mm diameter G.I. pipes with 1.25m long wire mesh strainer and full filters shall be installed by the Contractor to suitably monitor the ground water levels maintained by the Contractor’s de-watering system. The depth of wells shall be a minimum of 3m below the lowest level of the foundation excavation. The Contractor shall provide a means for locking the access to the observation wells and shall maintain a log book with daily readings of sub-soil water levels recorded every three hours, which shall be
made available at all times for inspection. The logbook shall be periodically checked and authenticated by the Engineer’s Representative.

**Removal of system**

The de-watering system shall be removed when the construction has progressed to a stage that Site de-watering is no longer required; but only after receiving the written permission from the Engineer. Certain portions of the Contractor’s de-watering system may be left in the ground when construction procedures will so require and when written permission of the Engineer is obtained. Any such portion of the de-watering system shall be plugged, capped and/or otherwise rendered harmless to the Work and the public.

**7.6 De-watering by surface water control system**

**General**

Evacuation of surface water is defined as draining out surface water by use of pumps, sump pump, gravel drain or other mechanical devices, but without requiring the use of a well point or tube-well system. Such water may accumulate from percolation, rain or pumping floodwater in to the area or any other source or combination of sources.

Work to be performed under this Sub-section include furnishing, installing, maintaining, operating and removal of the surface water draining system for de-watering the accumulated water from the area so that the desired construction can safely and properly be performed. The discharge line or the drainage system for the disposal of the evacuated water shall be constructed by the Contractor at his own costs in accordance with the approved Drawing and by arranging private lands, if needed any.

**Operation of de-watering system**

The Contractor shall make all arrangements for pumps, fuel, lubricants, maintenance and operation of the equipment and the whole Surface De-watering System and shall take precautions in advance against failure of any part of the system.

**Removal of system**

The Surface De-watering System shall be removed upon obtaining written permission from the Engineer when the construction has progressed at a stage that Site de-watering is no longer required.

**7.7 Measurement**

The work shall be measured for payment as an item on a lump sum basis as specified in the BOQ.

**7.8 Payment**

Payment shall only be admissible on implementation of the item of the BOQ and on the basis of the Engineer certifying that the work was necessary and implemented for the proper execution of construction work satisfying all Specifications described above. Payment shall be made at Lump Sum rate as quoted in the Contract. The rate shall cover the full compensation for all measures including the cost of labour, equipment, materials, tools required for this purpose and other incidentals necessary to complete this item of work strictly in accordance with the Specifications stated above and/or as accepted by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Pumping and bailing out water/de-watering of work Site</td>
<td>Lump sum</td>
</tr>
</tbody>
</table>
SECTION-8
EARTH FILLING AND SAND FILLING
# EARTH FILLING AND SAND FILLING

## CONTENTS

### SECTION-8

<table>
<thead>
<tr>
<th>Section</th>
<th>Title</th>
<th>Pages</th>
</tr>
</thead>
<tbody>
<tr>
<td>8.1</td>
<td>Earth filling</td>
<td>55-59</td>
</tr>
<tr>
<td>8.1.1</td>
<td>Description</td>
<td>55</td>
</tr>
<tr>
<td>8.1.2</td>
<td>Materials</td>
<td>55</td>
</tr>
<tr>
<td>8.1.3</td>
<td>Construction methods</td>
<td>55-58</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>55-56</td>
</tr>
<tr>
<td></td>
<td>Clearing</td>
<td>56</td>
</tr>
<tr>
<td></td>
<td>Grubbing</td>
<td>56</td>
</tr>
<tr>
<td></td>
<td>Ownership of cleared materials</td>
<td>56</td>
</tr>
<tr>
<td></td>
<td>Spreading and compaction of earth fill</td>
<td>56-57</td>
</tr>
<tr>
<td></td>
<td>Manually compacted fill</td>
<td>57</td>
</tr>
<tr>
<td></td>
<td>Procedures for manual compaction</td>
<td>57</td>
</tr>
<tr>
<td></td>
<td>Mechanical compaction</td>
<td>58</td>
</tr>
<tr>
<td>8.1.4</td>
<td>Measurement</td>
<td>58</td>
</tr>
<tr>
<td>8.1.5</td>
<td>Payment</td>
<td>58-59</td>
</tr>
<tr>
<td>8.2</td>
<td>Sand Filling</td>
<td>60-62</td>
</tr>
<tr>
<td>8.2.1</td>
<td>Description</td>
<td>60</td>
</tr>
<tr>
<td>8.2.2</td>
<td>Materials</td>
<td>60</td>
</tr>
<tr>
<td>8.2.3</td>
<td>Construction methods</td>
<td>60-62</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>60</td>
</tr>
<tr>
<td></td>
<td>Spreading and compaction of sand fill</td>
<td>60-61</td>
</tr>
<tr>
<td></td>
<td>Procedure for manual compaction</td>
<td>61</td>
</tr>
<tr>
<td></td>
<td>Mechanical compaction</td>
<td>61-62</td>
</tr>
<tr>
<td>8.2.4</td>
<td>Measurement</td>
<td>62</td>
</tr>
<tr>
<td>8.2.5</td>
<td>Payment</td>
<td>62</td>
</tr>
</tbody>
</table>
SECTION-8
EARTH FILLING AND SAND FILLING

8.1 Earth filling

8.1.1 Description

This work shall consist of filling any place by furnishing, placing, compacting and shaping suitable earth material of acceptable quality obtained from approved sources to make up levels to the lines, levels, grades, dimensions and cross sections in accordance with these specifications and as shown on the Drawings and/or as instructed by the Engineer.

8.1.2 Materials

All fill materials shall be free from roots, sods or other deleterious materials.

All fill materials shall be stockpiled outside the working areas. Materials shall be tested and approved by the Engineer.

The selected fill so stockpiled, shall satisfy the following criteria:

- Liquid limit of fraction passing 425 micron sieve shall not exceed 50%
  } STP Sec. 3
- Plasticity index of fraction passing 425 micron sieve shall not exceed 20%
- The dry density after compaction in layers more than 300mm below top level shall not be less than 90% of the maximum dry density as determined in accordance with STP T4.5 (Standard Compaction).
- The dry density after compaction within 300mm below the top level (or such greater depth if shown on the plans and drawings) shall not be less than 95% maximum dry density as determined in accordance with STP T4.5 (Standard Compaction).
- Soaked (4 day) CBR greater than 4% at 95% MDD (STD).
- The moisture content at the time of compaction shall be the optimum moisture content (Standard Compaction) ± 5%.
- Sampling to be carried out as per STP Section 2 and Sub-section 4.1.

8.1.3 Construction methods

General

Prior to placing any fill upon any area, all clearing and grubbing operations shall be completed following the procedures stated below.

The original ground surface should be prepared by scarifying, watering, aerating and compacting. The dry density after compaction shall not be less than 90% of MDD (STD).

Filling in swamps or water shall be carried out as indicated on the Drawings and as described in these Specifications. The Contractor shall, when ordered by the Engineer, excavate or displace swampy
ground and backfill with suitable materials. Such backfill shall be river or beach sand unless otherwise
directed by the Engineer.

The materials that are borrowed from canals or other waterlogged areas for use as fill material, being
saturated, shall initially be stockpiled to drain the excess water before placing it in the designated areas.

**Clearing**

Clearing shall consist of the removal and disposal of everything above foundation level except those the
Engineer directs are to be left undisturbed. The materials to be cleared shall include but not necessarily
be limited to trees, stumps, logs, bush, undergrowth, grass, crops, loose vegetable matter and structures
unless provided elsewhere.

All tree stumps shall completely be removed within the limits of earthwork.

Clearing shall also include the removal of existing fences, remnants of buildings, etc.

**Grubbing**

Grubbing shall be confined to major roots beneath the excavations.

In agricultural areas where the ground has been formed into ridges of dikes, the ground shall be roughly
leveled or graded to form a surface suitable for filling and to the satisfaction of the Engineer.

**Ownership of cleared materials**

All cleared materials shall, unless otherwise provided for in the Contract, be the property of the
Department.

**Spreading and compaction of earth fill**

Earth carried from outside shall be placed on the land to be developed in horizontal layers and each
layer shall not exceed a loose thickness that is required to obtain a compacted thickness of 150mm. The
earth of each basket is to be placed near to the earth placed before it and spread systematically. The
Contractor shall not be allowed to throw earth in heaps.

The materials to be compacted shall be deposited in horizontal layers on the land to be developed with a
loose thickness as stated above. The clods of earth shall be broken down to a maximum size of 25mm
by striking the clods with the back of a spade or by using wooden drag or ladder or by any other suitable
means before the next basket of earth is thrown close to it. Distribution of materials shall be made in such
a way that the compacted materials will become homogeneous and free from lenses, pockets, streaks or
other imperfections. Excavating and placing operations shall be such that the materials, when
compacted, will be blended sufficiently to secure the best practicable degree of compaction,
impermeability and stability and for this purpose the preceding compacted layer shall be scarified before
placing a new layer.

All fill materials shall generally be compacted mechanically. However, under some special circumstance
and when specifically allowed under the BOQ, the fill may be allowed to be compacted manually.

If the density measurement checks fall below the specified density level, re-compacting shall be required
irrespective of the field compaction trial results. The Contractor shall be carried out such works
Earth fill materials, which do not contain sufficient moisture requirement for compaction in accordance with the requirements of this Sub-section shall be reworked and watered as per direction of the Engineer. The Contractor shall carry out this work at his own expenses.

Earth fill materials containing excess moisture shall be reworked and dried prior to or during compaction. Drying of wet materials shall be performed by methods proposed by the Contractor and approved by the Engineer at the expenses of the Contractor.

Compaction of every layer shall have to be approved by the Engineer. In the event the Contractor fails to obtain the approval of the Engineer of a fill layer, the materials above the unsatisfactory layer shall be removed and the unsatisfactory layer shall be re-compacted to satisfy the specifications at the expenses of the Contractor.

**Manually compacted fill**

Fill shall be placed and compacted in layers for 150mm maximum compacted thickness, uniformly spread and compacted over the fill area of each layer. If for any reason, progress in compaction of the fill is interrupted for any unreasonable time, the surface area of the fill shall be scarified or ploughed before compaction continues. Each layer shall be compacted, using controlled manual compaction methods to achieve at least 85% of the Standard Proctor maximum dry density.

Compaction of every layer shall have to be approved by the Engineer. In the event the Contractor fails to obtain the approval of the Engineer of a fill layer, the materials above the unsatisfactory layer shall be removed and the unsatisfactory layer shall be re-compacted to satisfy the specifications at the expenses of the Contractor.

Under special circumstances and if directed by the Engineer, the Contractor shall excavate 5 to 10 trial pits each of size 2m long, 1m wide and 2m depth or to a depth of the improved land (whichever is less) at random spacing to test the degree of compaction. The size of voids encountered shall not exceed 5 cm in diameter and the number of voids shall be less than 10 per square meter.

**Procedures for manual compaction**

The earth shall be compacted manually using concrete drop hammers each weighing 6 kg to 7 kg, fitted with a shaft of about 1.5m long. Ramming shall reduce the voids and shall continue until no further shrinkage of earth is possible by ramming.

Before commencing ramming, the moisture content of the soil shall be increased or decreased as per requirement by sprinkling the soil with water or by allowing natural drying of the soil as applicable so as to ensure that the materials shall have a moisture content of not less than 5% or greater than 5% dry of the optimum moisture required for the purpose of compaction. Both wetting and drying may be aided by furrowing the fill and then re-spreading when the moisture content is suitable.

If the moisture content exceeds the aforementioned tolerance, the compaction operations shall not proceed until the material is wetted or allowed to dry out, as the case may be to obtain optimum moisture content within the permitted tolerances. However, there may be an exception with a specific approval of the Engineer. No adjustment in price shall be made on account of any operations of the Contractor related to wetting or drying the materials or on account of any delays occasioned thereby.

The preceding operations shall continue layer after layer until the top of the filling is reached.
Mechanical compaction

In the case of mechanical compaction, area of development, designated on the Drawings or by the Engineer, shall be compacted to the lines and grades shown on the Drawings or established by the Engineer. The Contractor’s operations in importing materials, designated for use, shall be such as will result in an acceptable gradation of material when placed as determined by the Engineer.

Just prior to and during placement operations, the materials shall have a moisture content of not greater than 5% wet or less than 5% dry of the optimum moisture required for the purpose of compaction, as determined by Test No. 12 of BS 1337 and approved by the Engineer. The materials shall be so worked as to have uniform moisture content throughout the entire layer.

If the moisture content exceeds the aforementioned tolerance, the compaction operations shall not proceed until the materials are wetted or allowed to dry out, as the case may be to obtain the optimum moisture content within the permissible tolerances. However, there may be an exception with a specific approval of the Engineer. No adjustment in price shall be made on account of any operations of the Contractor related to wetting or drying the materials or on account of any delays occasioned thereby.

When the material has been conditioned and placed as specified or directed, it shall be compacted with appropriate motorized vibratory compaction equipment or tampers of adequate weight and size as approved by the Engineer. Each layer shall be compacted to obtain at least 98% compaction of the maximum dry density. If the test results show that the density has not met the requirement, the Contractor shall have to carry out further compaction until the required density is achieved.

The in-situ dry density of the compacted fill shall be determined by the Sand Replacement Method described in Test No. 15 of BS 1377 or by other similar approved tests at locations as ordered by the Engineer.

8.1.4 Measurement

Measurements for earth filling works shall be taken for payment in cubic meters on cross sections compacted and accepted in place. The volume to be measured will be the net volume of required and accepted filling, actually constructed and completed in accordance with the Specifications, to the lines, levels and cross sections required as per the Drawings or such other dimensions as directed by the Engineer. This stipulation of volume determination will be regardless of the method of excavation, filling, re-sectioning and backfilling at structures or type of materials.

The cross sections to be used shall be measured by pre-work (after clearing and stripping) and post-work field surveyed sections. Pre-work sections of the portion of the work allotted to the Contractor, computed through survey works, shall be signed by the Contractor before executing the works for retention by the Engineer.

8.1.5 Payment

The unit rate paid per cubic meter for earth filling shall be in accordance with the Contract unit price, which payment shall constitute the full compensation for furnishing all materials and providing all labour, tools and equipment and works as specified. The rate shall also include costs of all other items related therewith and all incidentals, which may need to be completed to execute the work strictly in accordance with the Specifications and/or as per the directions of the Engineer.

Costs of all works and the cost of lead, lift or carriage shall be included in the unit rates for the relevant item of earth filling works of the BOQ of the Contract. Unless otherwise specified, no royalties will be paid for the purchase of earth from a private land regardless of its distance from the Site. No additional
payment shall be made for purchasing a land and excavating the fill outside the rate agreed in the Contract for the item of earth filling works.

No direct or separate payment shall be made for works required under the other sub-items of this item. Costs for such works shall be deemed to have included in the related items of the BOQ.

Payment shall only be made when all works have been completed in accordance with the designed sections satisfying all specifications and accepted by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tr>
<td>Earth filling</td>
<td>Cubic Meter</td>
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</tbody>
</table>
8.2 Sand filling

8.2.1 Description

This work shall consist of filling in foundation trenches, inside plinth or at any other places by furnishing, placing, compacting and shaping suitable sand of acceptable quality and F.M. to make up levels to the lines, levels, grades, dimensions and cross sections in accordance with these specifications and as shown on the Drawings or BOQ and/or as instructed by the Engineer.

8.2.2 Materials

Materials shall be of natural sand free from vegetable matters, from soft particles and from clay. F.M. of sand shall be in accordance with the stipulations of the BOQ or as per the direction of the Engineer.

All fill materials shall be stockpiled outside the working areas. Materials shall be tested and approved by the Engineer.

The selected sand fill so stockpiled, shall satisfy the following criteria:

- The fraction passing the 425 micron sieve shall have a Plasticity Index not greater than 10 (STP Section-3).
- The material shall have a soaked CBR value not less than 8% when compacted to 98% of maximum dry density as to be determined by STP T 4.5.

8.2.3 Construction methods

General

Prior to placing any sand fill upon any area, all clearing and grubbing operations shall be completed. Within the limits of sand filling, tree stumps shall completely be removed.

The original ground surface should be prepared by scarifying, watering, aerating and compacting.

Spreading and compaction of sand fill

Sand fill shall be placed on the desired place in horizontal layers and each layer shall not exceed a loose thickness that will be required to obtain a compacted thickness of 150mm. Sand in each basket is to be placed near to the sand placed before it and spread systematically. The Contractor shall not be allowed to throw sand in heaps.

The compacted materials should become homogeneous and free from lenses, pockets, streaks or other imperfections. Placing operations shall be such that the materials, when compacted, will be blended sufficiently to secure the best practicable degree of compaction, impermeability and stability and for this purpose the preceding compacted layer shall be scarified before placing a new layer.

All fill materials shall generally be compacted mechanically. However, under some special circumstance and when specifically allowed under the BOQ, the fill may be allowed to be compacted manually.

If the density measurement checks fall below the specified density level, re-compacting shall be required irrespective of the field compaction trial results The Contractor shall carry out such works at his own expenses.
Sand fill materials not containing sufficient moisture requirement for compaction in accordance with the requirements of this Sub-section, shall be reworked and watered as per the direction of the Engineer. The Contractor shall carry out this work at his own expenses.

Sand fill materials containing excess moisture shall be reworked and dried prior to or during compaction. Drying of wet materials shall be performed by methods proposed by the Contractor and approved by the Engineer at the expenses of the Contractor.

Compaction of every layer shall have to be approved by the Engineer. In the event the Contractor fails to obtain the approval of the Engineer of a fill layer, the materials above the unsatisfactory layer shall be removed and the unsatisfactory layer shall be re-compacted to satisfy the specifications at the expenses of the Contractor.

**Procedure for manual compaction**

Sand shall be compacted manually by using concrete drop hammers each weighing 6 kg to 7 kg, fitted with a shaft of about 1.5m long. Ramming shall reduce the voids and shall continue until no further shrinkage of sand is possible by ramming.

Before commencing ramming, the moisture content of sand shall be increased or decreased as per requirement by sprinkling water or by allowing natural drying of sand as applicable so as to ensure that the materials shall have a moisture content of not less than 3% or greater than 3% dry of the optimum moisture required for the purpose of compaction respectively.

The compaction operations shall not proceed until the material is wetted or allowed to dry out, as may be required, to obtain optimum moisture content within the tolerances as permitted above. However, there may be an exception with a specific approval of the Engineer. No adjustment in price shall be made on account of any operations of the Contractor in wetting or drying the materials or on account of any delays occasioned thereby.

The preceding operations shall continue layer after layer until the top of the filling is reached.

**Mechanical compaction**

In the case of mechanical compaction, area of filling, designated on the Drawings or by the Engineer, shall be compacted to the lines and grades shown on the Drawings or established by the Engineer. The Contractor’s operations in importing materials, designated for use, shall be such as will result the desired F.M.

Just prior to and during compacting operations, the materials shall have a moisture content of not greater than 3% wet or less than 3% dry of the optimum moisture required for the purpose of compaction, as determined by Test No. 12 of BS 1337 and approved by the Engineer. The materials shall be so worked as to have uniform moisture content throughout the entire layer.

If the moisture content is less than optimum by more than 3% or is greater than optimum by more than 3%, the compaction operations shall not proceed until the material is wetted or allowed to dry out, as may be required, to bring the optimum moisture content within the tolerances. However, there may be an exception with a specific approval of the Engineer. No adjustment in price shall be made on account of any operations of the Contractor in wetting or drying the materials or on account of any delays occasioned thereby.

When the material has been conditioned and placed as specified or directed, it shall be compacted with appropriate motorized vibratory compaction equipment or tampers of adequate weight and size as
approved by the Engineer. Each layer shall be compacted to obtain at least 98% compaction of the maximum dry density (STD). If the test results show that the density has not met the requirement, the Contractor shall have to carry out further compaction until the required density is achieved.

8.2.4 Measurement

Measurement shall be taken for payment on the compacted volume of completed and accepted works in cubic meter. The cross sections to be used will be the areas bound by the original ground (existing) shaped or leveled, the sides and the bottom of the foundation or the floor.

8.2.5 Payment

Payment for sand filling shall be made at the Contract unit price per cubic meter measured as provided above which price shall constitute the full compensation for furnishing all materials with their storage, placing, leveling and shaping, wetting or drying, compacting the fill materials and providing all equipment, tools and all incidentals necessary to complete the work true to the Specifications and/or as per the directions of the Engineer.

Payment shall only be made when all works have been completed in accordance with the designed sections satisfying all Specifications and accepted by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sand filling</td>
<td>Cubic Meter</td>
</tr>
</tbody>
</table>
SECTION-9
BRICK MASONRY AND BRICK WORKS
## BRICK MASONRY AND BRICK WORKS

### CONTENTS

#### SECTION-9

<table>
<thead>
<tr>
<th>Subsection</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>9.1</td>
<td>Brick masonry work</td>
<td>63–67</td>
</tr>
<tr>
<td>9.1.1</td>
<td>General</td>
<td>63</td>
</tr>
<tr>
<td>9.1.2</td>
<td>Description</td>
<td>63</td>
</tr>
<tr>
<td>9.1.3</td>
<td>Materials</td>
<td>63</td>
</tr>
<tr>
<td>9.1.4</td>
<td>Soaking of bricks</td>
<td>64</td>
</tr>
<tr>
<td>9.1.5</td>
<td>Preparation of mortars</td>
<td>64</td>
</tr>
<tr>
<td>9.1.6</td>
<td>Construction methods</td>
<td>64-65</td>
</tr>
<tr>
<td>9.1.7</td>
<td>Scaffolding</td>
<td>65</td>
</tr>
<tr>
<td>9.1.8</td>
<td>Protection and curing</td>
<td>66</td>
</tr>
<tr>
<td>9.1.9</td>
<td>Finishing of surfaces</td>
<td>66</td>
</tr>
<tr>
<td>9.1.10</td>
<td>Repairing of brick masonry work</td>
<td>66</td>
</tr>
<tr>
<td>9.1.11</td>
<td>Extension to the existing brick masonry work</td>
<td>66-67</td>
</tr>
<tr>
<td>9.1.12</td>
<td>Replacement of defective brick masonry work</td>
<td>67</td>
</tr>
<tr>
<td>9.1.13</td>
<td>Measurement</td>
<td>67</td>
</tr>
<tr>
<td>9.1.14</td>
<td>Payment</td>
<td>67</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Subsection</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>9.2</td>
<td>Machine made ceramic brick masonry work</td>
<td>68–70</td>
</tr>
<tr>
<td>9.2.1</td>
<td>Description</td>
<td>68</td>
</tr>
<tr>
<td>9.2.2</td>
<td>Materials</td>
<td>68</td>
</tr>
<tr>
<td>9.2.3</td>
<td>Mortar</td>
<td>68</td>
</tr>
<tr>
<td>9.2.4</td>
<td>Construction methods</td>
<td>69</td>
</tr>
<tr>
<td>9.2.5</td>
<td>Scaffolding</td>
<td>69</td>
</tr>
<tr>
<td>9.2.6</td>
<td>Protection and curing</td>
<td>69</td>
</tr>
<tr>
<td>9.2.7</td>
<td>Finishing of surfaces</td>
<td>70</td>
</tr>
<tr>
<td>9.2.8</td>
<td>Repairing of brick work</td>
<td>70</td>
</tr>
<tr>
<td>9.2.9</td>
<td>Measurement</td>
<td>70</td>
</tr>
<tr>
<td>9.2.10</td>
<td>Payment</td>
<td>70</td>
</tr>
</tbody>
</table>

<table>
<thead>
<tr>
<th>Subsection</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>9.3</td>
<td>Brick masonry work in partition wall</td>
<td>71–72</td>
</tr>
<tr>
<td>9.3.1</td>
<td>Description</td>
<td>71</td>
</tr>
</tbody>
</table>
9.3.2 Materials
- Bricks 71
- Cement 71
- Fine aggregate 71
- Water 71

9.3.3 Soaking of bricks 71

9.3.4 Mortar 71-72

9.3.5 Construction methods 72

9.3.6 Measurement 72

9.3.7 Payment 72

9.4 Brick flat soling 73-74

9.4.1 Single layer brick flat soling 73

9.4.1.1 Description 73

9.4.1.2 Materials
- Bricks 73
- Sand 73

9.4.1.3 Construction methods 73

9.4.1.4 Measurement 73

9.4.1.5 Payment 73

9.4.2 Double layer brick flat soling 74

9.4.2.1 Description 74

9.4.2.2 Materials
- Bricks 74
- Sand 74

9.4.2.3 Construction methods 74

9.4.2.4 Surface tolerance 74

9.4.2.5 Measurement 74

9.4.2.6 Payment 74

9.5 Herring-bone bond brick laying 75

9.5.1 Description 75

9.5.2 Materials
- Bricks 75
- Sand 75

9.5.3 Construction methods 75

9.5.4 Surface tolerance 75

9.5.5 Measurement 75

9.5.6 Payment 75
SECTION-9
BRICK MASONRY AND BRICK WORKS

9.1 Brick masonry work

9.1.1 General
This Sub-section shall apply to construction of all brick masonry works at the different places of a building.

9.1.2 Description
This item of work shall consist of constructing brick masonry work in cement mortar with specified proportion in required thickness and height. The work shall include supply of all labour, materials, tools and equipment, carriage and the performance of all works necessary for the construction of the brick masonry. All works including scaffolding shall be carried out in accordance with these Specifications conforming to the levels, dimensions and designs as shown on the Drawings, in the BOQ and/or as directed by the Engineer.

9.1.3 Materials
Bricks
Bricks shall be of First Class in quality unless otherwise required and shall comply with the requirements as stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate
Fine aggregates shall consist of natural sand conforming to the requirements of ASTM C 144 and ASTM C 33 or BDS 243.

Sand shall be completely non-saline, non-plastic and free from all clay, roots and all organic impurities.

Minimum F.M. of sand for any type of brickwork shall normally be 1.5 unless otherwise required for a specific work.

All other properties of Fine Aggregate shall be the same as have been described under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.
9.1.4 **Soaking of bricks**

Before use in works, all bricks shall be soaked in clear water for a minimum period of 24 hours. Soaking shall be discontinued 2 hours before use so that at the time of laying they are skin dry. Such soaked bricks shall be stacked on a clean place where they shall not be spoiled by dirt or any other objectionable materials.

9.1.5 **Preparation of mortar**

Cement mortar for brick masonry works shall generally consist by volume of one part Ordinary Portland cement and four parts screened sand or one part Ordinary Portland cement and six parts screened sand unless otherwise required by the Drawings or instructed by the Engineer. In each mortar, just enough water shall be added and the components mixed and thoroughly incorporated together to give workability appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform and cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only the minimum necessary to produce a workable mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as directed by the Engineer.

Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30 minutes after water is added. The adding of additional water and re-tempering (cement mortar that stiffened because of evaporation of water) shall be permitted only within 30 minutes from the time of addition of water at the time of initial mixing.

9.1.6 **Construction methods**

The method and equipment used for transporting and placing the bricks and mortar shall be such as will not damage the brick or delay the use of mixed mortar. All equipment and tools used for mixing or transporting mortar and bricks shall be clean and free from set mortar or other deleterious foreign substances.

All brick masonry works shall be placed only after the foundation surfaces have been prepared satisfactorily in accordance with the Specifications and the Engineer's instructions.

The bricks shall not be placed during rain sufficiently heavy or prolonged, which will wash the mortar from the bricks. Mortar already spread, but becomes diluted by rain, shall be removed and replaced before further continuing the work at the Contractor's own expenses.

All bricks to be used in brick masonry works with mortar joints shall be completely soaked in water for a minimum period of 24 hours before they are used. The bricks shall be used within two hours of taking out of water. All bricks shall be free from water adhering to their surface when they are placed in the brickwork.

Before laying bricks in foundation, a bed of lean concrete of thickness not less than 75mm shall be laid to prepare a smooth seat on which the brick works will rest. Immediately thereafter, the first course of bricks shall be laid.
Bricks shall be laid in English bond unless otherwise directed by the Engineer and shall be set with both bed and vertical joints filled with mortar and shall be bedded in by firmly tapping with the handle of the trowel. The face with the frog mark shall be placed upward to ensure that the frog mark is filled with mortar. Bricks shall be skillfully laid with the level courses, uniform joints, square corners, plumb verticals and true surfaces except when otherwise shown on the Drawings and/or directed by the Engineer.

Bricks, used on face, shall be selected whole or uniform size and with true rectangular face. Only full bricks shall be used in the brick works unless becomes absolutely necessary for breaking points or maintaining bond.

Bricks shall be laid on full bed of mortar and shall be slightly pressed so that mortar gets into all the surface pores of bricks to ensure proper adhesion. Bricks shall be laid, where possible, from one face only and each brick shall be set with both horizontal and vertical joints filled with mortar and the bricks shall be bedded in by firmly tapping with the handle of the trowel. Mortar joints shall be checked and any hollow or defective joints shall be racked and filled with mortar immediately.

Each course shall break the joints with the course below. All horizontal joints shall be parallel and all vertical joints in alternate courses shall be directly over one another. In thick walls or foundations, not only the face joints but the inside joints also shall break course.

The thickness of mortar in any joints shall not be less than 6mm and not more than 10mm and the height of four courses as laid shall not exceed more than 25mm the height of four dry bricks stacked one upon the other.

All brick masonry works shall be truly plumbed and shall always be carried out regularly along their entire length throughout the structure. When the entire works cannot be carried out in even courses, the break shall be made at regular steps each of a length of at least 1-1/2 times its height. Unless otherwise directed, no overhead work shall be allowed. Toothenning may be done where future extension is contemplated but shall be used as an alternative to racking back.

Where specified, fabric reinforcement shall be embedded completely in mortar. During construction of steining members, bars shall be placed accurately in accordance with the Drawings and/or as directed by the Engineer.

The surface of each course shall be thoroughly cleaned from all dirt before another course is laid on top of it. If the mortar in any course has begun to set, the joints shall be racked out to a depth of 25mm before any subsequent course is laid. When the top course has been exposed for more than two weeks, it shall be removed and the surface below shall be thoroughly cleaned before any more courses are added.

When fresh masonry is to be placed against the existing surface of structures, these surfaces shall be cleaned of all loose materials, roughened and wetted as directed by the Engineer so as to effect a good bond with the new work.

9.1.7 Scaffolding

The scaffolding shall be sound and strong to withstand all loads likely to be imposed upon it and subject to the Engineer’s approval. Pole going into the masonry should be at a place, which can be filled with a header brick. The holes, which provide resting space for horizontal members shall not be left in masonry under 1m in width or immediately near the skewbacks of arches. The holes left in the masonry work for supporting the scaffolding shall be filled and made good.
9.1.8 Protection and curing

Brick masonry works shall be protected during construction for a minimum period of 3 days after laying against harmful effects of weather by suitable covering. During hot weather, all finished or partly completed works shall be covered or wetted in such a manner as will prevent rapid drying of the brickwork.

All brick masonry, requiring mortar, shall be cured as it is constructed for not less than 7 days on completion of the last course by keeping continuously wet with water or by covering with water saturated materials or by other curing methods as may be approved by the Engineer.

On completion of the work, all visible surfaces shall be free from all damages and/or debris and shall look clean. Cares shall be taken that bricks are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.

9.1.9 Finishing of surfaces

General

The surfaces shall be finished by “Jointing” or “Pointing”. The surfaces which shall remain exposed shall be pointed and those which shall be buried underground shall be jointed. The mortar for finishing shall be prepared as stated under the Sub-section on ‘Mortars’ of this Section.

Jointing

In jointing, the face joints of the mortar shall be worked out while still green to give a finished surface flushed with the face of the brick masonry works. The faces of brick masonry works shall be cleaned to remove any splash of mortar during the course of raising the brick masonry.

Pointing

For pointing, the joints shall be squarely raked out to a depth of 15mm while the mortar is still green. The raked joints shall be well brushed to remove dust and loose particles and the surface shall be thoroughly washed with water, cleaned and wetted. The mortar shall be filled and pressed into the raked out joints before giving the required finish. The pointing shall then be finished to proper type given on the Drawings or as required under the BOQ and/or as directed by the Engineer.

If type of pointing is not mentioned on the Drawings or BOQ, flush pointing shall be used. For groove pointing, after the mortar has been filled and pressed into the joints and finished with the edges of the bricks, it shall be grooved along the center with a half round tool of such width as may be specified by the Engineer. Such works shall only be carried out within the period that the mortar remains green. The excess mortar shall then be cut off from the edges of the lines and the surfaces of the masonry shall also be cleaned of all mortars.

9.1.10 Repairing of brick masonry work

On completion of any brick masonry works, if any bricks is found out of alignment or level or does not conform to the lines and grades shown on the Drawings or gives a defective surface, it shall be removed and replaced by the Contractor at his own expenses as per the instructions of the Engineer.

9.1.11 Extension to the existing brick masonry work

Where existing brick masonry works is required to be extended, the existing skin courses shall be carefully removed as directed by the Engineer. The old exposed brick masonry works and joints shall be
carefully prepared and cleaned and thoroughly watered immediately prior to commencing extension of the brick masonry.

9.1.12 Replacement of defective brick masonry work

The extent of replacement of defective brick masonry works shall be jointly surveyed by the Contractor and the Engineer at the start of the work and the location of all repairs needed shall be recorded and permanently marked in paint.

Defective brick masonry works shall be carefully removed in a sequence as instructed by the Engineer. The existing stability and structural integrity of walls and arches shall not be impaired during this operation and the Contractor shall remain fully responsible in this respect. To this effect he shall for ensure providing all temporary supports as would be necessary. The defective brick masonry works shall be removed on an incremental basis, if this is deemed desirable and new brick masonry works shall be carefully constructed so that the structural integrity is maintained.

9.1.13 Measurement

The quantity of completed brick masonry works (both new and extension of the existing brick masonry works) including flush pointing as accepted by the Engineer shall be measured in cubic meter in accordance with the dimensions shown on the Drawings and/or as directed by the Engineer.

9.1.14 Payment

The works measured as provided above shall be paid at the relevant Contract unit prices per cubic meter and per square meter as would be applicable as per the BOQ. The payment shall constitute the full compensation for furnishing, storage, transporting, preparing, laying, racking out joints and curing of all materials and labour including scaffolding, tools and equipment and all incidentals necessary to complete the item. The payment shall also constitute full compensation for all temporary measures to retain the stability of the existing structures and for removal and replacement of any defective brick works and mortar.

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<thead>
<tr>
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<tr>
<td>New and extended brick masonry work</td>
<td>Cubic meter</td>
</tr>
<tr>
<td>Replacement of existing defective brick masonry work</td>
<td>Cubic meter</td>
</tr>
</tbody>
</table>
9.2 Machine made ceramic brick masonry work

9.2.1 Description
Works covered by this item shall consist of constructing 50mm and 100mm thick brickwork with First Class clinker pressed Machine Made Bricks laid normally or on edge pasted on wall, column, beam, floor and sun shed bottom or elsewhere as required by the Drawings or as per the instruction of the Engineer.

9.2.2 Materials

Bricks
Machine Made Ceramic Bricks shall be of First Class in quality and shall comply with the requirements of First Class Machine Made Bricks as stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate
Fine aggregates shall consist of natural sand conforming to the requirements of ASTM C 144 and ASTM C 33 or BDS 243.

Sand shall be completely non-saline, non-plastic and free from all clay, roots and all organic impurities.

Minimum F.M. of sand for this type of brickwork shall normally be 1.5 unless otherwise required for a specific work.

All other properties of Fine Aggregate shall be the same as have been described under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

9.2.3 Mortar
Portland cement mortar for installing ceramic wall shall comply with the requirements of ANSI A 108.1 and be of the composition specified in the Table given below.

<table>
<thead>
<tr>
<th></th>
<th>Scratch coat</th>
<th>Setting bed and leveling coat</th>
<th>Setting bed</th>
<th>Scratch coat or sand bed</th>
</tr>
</thead>
<tbody>
<tr>
<td>Walls</td>
<td>1 cement; 1/5 hydrated lime*; 4 dry or 5 damp sand</td>
<td>1 cement; 1/10 hydrated lime; 5 damp sand to 1 cement; 1 hydrated lime; 7 damp sand</td>
<td>1 cement; 1/10 hydrated lime; 5 dry or 6 damp sand; or 1 cement; 5 dry or 6 damp sand</td>
<td>1 cement; 1/5 hydrated lime; 2½ dry sand or 3 damp sand</td>
</tr>
<tr>
<td>Floors</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
<tr>
<td>Ceilings</td>
<td></td>
<td></td>
<td></td>
<td></td>
</tr>
</tbody>
</table>

Note:* Lime may be excluded from mortar if trial mixes indicate the desired workability and performances are achieved without lime.
9.2.4 Construction methods

The method and equipment used for transporting and placing bricks and mortar shall be such as will not damage the bricks or delay the use of mixed mortar. All equipment and tools used for mixing or transporting mortar and bricks shall be clean and free from set mortar or other deleterious foreign substances.

The bricks shall not be placed during rain sufficiently heavy or prolonged, which will wash the mortar from the bricks. Mortar already spread but becomes diluted by rain, shall be removed and replaced before continuing the work at the expenses of the Contractor.

All bricks to be used in the works shall be completely soaked in water for a minimum period of 24 hours before they are used. The bricks shall be used within two hours of taking out of water. All bricks shall be free from water adhering to their surface when they are placed in the brickwork.

The bricks used on face shall be selected whole or uniform size and with true rectangular face. Only full bricks shall be used in the brickwork unless and otherwise becomes absolutely necessary.

Works shall be built plumbed and shall be carried out regularly throughout the entire area of the structure by very skilled masons. Brick shall be laid, as per the Design and Drawings and only the smooth face of the Machine Made Bricks shall be used. Bricks shall be laid on full bed of mortar and shall be slightly pressed so that mortar gets into all the surface pores of bricks to ensure proper adhesion. Thickness of horizontal and vertical joints shall be as per the Drawings. Exposed joints shall be raked out and groove pointed as per requirement and/or directed by the Engineer. The face of the works shall be kept clean as work proceeds.

9.2.5 Scaffolding

The scaffolding shall be sound and strong to withstand all loads likely to be imposed upon it and subject to the Engineer’s approval. Pole going into the masonry should be at a place, which can be filled with a header brick. The holes, which provide resting space for horizontal members shall not be left in masonry under 1m in width or immediately near the skewbacks of arches. The holes left in the masonry work for supporting the scaffolding shall be filled and made good to resemble with the entire surfaces unless otherwise required by the Engineer.

9.2.6 Protection and curing

In exposed situation the works shall be protected during construction against harmful effects of weather by suitable covering during and for a period immediately following construction until the mortar has sufficiently hardened. During hot weather, all finished or partly completed works shall be covered or wetted in such manner as will prevent rapid drying of the brick masonry works.

All brick masonry, requiring mortar, shall be cured as it is constructed for not less than 7 days on completion of the last course by keeping continuously wet with water or by covering with water saturated material or other curing methods approved by the Engineer.

On completion of the work, all visible surfaces shall be free from damage or debris and shall look clean. Cares shall be taken that bricks are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.
9.2.7 Finishing of Surfaces

General

The surfaces shall be finished by “Pointing”. The surfaces which shall remain exposed shall be pointed. The mortar for finishing shall be prepared as stated under Sub-section on ‘Mortars’ of this item of work.

Pointing

The joints shall be squarely raked out to a depth of 15mm while the mortar is still green. The raked joints shall be well brushed to remove dust and loose particles and the surface shall be thoroughly washed with water, cleaned and wetted. The mortar shall be filled and pressed into the raked out joints before giving the required finish. The pointing shall then be finished to proper type given on the Drawings.

For groove pointing after the mortar has been filled and pressed into the joints and finished off level with the edges of the bricks, it shall while still green be grooved along the center with a half round tool of such width as may be specified by the Engineer. The excess mortar shall then be cut off from the edges of the lines and the surfaces of the masonry shall also be cleaned of all mortars.

9.2.8 Repairing of brick work

On completion of works, if any bricks is found out of alignment or level or does not conform to the lines and grades shown on the Drawings or shows a defective surface, it shall be removed and replaced by the Contractor at his expenses.

9.2.9 Measurement

The quantity of Machine Made brick masonry works shall be measured in square meter of the finished visible surfaces on wall, column, beam, floor and sun-shed bottom or elsewhere as required and as will be acceptable to the Engineer qualitatively. Addition may be made for thresholds and deduction for openings of the floor and other construction.

9.2.10 Payment

The works measured as provided above shall be paid at the relevant Contract unit prices as shown in the BOQ. The payment shall constitute the full compensation for furnishing, storage, transporting, preparing, laying, recessed pointing and curing of all materials and labour including scaffolding, tools and equipment and all incidentals necessary to complete the item. The payment shall also constitute full compensation for all temporary measures to retain the stability of the existing structures and for removal and replacement of any defective brick works and mortar.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Machine made ceramic brick masonry</td>
<td>Square Meter</td>
</tr>
</tbody>
</table>
9.3 **Brick masonry work in partition wall**

9.3.1 **Description**

This item of work shall consist of constructing flat-brick brick masonry walls or brick-on-edge brick masonry walls with brick masonry works of 1st class bricks and in cement mortar with specified proportion in required heights. All works shall be carried out in accordance with these Specifications conforming to the levels, dimensions and designs as shown on the Drawings and/or as directed by the Engineer.

9.3.2 **Materials**

**Bricks**

Bricks shall be of First Class in quality unless otherwise required and shall comply with the requirements as stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

**Cement**

Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

**Fine aggregate**

Fine aggregates shall consist of natural sand conforming to the requirements of ASTM C 144 and ASTM C 33 or BDS 243. Sand shall be completely non-saline, non-plastic and free from all clay, roots and all organic impurities. Minimum F.M. of sand for this type of brickwork shall normally be 1.5 unless otherwise required for a specific work. All other properties of Fine Aggregate shall be the same as have been described under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

**Water**

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

9.3.3 **Soaking of bricks**

Before use in works, all bricks shall be soaked in clear water for a minimum period of 24 hours. Soaking shall be discontinued 2 hours before use so that at the time of laying they are skin dry. Such soaked bricks shall be stacked on a clean place where they shall not be spoiled by dirt or any other objectionable materials.

9.3.4 **Mortar**

Unless otherwise specified on the Drawings, cement mortar for brick masonry works shall generally consists by volume of one part Ordinary Portland cement and four parts screened sand unless otherwise required by the Drawings or instructed by the Engineer. In each mortar, just enough water shall be added
and the components mixed and thoroughly incorporated together to give a workability, appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform and cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only the minimum necessary to produce a workable mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as directed by the Engineer.

Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30 minutes after water is added. The adding of additional water to and re-tempering (cement mortar that stiffened because of evaporation of water) shall be permitted only within 30 minutes from the time of addition of water at the time of initial mixing.

9.3.5 Construction methods

Construction methods shall be similar to that have been described in the relevant portion under the Sub-section on ‘Brick Masonry Work’ of this Section.

9.3.6 Measurement

Brick partition walls shall be measured in square meter of actually exposed brick surfaces wall that has been actually performed and accepted by the Engineer from which areas for all openings and incorporated foreign structures such as lintels, columns, beams, etc. shall be deducted.

9.3.7 Payment

The amount of completed and accepted work measured as provided above shall be paid at the relevant Contract unit prices as shown in the Bill of Quantities. The payment shall constitute the full compensation for furnishing, storing, transporting, preparing, laying, racking out joints and curing of all materials, all labours including scaffolding, tools and equipment and all incidentals necessary to complete the item. The payment shall also constitute full compensation for all temporary measures to retain the stability of the existing structures and for removal and replacement of any defective brick works and mortar.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Brick masonry works in partition wall</td>
<td>Square meter</td>
</tr>
<tr>
<td>Brick Pointing</td>
<td>Square meter</td>
</tr>
<tr>
<td>Re-pointing of existing brick masonry works joints</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
9.4 Brick flat soling

9.4.1 Single layer brick flat soling

9.4.1.1 Description

This item of work shall consist of providing brick soling laid flat in single layer to the lines, grades, levels, dimensions, cross sections and satisfying other requirements as shown on the Drawings and/or as directed by the Engineer.

9.4.1.2 Materials

Bricks

Bricks shall be of First Class in quality unless otherwise required and shall comply with the requirements as stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Sand

Unless otherwise directed by the Engineer minimum F.M. of sand for Brick Flat Soling should normally be 0.8 that should satisfy all properties of fine aggregate as have been described under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

9.4.1.3 Construction methods

The bricks shall be laid flat in one layer, frog mark downwards with the shortest side vertical on the compacted, trimmed and prepared surface unless otherwise shown on the Drawings or instructed by the Engineer. Bricks shall be laid in a regular and uniform manner. Interstices of bricks shall be filled with sand of approved F.M and water shall be applied by sprinkling. No brick shall be laid on a foundation or any surface until the same has been inspected and approved by the Engineer. The gap between two adjacent bricks should not exceed 10mm.

Brick shall be laid and the interstices shall be tightly filled with sand in a manner that no brick shall move under any condition.

9.4.1.4 Measurement

Brick flat soling shall be measured in square meter for areas covered by the same and completed in accordance with the Specifications and accepted by the Engineer.

9.4.1.5 Payment

The amount of completed and accepted works measured as provided above shall be paid at the relevant Contract unit price as shown in the Bill of Quantities. The price and payment shall constitute the full compensation for all costs for completion of the work and supplying, furnishing, transportation and storage of all required materials, including costs of all labours, equipment, tools and all incidentals necessary to complete the work satisfying the requirements of this Sub-section, as shown on the Drawings, as stated in the BOQ and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Single layer brick flat soling</td>
<td>Square Meter</td>
</tr>
</tbody>
</table>
9.4.2 Double layer brick flat soling

9.4.2.1 Description

This item of work shall consist of providing brick soling laid flat in double layers to the lines, grades, levels, dimensions, cross sections and satisfying other requirements as shown on the Drawings and/or as directed by the Engineer.

9.4.2.2 Materials

Bricks
Bricks shall be of First Class in quality unless otherwise required and shall comply with the requirements as stated under the relevant Sub-section of the Section on 'Construction Materials' of this Specification.

Sand
Unless otherwise directed by the Engineer minimum F.M. of sand for Brick Flat Soling should normally be 0.8 that should satisfy all properties of fine aggregate as have been described under the relevant Sub-section of the Section on 'Construction Materials' of this Specification.

9.4.2.3 Construction methods

The first layer of the brick soling shall be laid flat, frog mark downwards with the shortest side vertical on the compacted, trimmed and prepared surfaces unless otherwise shown on the Drawings or instructed by the Engineer. The interstices shall be filled with sand of approved FM, brushed in and the completed layer shall be sprinkled liberally with water. Then a sand cushion of 25mm thickness (minimum) with same type of sand should be placed over the first layer. Second layer of the bricks shall be laid flat on the sand cushion in a regular and uniform manner as stated above. Interstices of bricks shall be filled with sand of approved F.M, brushed in and sprinkling water as did in the case of first layer. No bricks shall be laid on a foundation or any surface until the same has been inspected and approved by the Engineer.

Bricks shall be laid, sand cushion shall be provided between the layers and the interstices shall be tightly filled with sand in a manner that no bricks shall move under any condition.

9.4.2.4 Surface tolerance

In the areas where Double Layer Brick Flat Soling is to be placed, any deviation in excess of 5mm from the specified surface within 3 meter shall be corrected by removal, reshaping and relaying of bricks at the own expenses of the Contractor.

9.4.2.5 Measurement

Brick flat soling in double layer shall be measured in square meter for areas covered by the same and completed in accordance with the Specifications and accepted by the Engineer.

9.4.2.6 Payment

The amount of completed and accepted works measured as provided above shall be paid at the relevant Contract unit price as shown in the Bill of Quantities. The price and payment shall constitute the full compensation for all costs for completion of the work and supplying, furnishing, transportation and storage of all required materials, including cost of all labour, equipment, tools and all incidentals necessary to complete the work satisfying the requirements of this Sub-section, as shown on the Drawings, as stated in the BOQ and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
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<tbody>
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<td>Double layer brick flat soling</td>
<td>Square Meter</td>
</tr>
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</table>
9.5 **Herring-bone-bond brick laying**

9.5.1 **Description**

This item of work shall consist of a base composed of 1st class bricks, laid on edge in a herring-bone pattern, placed over a prepared single layer brick flat soling in accordance with these Specifications and to the lines, grades, levels, dimensions, cross sections and satisfying other requirements as shown on the Drawings and/or as directed by the Engineer.

9.5.2 **Materials**

**Bricks**

Bricks shall be of First Class in quality unless otherwise required and shall comply with the requirements as stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

**Sand**

Unless otherwise directed by the Engineer minimum F.M. of sand for Brick Flat Soling should normally be 0.8 that should satisfy all properties of fine aggregate as have been described under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

9.5.3 **Construction methods**

A sand cushion of 25mm thickness (minimum) with sand of approved F.M. shall be placed over the previously laid brick flat soling. Bricks shall then be laid on edge with 125mm across the surface in a single layer in a herring-bone pattern to the lines, grades, levels, dimensions and cross section shown on the Drawings and as required by the Engineer. The edge of the layer shall be made with cut bricks to produce a line, which is compatible with brick edging. The interstices shall be filled with sand of approved FM brushed in and the completed layer shall be sprinkled liberally with water.

Brick shall be laid, sand cushion shall be provided between the layers and the interstices shall be tightly filled with sand in a manner that no brick shall move under any condition.

9.5.4 **Surface tolerance**

In the areas where Herring-bone-bond brick layer to be placed, any deviation in excess of 5mm from the specified surface within 3 meter shall be corrected by removal, reshaping and relaying of bricks at the own expenses of the Contractor.

9.5.5 **Measurement**

Herring-bone-bond brick layer shall be measured in square meter for areas covered by the same and completed in accordance with the Specifications and accepted by the Engineer.

9.5.6 **Payment**

The amount of completed and accepted works measured as provided above shall be paid at the relevant Contract unit price as shown in the Bill of Quantities. The price and payment shall constitute the full compensation for all costs for completion of the work supplying, furnishing, transportation and storage of all required materials, including cost of all labour, equipment, tools and all incidentals necessary to complete the work satisfying the requirements of this Sub-section, as shown on the Drawings, as stated in the BOQ and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
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<tbody>
<tr>
<td>Herring-bone-bond layer excluding brick flat soling</td>
<td>Square Meter</td>
</tr>
</tbody>
</table>
SECTION-10
CONCRETE WORK
## CONCRETE WORK

### CONTENTS

#### SECTION-10

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>10.1</td>
<td>Concrete for structures</td>
<td>76–106</td>
</tr>
<tr>
<td>10.1.1</td>
<td>Description</td>
<td>76</td>
</tr>
<tr>
<td>10.1.2</td>
<td>Materials</td>
<td>76-78</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>76</td>
</tr>
<tr>
<td></td>
<td>Cement</td>
<td>76</td>
</tr>
<tr>
<td></td>
<td>Use of cement</td>
<td>76</td>
</tr>
<tr>
<td></td>
<td>Coarse aggregate</td>
<td>77</td>
</tr>
<tr>
<td></td>
<td>Nominal size of coarse aggregate</td>
<td>77</td>
</tr>
<tr>
<td></td>
<td>Fine aggregate</td>
<td>77</td>
</tr>
<tr>
<td></td>
<td>Water</td>
<td>77</td>
</tr>
<tr>
<td></td>
<td>Admixtures</td>
<td>77-78</td>
</tr>
<tr>
<td>10.1.3</td>
<td>Testing of materials</td>
<td>78-80</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>78</td>
</tr>
<tr>
<td></td>
<td>Cement</td>
<td>78</td>
</tr>
<tr>
<td></td>
<td>Aggregate</td>
<td>78-79</td>
</tr>
<tr>
<td></td>
<td>Water</td>
<td>79</td>
</tr>
<tr>
<td></td>
<td>Admixtures</td>
<td>80</td>
</tr>
<tr>
<td>10.1.4</td>
<td>Composition of concrete</td>
<td>80</td>
</tr>
<tr>
<td></td>
<td>Concrete classes</td>
<td>80</td>
</tr>
<tr>
<td>10.1.5</td>
<td>Regulation of water content</td>
<td>81</td>
</tr>
<tr>
<td>10.1.6</td>
<td>Durability of concrete</td>
<td>81-82</td>
</tr>
<tr>
<td></td>
<td>Special exposures</td>
<td>81-82</td>
</tr>
<tr>
<td></td>
<td>Sulphate exposures</td>
<td>82</td>
</tr>
<tr>
<td></td>
<td>Corrosion of reinforcement</td>
<td>82</td>
</tr>
<tr>
<td>10.1.7</td>
<td>Design of concrete mix</td>
<td>83-84</td>
</tr>
<tr>
<td>10.1.8</td>
<td>Proportioning of mix</td>
<td>84-85</td>
</tr>
<tr>
<td>10.1.9</td>
<td>Concrete in blinding layers</td>
<td>85</td>
</tr>
<tr>
<td>10.1.10</td>
<td>Batching</td>
<td>85-86</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>85</td>
</tr>
<tr>
<td></td>
<td>Bins and hoppers</td>
<td>85</td>
</tr>
<tr>
<td></td>
<td>Scales</td>
<td>85-86</td>
</tr>
<tr>
<td>10.1.11</td>
<td>Quality control of concrete</td>
<td>86-88</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>86</td>
</tr>
<tr>
<td></td>
<td>Adjustment of water/cement ratio</td>
<td>86</td>
</tr>
<tr>
<td></td>
<td>Slump tests</td>
<td>86</td>
</tr>
<tr>
<td></td>
<td>Compressive strength</td>
<td>86-87</td>
</tr>
<tr>
<td></td>
<td>Failure to pass tests</td>
<td>87</td>
</tr>
<tr>
<td></td>
<td>Testing hardened concrete</td>
<td>87-88</td>
</tr>
<tr>
<td>10.1.12</td>
<td>Concrete construction</td>
<td>88-95</td>
</tr>
<tr>
<td>10.1.12.1</td>
<td>General</td>
<td>88</td>
</tr>
<tr>
<td>10.1.12.2</td>
<td>Mixing concrete</td>
<td>88-90</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>88</td>
</tr>
<tr>
<td></td>
<td>Mixers at local Site of construction</td>
<td>88-89</td>
</tr>
<tr>
<td></td>
<td>Central plant mixers</td>
<td>89</td>
</tr>
</tbody>
</table>
Hand mixing

10.1.12.3 Conveying concrete
   General
   Agitator trucks
   Non-agitator-trucks
   Truck or transit mixers

10.1.12.4 Handling and placing of concrete

10.1.12.5 Depositing concrete under water

10.1.12.6 Compaction of concrete

10.1.13 Protection of concrete from adverse conditions
   General
   Protection from rain
   Protection from hot weather
   Protection from cold weather
   Special requirements for roof slabs
   Concrete exposed to salt water

10.1.14 Perforations and embedding of special devices

10.1.15 Curing of concrete
   General
   Materials
   Water
   Liquid membranes
   Waterproof sheet materials
   Methods
   Forms-In-place method
   Water method
   Liquid membrane curing compound method
   Waterproof cover method
   Accelerated curing
   Field cured specimens

10.1.16 Finish and finishing
   General
   Concrete surface finishing
   Ordinary finish
   Grout cleaning
   Rubbed finish
   Tooled finish
   Sandblasted finish
   Wire brushed or scrubbed finish
   Inspection and making good
   Protection

10.1.17 Second stage concrete

10.1.18 Factory made pre-cast concrete elements
   Handling and stacking of pre-cast units

10.1.19 Control of heat in structures

10.1.20 Back-fill to structures

10.1.21 Cleaning up

10.1.22 Measurement

10.1.23 Payment
<table>
<thead>
<tr>
<th>Section</th>
<th>Topic</th>
<th>Pages</th>
</tr>
</thead>
<tbody>
<tr>
<td>10.2</td>
<td>False work and forms</td>
<td>107-116</td>
</tr>
<tr>
<td>10.2.1</td>
<td>Scaffolding (false work)</td>
<td>107-108</td>
</tr>
<tr>
<td>10.2.2</td>
<td>Formwork</td>
<td>108-111</td>
</tr>
<tr>
<td></td>
<td>Definition</td>
<td>108</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>109</td>
</tr>
<tr>
<td></td>
<td>Materials</td>
<td>109</td>
</tr>
<tr>
<td></td>
<td>Construction method</td>
<td>109-110</td>
</tr>
<tr>
<td></td>
<td>Formwork for exposed concrete surfaces</td>
<td>110-111</td>
</tr>
<tr>
<td></td>
<td>Formwork for non-exposed concrete surfaces</td>
<td>111</td>
</tr>
<tr>
<td>10.2.3</td>
<td>Formed surfaces and finish</td>
<td>111</td>
</tr>
<tr>
<td>10.2.4</td>
<td>Sizes of timber and other sections for formwork</td>
<td>111</td>
</tr>
<tr>
<td>10.2.5</td>
<td>Quality of shuttering</td>
<td>112</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>112</td>
</tr>
<tr>
<td></td>
<td>Tolerances</td>
<td>112</td>
</tr>
<tr>
<td>10.2.6</td>
<td>Preparation of formwork</td>
<td>112-114</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>112-113</td>
</tr>
<tr>
<td></td>
<td>Removal of forms</td>
<td>113-114</td>
</tr>
<tr>
<td>10.2.7</td>
<td>Openings</td>
<td>114</td>
</tr>
<tr>
<td>10.2.8</td>
<td>Defects in formed surfaces</td>
<td>114-115</td>
</tr>
<tr>
<td>10.2.9</td>
<td>Holes to be filled</td>
<td>115</td>
</tr>
<tr>
<td>10.2.10</td>
<td>Approval of scaffoldings and form</td>
<td>115-116</td>
</tr>
<tr>
<td>10.2.11</td>
<td>Measurement</td>
<td>116</td>
</tr>
<tr>
<td>10.2.12</td>
<td>Payment</td>
<td>116</td>
</tr>
<tr>
<td>10.3</td>
<td>Water proofing polythene sheet</td>
<td>117</td>
</tr>
<tr>
<td>10.3.1</td>
<td>Description</td>
<td>117</td>
</tr>
<tr>
<td>10.3.2</td>
<td>Construction requirement</td>
<td>117</td>
</tr>
<tr>
<td>10.3.3</td>
<td>Measurement</td>
<td>117</td>
</tr>
<tr>
<td>10.3.4</td>
<td>Payment</td>
<td>117</td>
</tr>
</tbody>
</table>
SECTION-10
CONCRETE WORK

10.1 Concrete for structures

10.1.1 Description

This work shall consist of construction of all Portland Cement Concrete in structures, with or without reinforcement, which shall involve furnishing, placing, finishing and curing of concrete. All items of concrete work shall include elements of structures constructed by cast-in-place and pre-cast methods using either plain or reinforced concrete or any combination thereof and shall conform to the specifications and requirements of the different Sub-sections of this item of work. All structures shall be built in a workman like manner to the lines, grades and dimensions shown on the Drawings or as directed by the Engineer.

All concrete works shall be carried out in accordance with BS 8110 or ASTM C-685 and as specified by the Engineer.

All sampling and testing of constituent materials shall be carried out in accordance with the provisions of the appropriate British or American Standard and all sampling and testing of fresh and hardened concrete shall be carried out in accordance with the provisions of BS 1881 “Method of Testing Concrete” or similar under ASTM C 39.

10.1.2 Materials

General

Concrete shall be manufactured with the essential ingredients of Portland cement, fine aggregate, coarse aggregate and water as specified and shall be well mixed and brought to the proper consistency. Type and source of ingredients used in concrete shall conform to the approved samples and shall not be varied. The requirement for concrete, its constituent materials, methods and procedures shall conform to any of the Standard Specifications of ASTM, or BS or BDS or any other equivalent standard unless otherwise specified herein or directed by the Engineer.

Materials shall conform to the requirements specified below and in the relevant Section titled ‘Construction Materials’ of this Specification.

Cement

Cement used in the works shall be Ordinary Portland Cement complying with the requirements of ASTM C 150 Type 1 or BS 12 or BDS 232 or equivalent standard. Special cements shall conform to the requirements provided by the Engineer.

Use of cement

Cement of different manufacturers and with different brands or types shall be kept separately and shall not be used in the same mix.

Consignment of cement shall be used in the order of delivery.

Only one brand, grade or kind of cement shall be used in a given structure, except upon the written permission of the Engineer.
Coarse aggregate

Coarse aggregate for all types of Concrete with the exception of blinding concrete shall conform to the requirements of ASTM C 33.

Coarse aggregate shall be hard, durable, clean, free from dust and other deleterious materials. The grading of the coarse aggregate shall be such that when combined with the approved fine aggregate and cement, it shall produce workable concrete of maximum density.

Nominal size of coarse aggregate

Different sizes of coarse aggregates should be mixed in proportions, which would be determined during trial mixes. The course aggregate to be used in the concrete mix shall be dry mixed from different sizes in specified/selected proportion one day before casting.

Nominal size of the coarse aggregate shall not be larger than one-fifth of the narrowest dimension between sides of forms or one-third the depth of slabs or three-fourth the minimum clear spacing between individual reinforcing bars or bundle of bars.

Fine aggregate

Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of concrete. It shall conform to the requirements of ASTM C-33 or BDS 243 or equivalent standard.

Water

All sources of water for use in concrete shall be subject to the approval of the Engineer. Water shall be reasonably clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid material. Water whose concentration of chloride ion is in excess of 3,000 ppm (parts per million) shall not be used for the production of concrete. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. Such comparison shall be made by means of standard cement tests for soundness, time of setting and mortar strength. Any indications of unsoundness, change in time of setting of plus or minus 30 minutes or more, or reduction of more than 10 percent in mortar strength shall be sufficient cause for rejection of the water under test.

Admixtures

Suitable admixtures may be used in concrete mixes with the prior acceptance of the Engineer. The type and source of admixture, and the amount added and method of use shall be to the acceptance of the Engineer, who shall be provided with the following data:

- The manufacturer’s recommended dosage and detrimental effects of under-dosage and over-dosage.
- The chemical name of the main active ingredients in the admixture.
- Whether or not the admixture contains chloride and, if so, the chloride content of the admixture expressed in percentage of equivalent anhydrous calcium chloride by weight of admixture.
- Whether or not the admixture leads to the entraining of air when used at the manufacturers recommended dosage.
- Evidence of previous satisfactory performance of concrete containing the additive.
Admixtures containing chloride other than impurities from admixture ingredients shall not be used in concrete containing embedded aluminium, or in concrete cast against permanent galvanized metal forms.

In admixtures for use in reinforced concrete, the chloride ion content shall not exceed one percent by weight of the admixture.

If more than one admixture is used, the admixtures shall be compatible with each other and shall be incorporated into the concrete mix in correct sequence so that the desired effects of all admixtures are obtained.

Fly ash or other pozzolans used as admixtures shall conform to ‘Specification for Fly Ash and Raw or Calcined Natural Pozzolan for use as a Mineral Admixture in Portland Cement Concrete (ASTM C 618)’. All air entraining admixtures shall conform to ‘Specification for Air entraining Admixtures for Concrete (ASTM C 260)’.

Air entraining and chemical admixtures shall be incorporated into the concrete mix in a water solution. The water so included shall be considered to be a portion of the allowed mixing water. Admixtures shall be incorporated through a dispensing system sufficiently accurate to deliver within + 5% of the approved dosage rate.

All admixtures shall be used strictly in accordance with the manufacturer's instructions. A ‘Literature of Compliance’ of the admixture shall be furnished to the Engineer for each shipment of admixture used in the work. The said literature shall be based upon laboratory test results from an approved testing facility and shall authenticate that the admixture meets all requisite specifications.

10.1.3 Testing of materials

General

All tests shall be performed at Site and/or in the LGED Laboratories. Testing outside the scope of Site or LGED Laboratories shall be carried out at a recognized laboratory that will be designated by the Engineer. The test results shall be authenticated by the Head of the Laboratory.

Cement

Hydraulic cement shall be sampled and tested in accordance with the standard methods referred to in STP Section 10 and AASHTO M 85.

Cement may be sampled either at the factory or at the Site of the Work as provided in the Specifications.

The Contractor shall notify the Engineer of dates of delivery so that there will be sufficient time for sampling the cement, either at the factory or upon delivery. If this is not done or if additional tests are necessary, the Contractor may be required to re-handle the cement in the store for the purpose of obtaining the required samples.

Sampling shall normally be instructed by the Engineer for every stored 200 cubic meter of concrete production with the concerned cement type or if the source of cement has been changed.

Aggregate

Tests to assess the suitability of the aggregates proposed for use in concrete to be placed in the permanent works shall be as follows:
- Grading
- Magnesium sulphate soundness
- Specific gravity and water absorption
- Clay, silt and dust content
- Organic impurities
- Sulphate and chloride content
- Elongation and flakiness
- Potential alkali reactivity
- Los Angeles Abrasion Test
- Aggregate drying shrinkage.

These tests are to be carried out in accordance with the appropriate ASTM Standards and the results shall comply with the limits given therein or as otherwise stated in this Specification. Grading shall be carried out at least at a weekly interval when concrete is being produced on a regular basis or before the start of production when irregular.

The Contractor shall supply samples of the aggregate materials proposed to be used for testing of Elongation and Flakiness Index, Los Angeles Abrasion Value (coarse aggregate) and Fineness Modulus (fine aggregate) and grading and other tests as required by the Engineer.

From the aggregate materials proposed by the Contractor, samples shall be selected according to STP Section 1 and 2 in the presence of the Engineer. The samples shall be brought to the Site laboratory and tested for proving their conformance with the relevant Section on ‘Material Testing’ of this Specification and of this Sub-section.

The quality control of the aggregate shall be as directed by the Engineer. Grading shall normally be checked daily.

Moisture content of the aggregate shall be determined daily and at any time when a change in the moisture content is expected.

If the Contractor proposes to change the source of supply of aggregates, samples from the new source shall similarly be supplied and tested.

Grading of mixed coarse aggregate shall be checked at Site.

**Water**

The water used in mixing or curing concrete shall be tested by methods described in AASHTO Test Method T 260.

In sampling water for testing, care shall be taken that the containers are clean and that samples are representative.

When comparative tests are made with a water of known satisfactory quality, any indication of unsoundness, marked change in time of setting, or a reduction of more than 10 percent in mortar strength, shall be sufficient cause for rejection of the water under test.

Water shall be tested before commencement of work or any time required by the Engineer, or if the source is changed.
Admixtures

The Contractor shall submit to the Engineer specifications and samples of any admixtures or additives that he proposes to use at least 28 days before the commencement of construction or manufacture of the particular structure on which he intends to use the admixture.

Any tests the Engineer may require on concrete mixes on account of the Contractor's proposal to use additives shall be carried out at the expenses of the Contractor.

10.1.4 Composition of concrete

Concrete classes

The class of concrete and properties applicable to the concrete in various parts of structures shall be as specified in the following table.

Each mix shall be designed to ensure optimum workability, prevent segregation and produce a dense, durable concrete by adjusting the fine and coarse aggregate proportions following procedures as stated under the Sub-section of 'Design of Concrete Mix' of this Specification.

<table>
<thead>
<tr>
<th>Concrete Class</th>
<th>28 day Cylinder Strength in kg/cm² (minimum)</th>
<th>Coarse Aggregate Type</th>
<th>Mix Ratio (by volume) (only indicative)</th>
</tr>
</thead>
<tbody>
<tr>
<td>A-1</td>
<td>250</td>
<td>Crushed stone</td>
<td>1 : 1.5 : 3</td>
</tr>
<tr>
<td>A-2</td>
<td>210</td>
<td>Crushed stone</td>
<td>1 : 2 : 4</td>
</tr>
<tr>
<td>A-3</td>
<td>200</td>
<td>Picked Jhama Brick Chips</td>
<td>1 : 1.5 : 3</td>
</tr>
<tr>
<td>A-4</td>
<td>170</td>
<td>Picked Jhama Brick Chips</td>
<td>1 : 2 : 4</td>
</tr>
<tr>
<td>B-3</td>
<td>105</td>
<td>Picked Jhama Brick Chips</td>
<td>1 : 3 : 6</td>
</tr>
</tbody>
</table>

The various classes of concrete shall be placed at locations as would be specified on the Drawings (if so) or elsewhere as directed by the Engineer.

Strength requirement is the only determining factor for acceptance of any above stated class of concrete. The mix ratio only shows the minimum cement requirement and it shall not put the Engineer under any obligation to accept concrete unless the requisite strength is established. If required, the cement content has to be increased to attain the desired strength without any additional costs to be paid to the Contractor.

Strength of each and every cylinder tested should conform the aforementioned specified value. Trial mixes for every class of concrete with representative material from the Site, shall be prepared by the Contractor in the laboratory in accordance with the approved procedures. The nominal strength in these tests shall exceed the specified minimum strength by 10%.

If required, suitable admixtures as approved by the Engineer would have to be added to the concrete mix to attain the desired strength without any additional costs to be paid to the Contractor. The effect of the admixture shall be carefully observed by trial mix and tests before its use.

As the work progresses, the Engineer reserves the right to change the proportions from time to time, if conditions warrant so in the interest of satisfactory output. Any such changes will be made at no additional compensation to the Contractor.
10.1.5 Regulation of water content

The amount of water used in the concrete for volume batching shall be regulated to adjust for any variation of the moisture content or grading of the aggregates as they enter the mixer as follows:

The batched volume of damp fine aggregate shall be corrected to the equivalent volume of dry aggregate. The volume of moisture in the aggregates shall be deducted from the free water to be added to the mix. To expedite correction to fine aggregate, a “bulking curve” showing the relation between moisture content and increase over dry volume shall be prepared in advance by tests on the fine aggregate used. The Engineer may direct the use of a slump less than that specified whenever concrete of such lesser slump can be consolidated into place by means of vibration specified herein. Addition of water to overcome stiffening of the concrete before placing will not be permitted. Concrete shall have uniform consistency from batch to batch. Aggregate shall not be batched for concrete when free water is dripping from the aggregate.

Concrete mix proportions shall be such that the concrete is of adequate workability and can properly be compacted. Suggested ranges of values of workability of concrete for some placing conditions are given in the following Table.

<table>
<thead>
<tr>
<th>Degree of Workability</th>
<th>Placing conditions</th>
<th>Nominal maximum aggregate (mm)</th>
<th>Compacting factor</th>
<th>Slump mm</th>
</tr>
</thead>
<tbody>
<tr>
<td>Very low</td>
<td>Small sections (i.e. pre-cast or &gt; 300mm thick) subjected to intensive vibration and large sections to normal vibration</td>
<td>20</td>
<td>0.78</td>
<td>0 – 10</td>
</tr>
<tr>
<td></td>
<td></td>
<td>40</td>
<td>0.78</td>
<td>0 – 25</td>
</tr>
<tr>
<td>Low</td>
<td>Simple reinforced sections with vibration and large sections without vibration</td>
<td>20</td>
<td>0.85</td>
<td>10 – 25</td>
</tr>
<tr>
<td></td>
<td></td>
<td>40</td>
<td>0.85</td>
<td>25 – 50</td>
</tr>
<tr>
<td>Medium</td>
<td>Simple reinforced sections without vibration and heavily reinforced sections with vibration</td>
<td>20</td>
<td>0.92</td>
<td>25 – 50</td>
</tr>
<tr>
<td></td>
<td></td>
<td>40</td>
<td>0.92</td>
<td>50 – 100</td>
</tr>
<tr>
<td>High</td>
<td>Heavily reinforced sections without vibration</td>
<td>20</td>
<td>0.95</td>
<td>50 – 125</td>
</tr>
<tr>
<td></td>
<td></td>
<td>40</td>
<td>0.95</td>
<td>100 – 175</td>
</tr>
</tbody>
</table>

When the consistency of the concrete is found to exceed the nominal slump, the mixture of subsequent batches shall be adjusted to reduce the slump to a value within the nominal range. Batches of concrete with a slump exceeding the maximum specified shall not be used in the work.

If concrete of adequate workability cannot be obtained by the use of the minimum cement content as would be allowed, the cement and water content shall be increased without exceeding the specified water/cement ratio, or an approved admixture shall be used.

10.1.6 Durability of concrete

Special exposures

For concrete intended to have low permeability when exposed to water, the water cement ratio shall not exceed 0.50.

For corrosion protection of reinforced concrete exposed to brackish water, sea water or spray from these sources, the water cement ratio shall not exceed 0.40.

If minimum requirement of concrete cover as given under the Section on ‘Reinforcing Steel’ is increased by 12mm, water cement ratio may be increased to 0.45.
The requirement of water cement ratio on Normal Weight Aggregate Concrete, if exposed to Sulphate containing solutions, shall be calculated using the weight of cement meeting the requirements of ASTM C 150 or C 595 plus the weight of fly ash or pozzolan satisfying ASTM C 618 and/or slag satisfying ASTM C 989.

**Sulphate exposures**

Concrete to be exposed to sulphate containing solutions or soils shall conform to the requirements of the Table given below or be made with a cement that provides sulphate resistance with the maximum water cement ratio provided in the Table.

Calcium chloride shall not be used as an admixture in concrete exposed to severe or very severe sulphate containing solutions, as defined in Table given below.

### Requirements for Normal Weight Aggregate Concrete Exposed to Sulphate Containing Solutions

<table>
<thead>
<tr>
<th>Sulphate exposure</th>
<th>Water Soluble Sulphate (SO₄) in soil (percent by weight)</th>
<th>Sulphate (SO₄) in water (ppm)</th>
<th>Cement Type¹</th>
<th>Maximum Water Cement Ratio, by weight</th>
</tr>
</thead>
<tbody>
<tr>
<td>Negligible</td>
<td>0.00-0.10</td>
<td>0-150</td>
<td>II, IP(MS), IS(MS), P(MS), I(PM) (MS)</td>
<td>0.50</td>
</tr>
<tr>
<td>Moderate²</td>
<td>0.10-0.20</td>
<td>150-1500</td>
<td>II (SM) (MS)</td>
<td></td>
</tr>
<tr>
<td>Severe</td>
<td>0.20-2.00</td>
<td>1500-10,000</td>
<td>V</td>
<td>0.45</td>
</tr>
<tr>
<td>Very severe</td>
<td>Over 2.00</td>
<td>Over 10,000</td>
<td>V plus pozzolan³</td>
<td>0.45</td>
</tr>
</tbody>
</table>

**Note:**
1. For types of cement see ASTM C150 and C595.
2. Sea water
3. Pozzolan that has been determined by test or service record to improve Sulphate resistance when used in concrete containing Type V cement.

### Corrosion of reinforcement

For corrosion protection, maximum water soluble Chloride ion concentrations in hardened concrete at ages from 28 to 42 days contributed from the ingredients including water, aggregates, cementitious materials, and admixtures, shall not exceed the limits specified in the Table given below. When testing is performed to determine water soluble Chloride ion content, test procedures shall conform to AASHTO T 260, “Methods of Sampling and Testing for Total Chloride Ion in Concrete and Concrete Raw Materials”.

### Maximum Chloride Ion Content for Corrosion Protection

<table>
<thead>
<tr>
<th>Type of member</th>
<th>Maximum water soluble Chloride ion (C₁) in Concrete, percent by weight of cement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Reinforced concrete exposed to chloride in service</td>
<td>0.15</td>
</tr>
<tr>
<td>Reinforced concrete that will be dry or protected from moisture in service</td>
<td>1.00</td>
</tr>
<tr>
<td>Other reinforced concrete construction</td>
<td>0.30</td>
</tr>
</tbody>
</table>

When reinforced concrete will be exposed to brackish water, sea water, or spray from these sources, the above requirements for water cement ratio, or concrete strength and minimum cover requirements (shown under the relevant Sub-section of the Section on ‘Reinforcing Steel’) shall be satisfied.
10.1.7 Design of concrete mix

When designing the concrete mix, the following conditions shall be considered:

Strength : The class of the concerned concrete is to be as shown on the Drawings (if shown). The class is the specified cylinder strength of 28 days and shall be determined as indicated above in the Table under the Sub-section on 'Concrete Classes' of this Section.

Water/Cement Ratio : The ratio of free water to cement when using saturated surface dry aggregate shall be as low as possible and not to exceed 0.50 by weight for all concrete. For concrete in pile caps in contact with the ground, the water cement ratio shall not exceed 0.45.

Cement Type and Minimum Content : Type-1 Cement shall be used for all classes for “Concrete”.

Minimum Filler Content : Filler is defined as fine concrete aggregates including cement with a grain diameter less than 0.25mm. It shall not be less than (except mass concrete) 435 Kg per cubic meter Concrete for maximum 20mm size Coarse Aggregate. The same for maximum 40mm size Coarse Aggregate shall not be less than 350 kg per cubic meter of Concrete.

Coarse Aggregate : The maximum size of the coarse aggregate shall be either 40mm or 20mm and the grading and quality shall be as indicated in the portion of ‘Coarse Aggregate’ under the Sub-section on ‘Construction Materials’ of this Specification or as specified on the Drawings or as directed by the Engineer.

Fine Aggregate : The grading and quality is to be as indicated in the portion of Sub-section on ‘Fine Aggregate’ under the Section on ‘Construction Materials’ of this Specification or as specified on the Drawings or as directed by the Engineer.

Workability : The concrete shall be of suitable workability to obtain full compaction. Slumps measured by STP T 8.1.1 shall be in accordance with the values shown unless otherwise required or approved by the Engineer.

The designed concrete mix shall be approved by the Engineer to meet the requirements for each structural component.

Prior to the commencement of concrete operations, the Contractor shall design a mix for the concrete and prepare and test concrete samples of this mix under laboratory conditions. Preliminary mixes shall be repeated and adjusted as necessary to produce a concrete mix meeting the requirements stated under the Sub-section on “Composition of Concrete” of this Specification. The details of the mix and test results shall be submitted to the Engineer for his approval.

Following the Engineer’s approval of the mix design, the Contractor shall prepare a trial mix in the presence of the Engineer. The trial mix shall be batched, mixed and handled using the same methods and plant, the Contractor proposes to use. The mix shall comprise not less than half a cubic meter of concrete. The proportions of cement, aggregates and water shall be carefully determined by weight in accordance with the Contractor’s approved mix design and sieve analysis shall be made for the fine and coarse aggregates.
Twelve concrete cylinder samples shall be made from the trial mix in the presence of the Engineer. The concrete cylinders shall be made, cured, stored and tested in accordance with BS 1881 or any other equivalent Standards. Six cylinders shall be tested at 7 days and six cylinders shall be tested at 28 days. If the strength of any of the cylinders tested at 28 days is recorded below the characteristic strength, the Contractor shall redesign the mix, make further preliminary mixes for the Engineer’s approval. He shall then undertake additional trial mixes and test the resultant samples until a satisfactory mix is obtained and approved by the Engineer.

From the same mix as that from which the test specimens are made, the workability of the concrete shall be determined by the slump test in accordance with STP T 8.1.1. The remainder of the mix shall be cast in a wooden mould and compacted. After 24 hours, the sides of the mould shall be struck off and the surface examined in order to satisfy the Engineer that an acceptable surface can be obtained with the mix.

When a proposed mix has been approved, no variation shall be made in the mix proportions, or in the type, size, grading zone or source of any of the constituents without the consent of the Engineer. He may require further trial mixes to be made before any such variation is approved.

Until the Engineer approves the results of trial mixes for a particular class of concrete, no concrete of the relevant class shall be placed in the works.

During production, the Engineer may require additional trial mixes before a substantial change is made in the materials or in the proportions of the materials to be used. However, it will not need to be carried out when adjustments are made to the mix proportions during production in order to minimize the variability of strength and to approach more closely the target mean strength.

Trial mixes for mass concrete are not requested provided the Contractor is able to submit test results from mixes carried out before which prove that the demanded quality of the mass concrete is according to the Specifications.

When the Contractor intends to purchase factory-made pre-cast concrete units, trial mixes may be dispensed with provided that evidence is given to satisfy the Engineer that the factory regularly produces concrete, which complies with the Specifications. The evidence shall include details of mix proportions, water-cement ratios, slump tests and strengths obtained at 28 days.

Selection of the trial mix is the ultimate responsibility of the Contractor regardless of its approval accorded by the Engineer

10.1.8 Proportioning of mix

Proportions of materials for concrete shall be such that:

a) Workability and consistency are achieved for proper placement into forms and around reinforcement, without segregation or excessive bleeding.

b) Resistance to special exposures to meet the durability requirements are provided, and

c) Conformance with strength test requirements is ensured.

The approved mix shall be proportioned by weight or, except cement by volume, if volume batching is approved by the Engineer. Allowance shall be made for the moisture content of the aggregates.
Fine and coarse aggregates and water may only be measured by volume in boxes or containers approved by the Engineer. Cement shall be added to Concrete Mixer by whole number of bags only.

10.1.9 Concrete in blinding layers

The blinding concrete/lean concrete (Mix 1:3:6) shall be mixed in proportion by volume wherever specified on the Drawings. Ordinary Portland Cement and well-graded aggregate of maximum nominal size, not exceeding 40mm, shall be used unless otherwise specified.

10.1.10 Batching

General

The Contractor shall provide and maintain suitable measuring equipment and devices of good order required to determine and control accurately the relative amount of various materials entering the mix.

All measurements shall be by weight/volume and shall be accurate within a tolerance of 1% for each batch. Besides, the deviation from the average amount of filler from ten samples of different batches of fresh concrete should not be more than 6%.

Satisfactory methods of handling materials shall be employed.

A batching plant shall be used for measuring materials but alternative methods proposed by the Contractor may be considered subject to the approval of the Engineer. The batching plant shall include bins, weighing hoppers and scales for the fine aggregate and for each separated size of coarse aggregate. If cement is used in bulk, a bin, hopper and scales for the cement shall be included. The container shall be watertight.

Provisions satisfactory to the Engineer shall be made for batching other components of the mix at the batching plant or at the mixer, as may be necessary. The batching plant may be either of stationary or of mobile type. It shall always be properly leveled within the accuracy required for the proper operation of the weighing mechanisms.

Bins and hoppers

Bins with adequate separate compartments for fine aggregate and for each required size of coarse aggregate shall be provided in the batching plant. Each compartment shall discharge efficiently and freely into the weighing hopper. Means of control shall be provided so that as the quantity desired in the weighing hopper is being approached, the material may be added slowly and shut off with precision. A port or other opening for removing an overload of the several materials from the hopper shall be provided.

Weighing hoppers shall be constructed so as to discharge fully.

Scales

The scales for weighing aggregates and cement shall be of either the beam type or the dial type without spring. They shall be accurate within one-half of 1% under operating conditions throughout the range of use. Ten 25 kilogram weights shall be available for checking the accuracy. All exposed fulcrums, clevises and similar working parts of scales shall be kept clean. When beam-type scales are used, provision shall be made for indicating to the operator that the required load in the weighing hopper is being approached. The device shall indicate at least the last 100 kilograms of load and up to 25 kilograms over-load. All weighing and indicating devices shall be in full view of the operator while charging the hopper and he shall have convenient access to all controls.
Cement may be measured by weight, or in standard bags weighing 50 kilograms net each. When measured by weight, a separate satisfactory scale and hopper shall be provided together with a boot or other approved device to transfer the cement from the weighing hopper.

The amount of water shall be measured by weight separately on an individual scale or may be measured by volume.

Any solid admixture, to be added, shall be measured by weight. However, liquid or pest admixtures may be measured by volume or weight.

10.1.11 Quality control of concrete

General
The Contractor shall assume the full responsibility that the quality of the concrete conforms to the Specifications and this responsibility shall not be waived by the tests carried out and the test results approved by the Engineer. The Contractor shall thus at his own discretion establish additional testing procedures as necessary.

The Contractor shall be responsible for providing samples of concrete and its constituent materials either for testing by himself or for testing at the Engineer’s laboratory or laboratory designated by the Engineer. For this purpose, concrete test cylinders, which shall be made in accordance with BS 1881/ASTM C 31 shall be deemed to be ‘Samples’. All sampling of constituent materials shall be carried out in accordance with the provisions of the appropriate British/American Standard and all sampling of fresh and of hardened concrete shall be carried out in accordance with the provisions of BS 1881/ASTM C 31 unless such provision is at variance with the Specification.

The tests, which the Contractor is required to undertake on behalf of the Engineer, are those to be carried out on fresh concrete at the place of final deposit, or elsewhere at Site as directed by the Engineer.

Adjustment of water/cement ratio
The Contractor shall test aggregates for moisture content and so determine the water-cement ratio of the fresh concrete. Determination of water-cement ratio shall be carried out as required by the Engineer and the results and calculations shall be submitted to him.

Slump tests
Slump testing of concrete shall be carried out as required by the Engineer. The minimum is one test at the commencement of each casting, one per hour of casting and one each time a strength test specimen is taken.

The Engineer shall make available a slump cone at Site and the testing shall be carried out in accordance with STP T 8.1.

The slump of concrete to be used in the works shall not exceed the slump of the trial mix by more than 10% and shall in any case be not more than the maximum specified.

Compressive strength
The Contractor shall, in the presence of the Engineer, sample concrete for testing from the batching and mixing plant at the time of pouring of concrete into the forms or elsewhere. Samples shall be obtained at uniform intervals throughout the production or delivery of concrete for a given placement.
The Contractor shall carry out cylinder testing of concrete strength as required by the Engineer. A minimum of three test cylinders shall be taken for each day’s casting or for every 15 cubic meters of concrete cast in larger pours.

After stripping, each cylinder shall be indelibly marked with the date of taking cylinder, location in the structure and prescribed number.

The Engineer shall make available 2 sets of three test moulds (cylinder) at Site. Samples for testing shall be taken in the presence of the Engineer and shall be dated.

Test cylinder shall be tested for 7 days and 28 days compressive strength in accordance with STP T 8.2.

A strength test result shall be the average of the strengths of two cylinders made from the same sample of concrete and tested at 28 days. Strength level of an individual class of concrete shall be considered satisfactory if both of the following requirements are met:

a) Average of three consecutive strength tests equals or exceeds the specified strength.

b) No individual strength test (average of two cylinders) falls below the specified strength by more than 3.5 N/mm².

However, the following may be an alternative –

The average strength of the three consecutive cylinders, tested at 28 days, shall exceed the specified strength. One out of the three cylinders tested may have a value less than the specified strength provided that it is not less than 85% of the specified strength, except that not more than one test result per element may be below the specified strength.

**Failure to pass tests**

If cylinders taken at Site during the progress of the works fail to reach the specified strength, no further pouring of concrete shall take place until the cause of the failure has been established and corrective measures have been taken to the satisfaction of the Engineer.

The Engineer may require that core samples are taken and tested in accordance with ASTM C 42 or a similar standard or other tests be performed on sections of the works made from the suspect concrete. If such tests fail to demonstrate the integrity of the sections of the works, all sections made with the suspect concrete shall be removed from the Site. Costs of all such tests and removal of concrete including the cost of the concrete shall be borne by the Contractor.

**Testing hardened concrete**

Entire operation shall be carried out as per the approval of the Engineer with due precaution so that the structural integrity is no way affected. The Contractor shall remain responsible for any negligence. If approved by the Engineer, on each specific occasion, hardened concrete liable to rejection shall be tested for compressive strength in accordance with ASTM C 42 at the Contractor's expenses. Unless otherwise directed, cores shall be 150mm in diameter. At least three specimens shall be cored and tested from the locations as directed by the Engineer.
If the average compressive strength of the core specimens, so obtained, is equal to or greater than 85% of the specified 28-days cylinder, compressive strength for that section of the work (the concrete represented by the core specimen) shall be considered to be structurally satisfactory.

If the concrete is considered to be structurally satisfactory, the holes left by the removal of the test cores shall be appropriately repaired or as directed by the Engineer. Unless otherwise directed, concrete that will fail to meet the requirements of the Specifications shall be removed and replaced in an approved manner without any extra costs to the Employer.

10.1.12 Concrete construction

10.1.12.1 General

The Contractor shall, in due time and as soon as possible, submit his proposed construction methods and work programme along with Shop Drawings to the Engineer and shall obtain his approval before commencement of any works.

The Contractor shall maintain an adequate number of trained and experienced supervisors and foremen at the Site to supervise and control the Work.

10.1.12.2 Mixing concrete

General

All concrete shall be mixed in batch mixers. It may be mixed at the Site of construction, at a central plant, or in transit. Each mixer shall have attached to it, in a prominent place, a manufacturer’s plate showing the capacity of the drum in terms of mixed concrete and the speed of rotation of the mixing drum.

Mixers at the Site of construction

Mixers at local Sites shall be approved drum-type capable of combining the aggregate, cement, and water into a thoroughly mixed and uniform mass within the specified mixing period and of discharging the mixture without segregation. The mixer shall be equipped with a suitable charging hopper, water storage and a water-measuring device, accurate within 1%. Controls shall be so arranged that the water can be applied only while the mixer is being charged. Suitable equipment for discharging the concrete shall be provided. The mixer shall be cleaned at suitable intervals. The pickup and throw over blades in the drum shall be replaced when they have lost 10% of their depth.

The mixer shall be operated at a drum speed of not less than 15 nor more than 20 revolutions per minute at the recommended speed of the manufacturer. The batched materials shall be so charged into the drum that a portion of the water shall enter in advance of the cement and aggregates and the water shall continue to flow into the drum for a minimum time of 5 seconds after all the cement and aggregates are in the drum. Mixing time shall be measured from the time all materials, except water, are in the drum and shall, in the case of mixers having a capacity of 1 cubic meter or less, not be less than 50 seconds nor more than 70 seconds. Mixing shall be continued for at least 90 seconds after all materials are in the drum, unless a shorter time is shown to be satisfactory by the mixing uniformity tests of ‘Specification for Ready Mixed Concrete’ (ASTM C 94). In the case of dual drum mixers, the mixing time shall not include transfer time. The contents of an individual mixer drum shall be removed before a succeeding batch is emptied therein. Any concrete mixed less than the specified minimum time shall be discarded and disposed of by the Contractor at his own expenses.

The volume of concrete, mixed per batch, shall not exceed the mixer’s nominal capacity in cubic meters as shown on the manufacturer’s guaranteed capacity standard rating plate on the mixer. However, an
overload upto 20% of the mixers nominal capacity may be permitted provided concrete test data for strength, segregation and uniform consistency are satisfactory, and provided no spillage of concrete takes place.

Re-tempering concrete by adding water or by other means shall not be permitted. Concrete, which is not of the required consistency at the time of placement, shall not be used.

Central plant mixers

These mixers shall be of approved drum type capable of combining the aggregate, cement and water into a thoroughly mixed and uniform mass within the specified mixing period and of discharging the mixture without segregation. Central plant mixers shall be equipped with an acceptable timing device that will not permit the batch to be discharged until the specified mixing time has elapsed. The water system for a central mixer shall be either a calibrated measuring tank or a meter and shall not necessarily be an integral part of the mixer.

The mixer shall be cleaned at suitable intervals. It shall be examined daily for changes in interior condition. The pick-up and throw-over blades in the drum shall be replaced when they have lost 10% of their depth.

Central plant mixers, which have a capacity of 2-5 cubic meters and greater than 5 cubic meters, may permit a minimum mixing time of 90 seconds and 120 seconds respectively provided a mixing analysis and tests of the job materials indicate such produced concrete is equivalent to strength and uniformity to that attained as stated in the preceding paragraphs.

Hand mixing

The Engineer shall normally not allow any hand mixing in the preparation of concrete. However, under some special circumstances, exigencies and for small works, it may be compelling to allow hand mixing while preparing the concrete. In the case hand mixing is allowed, the procedures stated below shall be followed in a chronological order:

- Water-tight platform should be constructed with cement concrete or bricks. The size of the platform shall be such that it will be possible to accommodate the requisite quantity of mixture in a single batch. The materials of a single batch should be calculated out carefully.

- The requisite quantity of sand, being determined at a certain proportion, should be measured in a wooden box of specified size and to be spread on the platform with uniform thickness and the top is to be leveled. The requisite quantity of cement should also be measured and spread with uniform thickness over the stack of sand.

- Sand and cement as stacked above shall have to be mixed up by reversing with spade starting from one end and progressing towards the other. This procedure to be carried on carefully, thoroughly and repeatedly in such a manner that the mixture ultimately turns into a uniform colour and density. The mixture should then be stacked in a heap on a portion of the platform.

- The requisite quantity of coarse aggregate should then be stacked on the left out spaces of the platform and the top surface be leveled. The previously mixed sand and cement mixture shall than be spread with uniform thickness over the coarse aggregate. The height of these two layers in combination should better not to exceed 250mm. They are then to be thoroughly mixed with spade for several times. In each time, the mixing should proceed from one end. The mixing shall be continued until the mixture takes a uniform colour and density. The mixture will than be stacked with uniform height and leveled (the height of the stack may normally be maintained at 250mm).
The top surface of the stack will then be shaped concave and the requisite amount of water to be poured in. It is then to be thoroughly mixed with spade with caution and as quickly as possible. The mixing shall be continued till the mixture takes a uniform colour and density. The mixture shall invariably be conveyed, placed, compacted and to be given the final shape within 45 minutes on mixing.

10.1.12.3 Conveying concrete

**General**

Concrete shall be conveyed from the mixer/batching plant to the place of final deposit as rapidly as possible by methods that will prevent segregation or loss of materials. Conveying equipment shall be capable of providing a supply of concrete to the place of deposit without segregation of ingredients and without interruptions sufficient to permit loss of plasticity between successive increments. Re-mixing of concrete shall not be allowed. Concrete, which does not reach its final position in the forms within the stipulated time, shall not be used.

Mixed concrete shall be transported from the central mixing plant to the work Site in agitator trucks or upon written permission by the Engineer in non-agitator trucks. Delivery of concrete shall be so regulated that placing is at a continuous rate unless delayed by the placing operations. The intervals between delivery of batches shall not be so great as to allow the concrete in place to harden partially, and in no case such an interval shall exceed 30 minutes.

**Agitator trucks**

Unless otherwise permitted in writing by the Engineer, agitator trucks may be used for transportation of central plant mixed concrete. Agitator trucks shall have watertight revolving drums suitably mounted and shall be capable of transporting and discharging the concrete without segregation. The agitating speed of the drum shall not be less than 2 or more than 6 revolutions per minute. The volume of the mixed concrete permitted in the drum shall not exceed the manufacturer's rating nor exceed 80% the gross volume of the drum.

Upon approval by the Engineer, open-top revolving-blade truck mixers may be used in lieu of agitator trucks for transportation of central plant mixed concrete.

The interval between introduction of water into the mixer drum and final discharge of the concrete from the agitator shall not exceed 45 minutes. During this interval the mix shall be agitated continuously.

**Non-agitator-trucks**

Bodies of non-agitating equipment shall be smooth, watertight metal containers equipped with gates that will permit control of the discharge of the concrete. Covers shall be provided when needed for protection against weather.

The non-agitating equipment shall permit delivery of the concrete to the work Site in a thoroughly mixed and uniform mass with a satisfactory degree of discharge.

Uniformity shall be satisfactory, if samples from the one-quarter and three-quarter points of the load do not differ by more than 30mm in slump. Discharge of concrete shall be completed within 30 minutes after the introduction of the mixing water to the cement and aggregate.

**Truck or transit mixers**

These shall be equipped with electrically actuated counters by which the number of revolutions of the drum or blades may readily be verified and the counters shall be actuated at the commencement of
mixing operations at designated mixing speeds. The mixer when loaded shall not be filled to more than 60% of the drum gross volume. The mixer shall be capable of combining the ingredients of the concrete in to a thoroughly mixed and uniform mass and of discharging the concrete with a satisfactory degree of uniformity.

Except when intended for use exclusively as agitators, truck mixers shall be provided with a water-measuring device to measure accurately the quantity of water for each batch. The delivered amount of water shall be within plus or minus 1% of the indicated amount.

Truck mixers may be used for complete mixing at the batch plant and as truck agitators for delivery of concrete to job Site or they may be used for complete mixing of the concrete at the job Site. They shall either be a closed watertight revolving drum or an open top revolving blade or paddle type.

The amount of mixing shall be designated in number of revolutions of the mixer drum. When a truck mixer is used for complete mixing, each batch of concrete shall be mixed for not less than 70 nor more than 100 revolutions of the drum or blades at the rate of rotation designated by the manufacturer of the equipment as the “mixing speed”. Such designation shall appear on a metal plate attached to the mixer. If the batch is at least 0.5 cubic meter less than guaranteed capacity, the number of revolutions at mixing speed may be reduced to not less than 50. Mixing in excess of 100 revolutions shall be at the agitating speed. All materials, including the mixing water, shall be in the mixer drum before actuating the revolution counter, which will indicate the number of revolutions of the drum or blades.

When wash water (flush water) is used as a portion of the mixing water for the succeeding batch, it shall be accurately measured and taken into account in determining the amount of additional mixing water required. When wash water is carried on the truck mixer, it shall be carried in a compartment separate from the one used for carrying or measuring the mixing water. The Engineer will specify the amount of wash or flush water and may specify a “dry” drum, if wash water is used without measurement or without supervision.

When a truck is used for complete mixing at the batch plant, mixing operations shall begin within 30 minutes after the cement has been added to the aggregate. After mixing, the truck mixer shall be used as an agitator, at the speed designated as agitating speed by the manufacturer of the equipment. Concrete discharge shall be completed within 45 minutes after the addition of cement to the aggregates. Each batch of concrete, delivered at the job Site, shall be accompanied by a time slip issued at the batching plant, bearing the time of departure therefrom. When the truck mixer is used for the complete mixing of the concrete at the job Site, the mixing operation shall begin within 30 minutes after cement has been added to the aggregates.

The rate of discharge of the plastic concrete from the mixer drum shall be controlled by the speed of rotation of the drum in the discharge direction with the discharge gate fully opened.

10.1.12.4 Handling and placing of concrete

Concrete placing shall not be commenced without the written approval of the Engineer or his representative. This approval shall be in the form of a standard checklist approved by the Engineer prior to the commencement of the Work. The checklist shall be filled in and approved by the Engineer or his representative during his inspection and acceptance of materials, plant and equipment, concrete pouring arrangements, the positioning, fixing and condition of reinforcement and any other items to be embedded including the cleanliness, alignment and suitability of the containing surfaces or formwork.

The temperature of concrete at the time of placing shall not exceed 35°C.
In preparation for the placing of concrete all sawdust, chips and other construction debris and extraneous matter shall be removed from the interior of forms. Struts, stays and braces, serving temporarily to hold the forms in correct shape and alignment, pending the placing of concrete at their locations, shall be removed when the concrete placing has reached an elevation rendering their service unnecessary. These temporary members shall entirely be removed from the forms and not be buried in the concrete. The concrete shall be placed in the position and sequences indicated on the Drawings, and Specification or as directed by the Engineer. The concrete shall be placed in clean, oiled formwork and compacted before initial set has occurred. In any event concrete shall not be placed later than 30 minutes from the time of mixing.

Concrete shall be placed in horizontal layers and each layer shall not be more than 600mm thick except as hereinafter provided. When less than a complete layer is placed in one operation, it shall be terminated in a vertical bulkhead. Each layer shall be placed and compacted before the preceding batch has taken initial set to prevent injury to the green concrete and avoid surfaces of separation between the batches. Each layer shall be compacted so as to avoid the formation of a construction joint with a preceding layer that has not taken the initial set.

The concrete shall be deposited as far as possible in its final position without re-handling or segregation and in such a manner so as to avoid displacement of the reinforcement and other embedded items or formwork.

Open troughs and chutes shall be of metal or metal line. The use of long troughs, chutes and pipes for conveying concrete from the mixer to the forms shall be permitted only on written authorization of the Engineer. Where chutes are used to convey the concrete, their slopes shall not be such as to cause segregation. Where long steep slopes are required, the chutes shall be equipped with baffles or be in short lengths that reverse the direction of movement. In case an inferior quality of concrete is produced by the use of such conveyors, the Engineer may order discontinuation of their use and the installation of a satisfactory method of placing.

Pneumatic placing of concrete shall be permitted only if authorized by the Engineer. The equipment shall be so arranged that a vibration does not damage freshly placed concrete.

Where concrete is conveyed and placed by pneumatic means, the equipment shall be suitable in kind and adequate in capacity for the work. The machine shall be located as close as practicable to the place of deposit. The position of the discharge end of the line shall not be more than 3m from the point of deposit. The discharge lines shall be horizontal or inclined upwards from the machine. At the conclusion of placement, the entire equipment shall be thoroughly cleaned.

Placement of concrete by pumping shall be permitted only if authorized by the Engineer. The equipment shall be so arranged that vibrations do not damage freshly placed concrete. Where concrete is conveyed and placed by mechanically applied pressure, the equipment shall be suitable in kind and adequate in capacity for the work. The operation of the pump shall be such that a continuous stream of concrete without air pockets is produced. When pumping is completed, the concrete remaining in the pipeline, if it is to be used, shall be ejected in such a manner that there is no contamination of the concrete or separation of the ingredients. After this operation, the entire equipment shall be thoroughly cleaned.

For simple spans, concrete shall preferably be deposited by beginning at the centre of the span and working from the centre towards the ends. Concrete in girders shall be deposited uniformly for the full length of the girder and brought up evenly in horizontal layers. For continuous spans, the concrete placing sequence shall be as shown on the plans or agreed by the Engineer.
Concrete in slab and girder haunches less than 1m in height shall be placed at the same time as that in the girder stem.

Concrete in slab spans shall be placed in one continuous operation for each span unless otherwise provided.

Concrete in T-beam or deck girder may be placed in one continuous operation, if permitted by the Engineer.

Concrete in columns and pier shafts shall be placed in one continuous operation unless otherwise directed.

Unless otherwise permitted by the Engineer, no concrete shall be placed in the superstructure until the column forms have been stripped off sufficiently to determine the character of the concrete in the columns. The load of the superstructure shall not be applied to the supporting structures until they have been in place at least 14 days unless otherwise permitted by the Engineer.

When the placing of concrete is temporarily discontinued, the concrete, after becoming firm enough to retain its form, shall be cleaned of Latinate and other objectionable materials to a sufficient depth to expose sound concrete. To avoid visible joints as far as possible upon exposed faces, the top surface of the concrete adjacent to the forms shall be smoothen with a trowel. Where a “feather edge” might be produced at a construction joint, an inset form shall be used to produce a blocked out portion in the preceding layer which shall produce an edge thickness of not less than 150mm in the succeeding layer. Work shall not be discontinued within 450mm of the top of any face unless provision has been made for a coping less than 450mm thick, in which case, if permitted by the Engineer, a construction joint may be made at the under side of the coping.

Immediately following the discontinuance of placing concrete, all accumulations of mortar splashed upon the reinforcement steel and the surfaces of forms shall be removed. Dried mortar chips and dust shall not be puddled into the unset concrete. If the accumulations are not removed prior to the concrete becoming set, care shall be exercised not to injure or break the concrete-steel bond at and near the surface of the concrete while cleaning the reinforcement steels.

Where concrete is required to be placed against undisturbed ground, the entire space between the finished concrete surface and the ground, including any over-break, is to be completely filled with concrete of the specified class. The concrete shall be well rammed and compacted to ensure that all cavities are filled and the concrete is everywhere in contact with the ground. Where permitted by the Engineer, any extensive patches of over-break may first be filled with concrete belonging to the appropriate Class as directed by the Engineer.

Where concrete is required to be placed against a metal surface to which it is required to adhere, care shall be taken to work the concrete well into the re-entrant angles and to ensure contact by hammering the metal part on its free side provided that this is done without damaging the metal or its protective coating, if any.

Concrete shall not be dropped through a height greater than 1200mm except with the approval of the Engineer who may order the use of bankers and the turning over of the deposited concrete by hand before being placed.

When placing operations would involve dropping the concrete more than 1200mm, it shall be deposited through sheet metal or other approved pipes. As far as practicable, the pipes shall be kept full of concrete during placing and their lower ends shall be kept buried in the newly placed concrete. After
initial set of the concrete, the forms shall not be jarred and no strain shall be placed on the ends of reinforcement bars, which are projected.

All chutes, troughs and pipes shall be kept clean and free from coatings of harden concrete by thoroughly flushing with water after each run. Water used for flushing shall be discharged clean.

The laying of concrete shall be carried out in such a way that the exposed faces of concrete shall be plain, smooth, sound and solid, free from honeycomb and excrescencies. After compaction the exposed concrete surface shall be struck off smooth with hand held steel floats. No plastering of imperfect concrete faces will be allowed. Any concrete that is defective in any way shall, if so ordered by the Engineer, be cut out and replaced to such depth or be made good in such manner as the Engineer may direct.

Construction joints shall be formed in the work where indicated on the Drawings or as previously approved by the Engineer. Where necessary, the Contractor shall allow for working beyond ordinary working hours to allow each section of concrete to be completed in a continuous pour with the placing of concrete carried upto each construction joint.

10.1.12.5 Depositing concrete under water

Concrete shall not be deposited in water except with the approval of the Engineer and under his immediate supervision and in this case the method of placing shall be as defined in this portion.

Concrete deposited in water shall be with 10 percent excess cement. It shall be carefully placed in a compact mass in its final position by means of Tremie, a bottom opening bucket or other approved methods and shall not be disturbed after being deposited. Special cares must be exercised to maintain still water at the point of deposit. Concrete shall not be placed in running water. The method of depositing concrete shall be so regulated as to produce approximately horizontal surfaces. The forms under water shall be watertight.

The discharge end of the Tremie shall be closed at the start of work so as to prevent water entering the tube and shall be entirely sealed at all times. The Tremie tube shall be kept full to the bottom of the hopper. When a batch is dumped into the hopper, the flow of concrete shall be induced by slightly raising the discharge end, always keeping it in the deposited concrete. The flow shall be continuous until the work is completed. Concrete slump shall be in between 100mm and 150mm.

Depositing of concrete by the opening bucket method shall conform to the following specifications. The top of the bucket shall be open. The bottom doors shall open freely downward and outward when tripped. The bucket shall be completely filled and slowly lowered to avoid backwash. It shall not be dumped until it rests on the surface upon which the concrete is to be deposited. When discharged, it shall be withdrawn slowly until it goes well above the concrete.

10.1.12.6 Compaction of concrete

Concrete, during and immediately after depositing, shall be thoroughly compacted. The compaction shall be done by mechanical vibration subject to the following provisions:

- The vibration shall be internal unless special authorization of other methods are given by the Engineer or as provided herein.
- Mechanical vibrators of the capacity as approved by the Engineer shall be used in conjunction with or without hand rammers, pokers or any other means as directed by the Engineer.
Vibrators shall be of a type and design as approved by the Engineer. They shall be capable of transmitting vibration to the concrete at frequencies of not less than 4,500 impulses per minute.

The intensity of vibration shall be such as to visibly affect a mass of concrete of 20mm slump over a radius of at least 450mm.

Vibrators must be operated by skilled workmen engaged/appointed by the Contractor mainly for this job.

Surface vibrators of the type of Pan-vibrators, or vibrating screens shall be used for compacting castings of shallow depth as directed by the Engineer.

The Contractor shall provide a sufficient number of vibrators to properly compact each batch immediately after it is placed in the forms. Spare vibrators shall be readily on hand in case of breakdown.

Vibrators shall be manipulated so as to thoroughly work the concrete around the reinforcement and embedded fixtures, and into the corners and angles of the forms.

Vibration shall be applied at the point of deposit and in the area of freshly deposited concrete. The vibrators shall be inserted and withdrawn from the concrete slowly. The vibration shall be of sufficient duration and intensity to thoroughly compact the concrete, but shall not be continued so as to cause segregation. Vibration shall not be continued at any one point, to the extent that localized areas of grout are formed.

While using immersion vibrators in walls, these should be lowered to the bottom of the wall before depositing of concrete is started and pulled up as it proceeds. When using vibrators, concrete can be placed from bottom to top of wall in one process, provided it is laid in regular layers. Cares should be taken to ensure that vibrators are not trapped under a great depth of concrete.

Application of vibrators shall be at points uniformly spaced and not further apart than twice the radius over which the vibration is visibly effective.

Vibration shall not be applied directly or through the reinforcement to sections or layers of concrete, which have hardened to the degree that the concrete ceases to be plastic under vibration. It shall not be used to make concrete flow in the forms over distances so great as to cause segregation, and vibrators shall not be used to transport concrete in the forms.

Vibration shall be supplemented by such spading as is necessary to ensure smooth surface and dense concrete along form surfaces and in corners and locations impossible to reach with the vibrators.

In columns, deep beams and walls mild striking by mallets at the outer faces of the form works should also be done simultaneously during use of vibrator for compaction.

The provisions of this Sub-section shall also apply to pre-cast piling, concrete cribbing and other pre-cast members except that the manufacturer's methods of vibration may be used, if approved by the Engineer.

10.1.13 Protection of concrete from adverse conditions

General

Concrete shall be protected from damage from the effects of sunshine, dry wind, rain, running water or mechanical damage for a continuous period, until the concrete has reached at least three quarters of its
28-days strength, but for not less than 10-days. Temperature of the concrete mixture shall require to be maintained between 10°C and 32°C unless otherwise provided herein. The Contractor shall submit his proposals to achieve this protection for the Engineer’s approval.

Damaged concrete shall be removed and replaced generally. However, it may be repaired to an acceptable condition if found appropriate by the Engineer.

**Protection from rain**

During rainy weather, proper protection shall be given to ingredients, production methods, handling and placing of concrete. If required in the opinion of the Engineer, the concrete depositing operation shall be postponed and newly placed concrete shall be protected from rain after forming proper construction joint for future continuation.

**Protection from hot weather**

During hot weather, proper attention shall be given to ingredients, production methods, handling, placing, protection, and curing to prevent excessive concrete temperatures or water evaporation that could impair required strength or serviceability of the member or structure.

Under a temperature above 32°C surfaces of forms, reinforcing steel, steel beam flanges etc. that remain in contact with the mix shall be cooled down below this temperature by means of water spray or by any other appropriate methods.

**Protection from cold weather**

Under a cold weather condition, temperature of the concrete shall be maintained not below 7°C during the curing period for the first six days on placement of concrete unless pozzolan cement or fly ash cement is used. Periods to be followed in the later case has been shown in the table given below:

<table>
<thead>
<tr>
<th>% of cement replaced by weight with pozzolans</th>
<th>Required period of controlled temperature</th>
</tr>
</thead>
<tbody>
<tr>
<td>10%</td>
<td>8 days</td>
</tr>
<tr>
<td>11-15%</td>
<td>9 days</td>
</tr>
<tr>
<td>16-20%</td>
<td>10 days</td>
</tr>
</tbody>
</table>

However, this requirement may be waived in the case the compressive strength of 65% of the specified 28-days design strength is achieved in 6-days.

If external heating is used in maintaining the requisite temperature, heat shall be applied and withdrawn gradually and uniformly so that the concrete surface is not heated more than 32°C.

Temperature of concrete at the time of placement in sections less than 300mm in thickness shall not be less than 16°C when the air temperature is below 2°C.

**Special requirements for roof slabs**

Prior to the application or curing, concrete being placed and finished for roof slabs shall be protected from damage due to rapid evaporation when the weather is low humid, windy or having high temperature. Such protection shall be adequate to prevent premature crusting of the surface or an increase in dry cracking. In providing such protection the humidity of the surrounding air shall be raised with fog sprayers operated upwind of the deck.
Concrete exposed to salt water

Unless otherwise specifically provided, concrete for structures exposed to salt water shall be mixed for a period of not less than 2 minutes and water content of the mixture shall be carefully controlled and regulated so as to produce concrete of maximum impermeability. The concrete shall be thoroughly consolidated as necessary to produce maximum density and a complete lack of rock pockets. Unless otherwise shown on the Drawings, the clear distance from the face of the concrete to the reinforcing steel shall not be less than 100mm. No construction joints shall be formed between levels of extreme low water and extreme high water or the upper limit of wave action as determined by the Engineer. Between these levels the forms shall not be removed, or other means provided to prevent salt water from coming in direct contact with the concrete for a period of not less than 30 days after placement. Except for the repair of any rock pockets and the plugging of form tie holes, the original surface, as the concrete comes from the forms, shall be left undisturbed. Special handling shall be provided for pre-cast members to avoid even slight deformation cracks.

10.1.14 Perforations and embedding of special devices

The Contractor is responsible for determining in advance of making any concrete pours, all requirements for perforation of concrete sections or embedding therein of special devices of other trades, such as conduits, pipes, weep holes, drainage pipes, fastenings, etc. Any concrete, poured without prior provision having been made, shall be subject to correction at the Contractor’s own expenses.

Devices to be embedded in the concrete shall be shown on the Drawings or directed by the Engineer.

Conduits, pipes and sleeves of any material not harmful to concrete and within the limitations specified herein shall be permitted to be embedded in concrete with the approval of the Engineer, provided they are not considered to replace structurally the displaced concrete.

Conduits and pipes of aluminium shall not be embedded in structural concrete unless effectively coated or covered to prevent aluminium concrete reaction or electrolytic action between aluminium and steel.

Conduits, pipes, and sleeves passing through a slab, wall, or beam shall not impair significantly the strength of the construction.

Conduits and pipes, with their fittings, embedded within a column, shall not displace more than 4% of the area of cross-section on which strength is calculated or which is required for fire protection.

Except when the Engineer approves Drawings for conduits and pipes, embedded conduits and pipes within a slab, wall or beam (other than those merely passing through) shall satisfy the following:

i. They shall not be larger in outside dimension than 1/3rd the overall thickness of slab, wall, or beam in which they are embedded.

ii. They shall not be spaced closer than 3 diameters or widths on centers.

iii. They shall not impair significantly the strength of the construction.

Conduits, pipes and sleeves shall be permitted provided that they are not exposed to rusting or other deterioration, have nominal inside diameter not over 50mm and are spaced not less than 3 diameters on centers.

Pipes and fittings shall be designed to resist effects of the material, pressure, and temperature to which they will be subjected.
No liquid, gas, or vapor excepting water, not exceeding 30°C nor 0.3 N/mm² pressure, shall be placed in the pipes until the concrete has attained its design strength.

Piping in solid slabs, unless it is for radiant heating, shall be placed between the top and bottom reinforcements.

Concrete cover for pipes, conduits, and fittings shall be not less than 40mm for concrete exposed to earth or weather nor 20mm for concrete not exposed to weather or in contact with the ground.

Reinforcement with an area not less than 0.002 times the area of concrete section shall be provided normal to piping.

Piping and conduit shall be so fabricated and installed that cutting, bending, or displacement of reinforcement will not be required.

10.1.15 Curing of concrete

General

In order to prevent loss of water, all newly placed concrete shall be cured by use of one or more of the methods specified herein. The Engineer shall select the method that should be followed for curing a concrete of particular type of work or member. Curing shall commence immediately after the free water has left the surface and finishing operations are complete. In the case the concrete surface begins to dry before the selected cure method is applied, the surface of the concrete shall be kept moist by a fog spray application so as to prevent any damages to the surfaces.

Curing by other than steam or radiant heat methods shall continue uninterrupted for at least 7 days except that when pozzolans in excess of 10 percent, by weight, of the Portland cement are used in the mix. When such pozzolans are used, the curing period shall be at least 10 days. For other than top slabs of structures, the above curing periods may be reduced and curing may be terminated when test cylinders, cured under the same conditions as the structure, indicate that concrete strength of at least 70 percent of that specified has been reached.

High early strength concrete shall be maintained above 10°C and in a moist condition for at least the first three days, except when cured in accordance with Accelerated Curing Method.

During periods of hot weather, water shall be applied to the concrete surfaces being cured by the liquid membrane method or by the forms-in-place method, if considered necessary by the Engineer. The process shall continue for a period that the Engineer determines a cooling effect is no longer required.

Materials

Water

Water used in curing of concrete shall be subject to approval and shall be reasonably clean and free of oil, salt, acid, alkali, sugar, vegetable, or other injurious substances. Water shall be tested in accordance with and shall meet the suggested requirements of AASHTO T 26. Where the source of water is relatively shallow, the intake shall be so enclosed as to exclude silt, mud, grass, or other foreign materials.

Liquid membranes

Liquid membrane forming compounds for curing concrete shall conform to the requirements of ASTM C 309.
Waterproof sheet materials

Waterproof paper, polyethylene film, and white burlap polyethylene sheet shall conform to the requirements of ASTM C 171.

Methods

Forms-in-place method

Formed surfaces of concrete may be cured by retaining the forms in place for the required time.

Water method

Concrete surface shall be kept continuously wet by ponding, spraying or covering with materials that are kept continuously and thoroughly wet. Such materials may consist of cotton mats, multiple layers of burlap or other approved materials, which do not discolor or otherwise damage the concrete.

Liquid membrane curing compound method

The liquid membrane method shall not be used on surfaces where a rubbed finish is required or on surfaces of construction joints unless it is removed by sand blasting prior to placement of concrete against the joint. Type 2 white pigmented liquid membranes may be used only on the surfaces that will not be exposed to view in the completed works or on surfaces where their use has been approved by the Engineer.

When membrane curing is used, the exposed concrete shall be thoroughly sealed immediately after the free water has left the surface. Formed surfaces shall be sealed immediately after the forms are removed and necessary finishing has been done. The solution shall be applied by power-operated atomizing spray equipment in one or two separate applications. Hand-operated sprayers may be used for coating small areas. Membrane solutions containing pigments shall be thoroughly mixed prior to use and agitated during application. If the solution is applied in two increments, the second application shall follow the first application within 30 minutes. Satisfactory equipment shall be provided, together with means to properly control and assure the direct application of the curing solution on the concrete surface so as to result in a uniform coverage at the rate of 4.5 liters for each 14 square meter of area.

If the film is damaged by inclement weather condition or in any other manner during the curing period and before the film has dried sufficiently, a new coat of the solution shall be applied to the affected portions equal in curing value to that specified above.

Waterproof cover method

This method shall consist of covering the surface with a waterproof sheet material so as to prevent moisture loss from the concrete. This method may be used only when the covering can be secured adequately to prevent moisture loss.

The concrete shall be wet at the time the cover is installed. The sheets shall be of the widest practicable width and adjacent sheets shall overlap a minimum of 150mm and shall be tightly sealed with pressure sensitive tape, mastic, glue, or other approved methods to form a complete waterproof cover of the entire concrete surface. The paper shall be secured so that wind will not displace it. Should any portion of the sheets be broken or damaged before expiration of the curing period, the broken or damaged portions shall be immediately repaired. Sections that have lost their waterproofing qualities shall not be used.
**Accelerated curing**

Curing by high-pressure steam, steam at atmospheric pressure, heat and moisture or other accepted processes, shall be permitted to accelerate strength gaining and reduce time of curing.

Accelerated curing shall provide a compressive strength of the concrete at the load stage considered, at least equal to the required design strength at that load stage.

Curing process shall be such, as to produce concrete with a durability at least equivalent to that obtained for concrete cured by the above methods.

The use of accelerated curing method for concrete containing other types of cement or any admixture shall be subject to the Engineer’s acceptance.

**Field cured specimens**

The Engineer may require strength tests of cylinders cured under field conditions to check adequacy of curing and protection of concrete in the structure.

Field cured cylinders shall be cured under field conditions in accordance with “Practice for Making and Curing Concrete Test Specimens in the Field” (ASTM C 31).

Field cured test cylinders shall be moulded at the same time and from the same samples as laboratory cured test cylinders.

Procedures for protecting and curing concrete shall be improved when the strength of field cured cylinders at the test age designated for determination of $f'_c$ is less than 85% of that of companion laboratory cured cylinders. The 85% limitation shall not apply, if field cured strength exceeds $f'_c$ by more than 3.5 N/mm².

**10.1.16 Finish and finishing**

**General**

Surface irregularities shall be classified as “abrupt” or “gradual”. Offsets caused by displaced or misplaced form sheathing or lining of form sections, or loose knots in forms or otherwise defective formwork, will be considered as “abrupt” irregularities. All other irregularities will be considered as gradual irregularities.

Where a surface is partly below and partly above the final ground level, the finish for the exposed surface shall extend for 0.15m below the ground level.

The formed surfaces, which will be permanently buried under earth, will require no treatment for abrupt or gradual irregularities. However, repair of defective concrete and filling of holes left by the removal of fasteners from the ends of tie rods shall be undertaken.

All abrupt and gradual irregularities on all exposed surfaces shall be removed by sack rubbing or sand blasting or grinding or by all these methods or any other methods approved by the Engineer, which is not harmful to the concrete. The permissible surface irregularities shall not exceed 6mm for abrupt irregularities and 13mm for gradual irregularities. The permissible irregularities may be reduced at places of the surface where, in the opinion of the Engineer, the formed finish does not provide the desired effect and no extra payment shall be permissible for such work.

Holes, honeycombs, or other defects left by forms shall be promptly repaired in accordance with the relevant Sub-section of this Specification.
All surfaces such as blinding concrete, opening for second stage concrete etc. on which concrete is to be placed subsequently, shall not be finished for abrupt or gradual irregularities.

Generally, concrete surface shall remain as cast and no plastering work will be performed on it. The formwork shall be lined with a material approved by the Engineer to provide a smooth finish of uniform texture and appearance. This material shall leave no stain on the concrete and shall be so joined and fixed to its backing that it imparts no blemishes. It shall be of the same type and obtained from only one source throughout any one structure. The Contractor shall repair any imperfections in the resulting finish as required by the Engineer for which no extra payment shall be made to him. Internal ties and embedded metal parts will be allowed only with the Engineer’s specific approval.

**Concrete surface finishing**

Skilled workmen shall perform finishing of concrete surfaces to the satisfaction of the Engineer. Exposed flat concrete surfaces shall be screed to produce an even and uniform surface and then they shall be given a trowel finish unless otherwise specified on the Drawings. All exposed and unprotected edges shall be given 20mm x 20mm chamfers.

The Concrete surface finish on upward facing, horizontal or sloping faces shall be, except for blinding concrete or otherwise stated on the Drawings, a “fair” surface. A ‘fair’ surface shall be obtained by screeding and trowelling with a wood float.

Screeding shall be carried out following compaction of the concrete by the slicing and tamping action of a screed board running on the top edges of the formwork or screeding guides to give a dense concrete skin true to line and level.

Wood float trowelling shall be carried out after the concrete has stiffened and the film moisture has disappeared. Working should be kept to the minimum compatible with a good finish and the surface shall be true to the required profile to fine tolerance. Whenever necessary, the Contractor shall provide and erect overhead covers to prevent the finished surfaces from being marred by rain drops or dripping water.

The surface of blinding concrete shall be obtained by screeding as described above.

Where a “fine” surface is indicated on the Drawings, this shall be obtained in a similar manner to “fair” surface except that a steel float shall be used in lieu of the wood float.

Formed surface for painting exposed to view shall be smooth and free from projections and shall be rubbed smooth immediately after the forms are removed. Formed surfaces shall be classified as follows:

- Unexposed concrete surfaces upon or against which backfill or concrete is to be placed, require no treatment except the removal and repair of defective concrete.
- Exposed surfaces shall have a very smooth, sound surface by control of formwork, concrete placement and repair of abrupt surface irregularities by grinding or rubbing of high spots and filling of voids.

**Ordinary finish**

An ordinary finish is defined as the finish left on a surface after the removal of the forms when all holes left by form ties have been filled and all irregular projections and any other minor surface defects have been mended. The surface shall be true and even, free from depression fins or projections.
The concrete shall be struck off with a straight edge and floated to true grade. Under no circumstance, the use of mortar topping for concrete surfaces shall be permitted.

**Grout cleaning**

Grout cleaning may be called for on the Drawings or required by the Engineer because of unsatisfactory appearance. The operation requires that the surface is wetted and uniformly covered with a grout consisting of 1 part cement to 1.5 parts fine sand. White cement shall be used for all or part of the cement in the grout to give the colour required to match the concrete. The grout shall be uniformly applied with brushes or a spray gun and all air bubbles and holes shall be completely filled. Immediately after the application of the grout, the surface shall be vigorously scoured with a cork or other suitable float. While the grout is still plastic, the surface shall be finished with a sponge rubber or other suitable float removing all excess grout. This finishing shall be done at the time when grout will not be pulled from the holes or depressions. After being allowed to be thoroughly dry, the surface shall be vigorously rubbed with a dry burlap to completely remove any dried grout. There shall be no visible film of grout remaining on the surface after this rubbing and the entire cleaning operation of any area must be completed on the day it is started. If any dark spot or streak remains after this operation, they shall be removed with a fine-grained silicon carbide stone, but the rubbing shall not be as much to change the texture of the surface. Unless it is required by the Drawings or directed by the Engineer, grout cleaning should be delayed until the final clean up of the Work.

**Rubbed finish**

On removal of forms, the rubbing of concrete shall be started as soon as its condition permits. Immediately before starting this work, the concrete shall be kept thoroughly saturated with water for a minimum period of 3 hours. Sufficient time shall elapse before wetting down to allow the mortar used in patching to have thoroughly set. A medium coarse carborundum stone shall be used for rubbing a small amount of mortar on the face. The mortar used shall be composed of cement and fine aggregate mixed in the same proportions as that used in the concrete being finished. Rubbing shall be continued until all form marks, projections and irregularities have been removed, all voids filled, and a uniform surface has been obtained. The paste produced by this rubbing shall be left in place at this time. The final finish shall be obtained by rubbing with a fine carborundum stone and water until the entire surface is of a smooth texture and uniform colour.

After the final rubbing has been completed and the surface has dried up, burlap shall be used to remove loose powder. The final surface shall be free from unsound patches, paste, powder and objectionable marks.

Any surface that has been given a rubbed finish shall be protected from subsequent construction operations. Any surface not protected, shall be cleaned and again rubbed, if necessary to secure a uniform and satisfactory surface at the own expenses of the Contractor.

On completion of initial rubbing, curing shall be continued.

**Tooled finish**

Tooled finishing shall be carried out by treating the surface with an approved heavy duty power hammer fitted with a multi-point tool, which shall be operated over the surface to remove 5mm to 6mm of concrete and expose maximum areas of coarse aggregate.
Aggregate left embedded shall not be fractured or loose. 25mm wide bands at all corners and arises shall be left as cast. The finished surfaces shall have even and of uniform appearance and shall be washed with water upon completion.

**Sandblasted finish**

Sandblasted finishing will be carried out on a thoroughly cured concrete surface with hard, sharp sand to produce an even fine-grained surface in which the mortar has been cut away, leaving the aggregate exposed.

**Wire brushed or scrubbed finish**

Wire brushed or scrubbed finish will be performed as soon as the forms are removed and while the concrete is yet comparatively green. The surface shall be thoroughly and evenly scrubbed with stiff wire or fiber brushes, using a solution of muriatic acid. The proportion of the solution shall constitute of one part acid to four parts water. This shall be continued until the cement film or surface is completely removed and the aggregate particles are exposed, leaving an even-pebbled texture presenting an appearance grading from that of fine granite to coarse conglomerate, depending upon the size and grading of aggregate used. When the scrubbing has progressed sufficiently to produce the texture desired, the entire surface shall be thoroughly washed with water to which a small amount of Ammonia has been added in order to remove all traces of acid.

**Inspection and making good**

Concrete surface shall be inspected for defects and for conformity with the Specifications and where appropriate, for comparison with approved sample finishes. Subject to the strength and durability of the concrete being unimpaired, the making good of surface defects may be permitted but the standard of acceptance shall be appropriate to the type and quality of the finish specified to ensure satisfactory performance and durability. On permanently exposed surfaces, great care is essential in selecting the materials and the mix proportions to ensure that the final colour of the faced area blends with the parent concrete in the finished structure.

Voids can be filled with fine mortar, preferably incorporating Styrene Butadiene Rubber (SBR) or Polyvinyl Acetate (PVA), while the concrete is still green or when it has hardened. Fine cracks can be filled by wiping a cement grout, a SBR, PVA or latex emulsion, a cement/SBR or a cement/PVA slurry across them. Fins and other projections shall be rubbed down.

**Protection**

High quality surface finishes are susceptible to damage during subsequent construction operations and temporary protection may have to be provided in vulnerable areas. The protective measures, among others, include the strapping of laths to arrises and the prevention of rust being carried from exposed starter bars to finished surfaces.

**10.1.17 Second stage concrete**

Unless shown on the Drawings or otherwise instructed by the Engineer, second stage concrete shall be of class for major RCC structures.

Block-outs for second stage concrete and the specifications and locations of the embedded parts shall be in accordance with the Drawings.
The surface against which the second stage concrete are to be placed shall be thoroughly cleaned to make the surface free from all loose particles, organic substances, oil, grease, rust, plastic materials, wood and defective concrete.

The projected parts of the embedded items or the parts that will remain embedded shall be thoroughly cleaned of oil, grease and rust. All such parts shall be true to dimensions, plumb and levels as shown on the Drawings and directed by the Engineer.

**10.1.18 Factory made pre-cast concrete elements**

The Engineer shall approve in writing any supplies of pre-cast concrete elements. The Engineer, if he so desires, may withdraw the approval later on.

All concrete works of such elements shall fully conform all requirements of this Specification.

The supplier shall maintain standard laboratory facilities.

Concrete members, specified to be fabricated as pre-cast concrete units, shall be fabricated with concrete of the specified class placed into a grout tight mould. If so required, the mould shall be laid on a vibrating table and vibration should be applied while concrete is placed.

Members, structurally dependent on a rigid fixing with the adjoining structures, should not in general be permitted to be pre-cast.

Unless otherwise approved by the Engineer, pre-cast concrete members shall neither be moved from the casting position until the concrete has attained a compressive strength of 80% of the specified 28-days strength, nor transported until it has gained a strength of 90% of the specified 28-days strength.

Extreme cares shall be taken in handling and moving pre-cast concrete members. Pre-cast girders and slabs shall be transported in an upright position. Shock shall be avoided and the points of support and directions of the reactions with respect to the member shall be approximately the same during transportation and storage as and when the member would be in its final position. If the Contractor finds it expedient to transport or store pre-cast units in other than this position, it shall be done at his own risks after notifying the Engineer of his intention to do so. Any units rejected shall be replaced at the Contractor’s own expenses by an acceptable unit.

All details on the handling and transportation of pre-cast members shall be submitted in writing to the Engineer for his approval.

Each pre-cast member is to be uniquely and permanently marked so as to show its type, date of casting and reinforcement.

**Handling and stacking of pre-cast units**

The Contractor shall give the Engineer full details of his proposed methods of handling, transportation and stacking of pre-cast concrete units. The Engineer will examine these in details and will either approve the methods or order modifications to ensure that the units are not subject to excessive stresses. The finally approved methods are to be adhered to at all times and the Contractor shall be deemed to have included in his rates for all measures required to handle, transport and stack the units safely and without undue stressing. However, such approval by the Engineer shall neither relieve the Contractor from his full responsibilities and liabilities of safe transportation and installation of any pre-cast units at the designated location as shown on the Drawings or as directed by the Engineer without any damage nor to make any deviation from the Specifications in fabricating the unit.
10.1.19 Control of heat in structures

The Contractor shall establish measures to control the heat deriving from the hydration of the concrete in structures of major dimensions. Temperature gradients introducing risks of cracking shall not occur and the temperature shall not exceed 70°C.

The Contractor shall also establish measures to avoid harmful excessive heat generation in massive structures, such as cooling down aggregates before mixing.

The Contractor shall submit in due time a proposal for the establishment of the aforementioned measures to the Engineer for his approval. The measures shall immediately be changed, if requested by the Engineer even later.

10.1.20 Back-fill to structures

All spaces, which have been excavated but are not occupied by the concrete structure shall be back-filled and compacted with materials acceptable to the Engineer or as shown on the Drawings and/or as per the directions of the Engineer.

10.1.21 Cleaning up

Upon completion of structure and before final acceptance, the Contractor shall remove all forms and scaffoldings, etc. down to 0.5m below the finished ground line. Excavated or garbage materials, rubbish etc. shall be removed from the Site, which shall be left in a neat condition satisfactory to the Engineer.

10.1.22 Measurement

The concrete of the several different grades and types completed in place in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer and accepted by the Engineer shall be measured by either the cubic meter for each class of concrete included in the BOQ or by the unit for each type of pre-cast concrete member listed in the BOQ. In computing quantities, the dimensions used shall be those shown on the Drawings or ordered by the Engineer; but the measurement shall not include any concrete used for the construction of temporary works or which is included in other billed items. No deduction from the measured quantity shall be made for drainage openings and pipes of less than 300mm in diameter, conduits, chamfers, reinforcement bars and expansion joint filler materials. However, deduction will be made for the volume of concrete displaced by piles embedded in the concrete.

The quantities of reinforcing steel and other related items as shown in the Contract Documents, which are included in the completed and accepted structure shall be separately measured for payment as per the provisions made under the Section on ‘Reinforcing Steel’ of this Specification.

Formwork and false work shall not be measured separately but shall be deemed to be an integral part of the concrete items.

Surface finishes shall not be measured separately but shall be deemed to be an integral part of the concrete items.

Joints including fillers and expansion joints shall not be measured separately unless they are specified as separate items in the BOQ.

The number of pre-cast concrete members of each type listed in the BOQ will be the number of acceptable members of each type furnished and installed in the work.
10.1.23 Payment

The cubic meters of concrete and the number of pre-cast concrete members, measured as provided above will be paid for at the Contract unit prices per cubic meter or the Contract unit prices per each member for each type or class as would be applicable as per the BOQ.

Payment for concrete of the various classes and for pre-cast concrete members of the various types shall be considered to be the full compensation for the costs for furnishing all materials including their transportation and storage, providing all equipment, labourers and incidentals and for doing all works involved in constructing the concrete work complete in place as shown on the Drawings and as specified. Such payment shall also include the full compensation for placing of rod in position, mixing the concrete mixture, concrete pouring, compacting by vibrator machine and curing, furnishing and placing expansion joint fillers, sealed joints, water-stops, drains, vents, miscellaneous metal devices and the drilling of holes for dowels and the grouting of dowels in drilled holes, unless payment for such works would be specified under another item of the BOQ.

Payment for all types of concrete work shall be considered to be the full compensation for the costs of furnishing and installing and removal of all temporary works like staging, formwork, working platforms, cranes, transporting, placing, compaction, finishing, curing and rendering of the concrete as specified till the concrete work becomes self-supporting and can perform its intended functions.

The Contractor’s rates shall be fully inclusive of all costs of all laboratory tests to be carried out as specified under different sub-items unless any payment is separately specified under the BOQ.

The payment shall be the full compensation of all incidentals necessary to complete the Work.

Payment for pre-cast units shall include all concrete, formwork, transport and erection and where applicable any bolts or other devices and bedding necessary to fix them in their permanent positions, all incidentals and all other works that will be necessary for full completion from transportation to safe erection of the members at the designated locations as shown on the Drawings or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Concrete Class as detailed on the Drawings and as specified in the Bill of Quantities.</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Pre-cast concrete elements as detailed on the Drawings and as specified in the Bill of Quantities</td>
<td>Number/Linear Meter /Cubic Meter</td>
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</table>
10.2 False work and forms

10.2.1 Scaffolding (false work)

Scaffolding is defined to be any temporary structure required to support structural elements of concrete, steel, masonry, or other materials at the time of their construction or erection.

Plans, Drawings and structural calculations in details shall be submitted to the Engineer for approval, but in no case shall the Contractor be relieved of his responsibilities for results obtained by using this Document.

All scaffolding shall be designed and constructed to provide the necessary rigidity and strength to safely support all loads imposed and produced in the finished structure, the lines and grades indicated on the Drawings. The supports shall be designed to withstand the worst combination of self-weight, formwork weight, formwork forces, reinforcement weight, wet concrete weight, construction and wind loads, together with all incidental dynamic effects caused by placing, vibrating and compacting the concrete. No harmful cracking should occur in the placed concrete. The Engineer may require the Contractor to employ screw jacks or hardwood wedges to take up any settlement in the formwork either before or during the placing of concrete.

All scaffolding, exceeding 20m or six storeys in height, shall be constructed of noncombustible or fire-retardant materials.

Scaffolding shall be founded on a solid base, which is safe against undermining, protected from softening and capable of supporting the loads imposed on it. Scaffolding which cannot be founded on a satisfactory footing shall be supported on piling, which shall be spaced, driven and removed in a manner approved by the Engineer.

Horizontal and inclined bracings shall be provided for posts higher than 3m. Spans of beam bottoms shall be supported by posts with maximum 1m apart when steel is used and instructions from the manufacturer/supplier shall be strictly followed. Spacing of the props under beams shall consider the increased load and shall be posted closer than those under the floor slab.

Scaffolding can, in certain cases, be supported on structures already constructed. In that case, the Contractor shall submit in due time to the Engineer in writing all information on the loading from the scaffolding as requested. The Engineer shall consider the loading and submit his approval in writing.

Scaffolding shall be set to give the finished structure the camber shown on the Drawings or specified by the Engineer. If any weakness develops or the scaffolding shows undue settlement or distortion during construction, the work shall be stopped and any structure affected thereby shall be removed and the scaffolding shall be further strengthened before work is resumed. Suitable screw jacks, pairs of wages or other devices shall be used at each post to adjust scaffolding to grade.

All materials used in the construction of the scaffolding shall conform to the corresponding ASTM or BS Standards or any other equivalent International Standards. Material tests and certificates may be required by the Engineer. Examinations of welding may also be requested. Test loading of the scaffolding may be requested for the determination of the flexibility and the strength. All expenses of the tests and examinations of scaffoldings shall be borne by the Contractor on non-reimbursable basis.

Scaffolds shall be made from strong bamboo poles, wooden posts, steel pipes or any other suitable materials. They shall be adequately tied to vertical members resting on firm floor. Strong ropes shall be
used to tie up bamboo poles. In addition, cross-bracing with bamboo or wooden posts shall be provided along with ties or guys of steel wire or rod not less than 6mm in diameter.

Good, sound and uniform bamboo shall be collected in sufficient quantities for providing scaffolding, propping, temporary staging, ramp etc. The bamboos shall be free from any defects, firmly ties to each other and joints made smooth. Joining members only with nails shall be prohibited. Bamboos for vertical support shall not be less than 75mm in diameter and shall be straight as far as possible. Bamboos may be used as vertical support for up to a height of 4m, if horizontal bracings are provided at the centre. Splicing shall be prohibited.

After stripping the formwork, the bamboo posts shall be cleaned and stacked vertically in shade protected from rain and sun. Defective or damaged bamboo posts shall be removed from the Site.

Timber posts shall be used in supporting formwork upto a height of 6m. The posts shall not be less than 80mm in diameter at any place and shall spread to at least 150mm in diameter at the top. The timber posts shall be supported on timber planks at the bottom. Either the bottom or the top of the posts shall be wedged with a piece of triangular wood peg for easy removal. Adequate horizontal and inclined braces shall be used for all timber centering. All timber posts shall be carefully inspected before use and members with cracks and excessive knots and crookedness shall be discarded. The joints shall normally be made with bolts and nuts. No rusted or spoilt threaded bolts and nuts shall be used.

When steel scaffoldings are used, it shall be painted in a manner that no mark of corrosion shall appear on the permanent concrete structures.

The Engineer shall only select the type of scaffolding. Bamboo scaffolding will only be used, if agreed and allowed by the Engineer. All scaffoldings shall remain in place for a period, which shall be determined by the Engineer.

Scaffold shall be dismantled after use piece by piece. Holes in the wall shall be filled up with the same materials as that of the wall. Filled up holes shall have uniformity in texture and colour with the surrounding surface. Crash striking shall not be allowed.

Triangular wooden wedges shall be put under the posts for easy dismantling of the members. Timber planks or steel sheets shall be placed at a time below the vertical or inclined posts covering several posts.

Materials and joints in scaffolding shall be inspected from time to time both before and after erection for the soundness, strength, damage due to weathering etc. Inspections shall be made for spillage of material or liquids, loose material lying on the gangways and proper access to the platform.

The scaffold shall be secured to the building at enough places; no ties shall be removed. Warning sign, prohibiting the use of any defective or incomplete scaffold and working in bad weather and high wind, shall be posted in a prominent place. Inspections shall be made for the observance of these requirements.

10.2.2 Formwork

Definition

Formwork is defined to be an enclosure or panel, which contain the fluid concrete and withstand the forces due to its placement and consolidation. Forms in turn be supported on scaffolding.
**General**

The work to be performed under this Sub-section includes the furnishing and installing and removing of forms for all cast-in-places concrete work as shown and noted on the Drawings and as specified herein or as directed by the Engineer.

Forms shall be substantial and sufficiently tight to prevent leakage of mortar. Forms shall be of sufficient rigidity to prevent objectionable distortion of the formed concrete surface due to pressure of the concrete and other loads incidental to construction operations. They shall be properly braced or tied together to maintain position and shape. Forms and their supports shall be so designed as not to damage previously placed structure.

Relevant provisions of the American Concrete Institute (ACI) issue of ACI 347 on ‘Recommended Practice for Concrete Formwork’ or some other generally accepted Standards shall apply for the structural designing of the formwork, except as they may be modified herein.

**Materials**

Formwork shall be constructed from sound materials of sufficient strength, properly braced, strutted and shored as to ensure rigidity throughout the placing and compaction of the concrete without visible deflection. The materials used be of wood, steel or other approved materials and shall be mortar-tight. Formwork shall be so constructed that it can be removed without shock or vibration to the concrete.

Formwork for concrete, permanently exposed to public inspection, shall be faced with plain 28/26 gauge steel sheet fitted over 38mm thick wooden plank panels suitably braced or steel framing faced with minimum 12/14 BWG mild steel sheet. Formwork for cement concrete blocks shall be fabricated from M.S. sheet of sufficient thickness to prevent any distortion.

Where metal forms are used, all bolts and rivets shall be countersunk and well grounded to provide a smooth plane surface.

Where timber is used, it shall be well seasoned, free from loose knots, projecting nails, splits or other defects that may mark the surface of concrete.

Form ties shall be prefabricated rod, flat band, or wire type, or threaded internal disconnected type, of sufficient tensile capacity to resist all imposed load of freshly placed concrete and having external holding devices of adequate bearing area. Ties shall permit tightening and spreading of forms and shall leave no metal closer than 25mm from surface. Ties shall fit tight to prevent mortar leakage at holes in forms. Removable ties shall be coated with non-staining bond breaker. All ties shall be protected from rusting at all times. No wire ties or wood spreaders shall be permitted. Cutting ties back from concrete face will not be permitted. Ties for exposed Architectural Concrete shall be plastic cone snap ties.

**Construction method**

The Contractor shall submit for the approval of the Engineer details of the methods and materials proposed for formwork to each section of the Work. Details of all proposed wrought formwork and formwork to produce special finishes are to be submitted for approval in writing to the Engineer before any material is hauled at Site. If the Engineer so requires, samples of formwork shall be constructed and concrete be placed so that the proposed methods and finish effect can be demonstrated.

All joints shall be close fitting to prevent leakage of grout. At construction joints the formwork shall be tightly secured against previously cast or hardened concrete in order to prevent stepping or ridges to exposed surfaces.
Where the Contractor proposes to make the formwork from standard sized manufactured formwork panels, the dimensions of such panels shall be approved by the Engineer before they are used for construction of the Work. The finished appearance of the entire elevation of the structure and the adjoining structures shall be considered when planning the patterns of joint lines caused by the formwork and by construction joints to ensure continuity of horizontal and vertical lines.

Formwork shall be constructed to provide the correct shape, lines and dimensions of the concrete shown on the Drawings. Due allowance shall be made for any deflection, which will occur during the placing of concrete within the formwork. Panels shall have true edges to permit accurate alignment and provide a neat line with adjacent panels and at all construction joints. All panels shall be fixed with their joints either vertical or horizontal, unless otherwise specified or approved.

Formwork shall be provided for the top surfaces of sloping work where the slope exceeds 15° with the horizontal and shall be anchored to enable the concrete to be properly compacted and prevent floating. Cares shall be taken to prevent air being entrapped. Openings for inspection of the inside of the formwork and for the removal of water used for washing shall be provided and so formed as to be easily closed before placing concrete.

**Formwork for exposed concrete surfaces**

All exposed concrete surfaces are to be ‘form finish’ and shall be cast in any approved formwork and shall be free from honeycomb, fins, projections and air holes. All external angles to form finish concrete surfaces shall be chamfered as directed.

Forms for concrete surfaces exposed to view shall produce a smooth surface of uniform texture and colour substantially equal to that which would be obtained with the use of plywood conforming to the National Institute of Standards and Technology Product Standard PSI for Exterior B-B Class I Plywood. Panels lining such forms shall be arranged so that the joint lines form a symmetrical pattern conforming to the general lines of the structure. The same type of form lining material shall be used throughout each element of a structure. Such forms shall be sufficiently rigid so that the undulation of the concrete surface shall not exceed 3mm when checked with a 1.5m long straight edge or template.

The Contractor shall submit shuttering Drawings and details of pattern and the method of forming joints in the exposed (form finish) concrete to the Engineer for his approval. All changes and modification made by the later shall be appropriately incorporated by the former and final approval whereof be obtained from the Engineer.

Unless otherwise stated on the Drawings, wrought formwork shall be used for all permanently visible concrete surfaces. Wrought formwork shall be such as to produce a smooth and even surface free from perceptible irregularities. Tongues and grooved paneled boards, plywood or steel forms shall have their joints flushed with the surface. The formwork shall be formed with approved standard size panels. The panels shall be arranged in a uniform approved pattern, free from defects likely to be detected in the resulting concrete surface.

In all types of formwork to form finished exposed concrete, only non-steining mould oil shall be used as approved by the Engineer.

The respective usage of the same formwork to cast form-finished exposed concrete shall be as decided by the Engineer and in no case the formwork, not guaranteed to produce the required form-finish to the satisfaction of the Engineer, shall be used.
The exposed concrete shall have a uniform finish. The finish of the concrete when shuttering and formwork are removed will generally be without any blemish and will be such as will not require touch up. Slight touch up for a small spot or two, if necessary shall be carried out skillfully so as to be synonymous with the entire surfaces.

The finished surfaces shall be within the specified tolerances and full cover to the reinforcement steel shall be maintained.

**Formwork for non-exposed concrete surfaces**

Unless otherwise stated on the Drawings, rough formwork may be used for all surfaces, which are not permanently exposed. Rough formwork may be constructed of plain butt-joined sawn timber. But the Contractor shall ensure that all joints between boards shall be grout-tight.

The finished surfaces shall be within the specified tolerances and full cover to the reinforcement steel shall be maintained.

**10.2.3 Formed surfaces and finish**

The formwork shall be lined with a material approved by the Engineer so as to provide a smooth finish of uniform texture and appearance. This material shall leave no stain on the concrete and so joined and fixed to its backing as not to impart any blemish. It shall be of the same type and obtained from only one source throughout the construction of any individual structure. The Contractor shall make good any imperfection in the finish as required by the Engineer. Internal ties and embedded metal parts will be allowed only with the specific approval of the Engineer.

**10.2.4 Sizes of timber and other sections for formwork**

Scaffolds, formwork and components thereof shall be capable of supporting without failure, at least two times the maximum intended load. The following types of loading shall be considered in designing the formwork:

- **a)** Weight of wet concrete : 20 kN/m².
- **b)** Live load due to workmen and impact of ramming or vibrating : 15-40 kPa (light duty for carpenter and stone setters, medium duty for brick layers and plasterers, heavy duty for stone masons).
- **c)** Allowable bending stress (flexural tensile stress) in soft timbers : 8,000 kPa.

The sizes for formwork elements specified in the Table given below are applicable for spans of upto 5m and height of upto 4m. In case of longer span and height, formwork and support sizes shall be determined by calculating the load and approved by the Engineer before use.

<table>
<thead>
<tr>
<th>Types of Formwork</th>
<th>Members Size in mm</th>
</tr>
</thead>
<tbody>
<tr>
<td>Flat sheetings for slab bottoms, columns and beam sides</td>
<td>25 to 50</td>
</tr>
<tr>
<td>Beam bottoms</td>
<td>75x100 to 150x150</td>
</tr>
<tr>
<td>Vertical posts</td>
<td>75x100 to 150x150</td>
</tr>
<tr>
<td>Bamboo posts</td>
<td>Minimum 75 dia</td>
</tr>
<tr>
<td>Timber posts</td>
<td>Not less than 100 dia at mid-length and 80 dia at thin end</td>
</tr>
<tr>
<td>Joist and ledgers supporting sheetings of slab</td>
<td>50x100 to 75x200</td>
</tr>
<tr>
<td>Studs for supporting vertical wall sheetings</td>
<td>50x100 to 150x150</td>
</tr>
<tr>
<td>Columns yokes-horizontal cross, pieces supporting vertical sheetings</td>
<td>50x100 to 100x100</td>
</tr>
</tbody>
</table>
10.2.5 Quality of shuttering

**General**

The shuttering shall have smooth and even surface and its joints shall not permit leakage of cement grout.

Ply-board shuttering material shall be well seasoned free from projecting nails, splits or other defects that may mark the surface of concrete. It shall not be so dry as to absorb water from concrete and swell and bulge, nor so green or wet as to shrink after erection.

The timber shall be accurately sawn and plain on the sides and the surface coming in contact with concrete.

Wooden formwork with metal sheet lining or steel plates stiffened by steel angles shall also be permitted. Where metal forms are used, all bolts and nuts shall be countersunk and well grounded to provide a smooth plain surface.

The chamfers, leveled edges and mouldings shall be made in the formwork itself. Opening for fixture and other fittings connected with the services shall be provided in the shuttering as directed by the Engineer.

Clamps shall be used, to its practicality, to hold the forms together. Where use of nails is unavoidable, it shall be kept to minimum number and these shall be left projected so that they can easily be withdrawn. Use of double-headed nails shall be preferred.

**Tolerances**

The formwork shall be made so as to produce a finished concrete true to shape, lines, levels, plumb and dimensions as shown on the Drawings subject to the following tolerances unless otherwise specified in this document or Drawings or as directed by the Engineer.

- **Sectional dimension** ± 5mm
- **Plumb** ± 1 in 1000 of height
- **Levels** ± 3mm before any deflection has been taken place

Tolerances given above are specified for local aberrations in the finished concrete surface and should not be taken as tolerance for the entire structure taken as a whole or for the setting and alignment of formwork, which should be as accurate as possible to the entire satisfaction of the Engineer. Errors, if noticed in any lift/tilt of the structure after stripping of forms, shall be corrected in the subsequent work to bring back the surface of the structure to its true alignment.

10.2.6 Preparation of formwork

**General**

The formwork shall be arranged in a manner as to readily be dismantled and removed from the cast concrete without shock, disturbance or damage. Where necessary, the formwork shall be so arranged that the soffit form, properly supported on props only, can be retained in position for such period as may be required by maturing conditions or Specification.

The surfaces of formwork shall be free from foreign matters, projecting nails and the like, splits or other defects, and all formwork shall be cleaned and made free from standing water, dirt, shavings, chippings or other foreign matter before concrete is placed.
Before placing concrete, all built-in reinforcement bars, anchoring, steel beams, cables, fixing truss, bolts, pipes or conduits or any other fixtures shall be fixed in their correct positions. The cores and other devices for forming holes shall be held fast by fixing to the formwork or otherwise. Holes shall not be cut in any concrete without the approval of the Engineer.

All exterior and interior angles on the finished concrete of 90° or less shall be given 12mm – 20mm chamfers unless otherwise shown on the Drawings or directed by the Engineer. When chamfers are to be formed, the fillets shall be accurately cut to size to provide a smooth and continuous chamfer.

No ties or bolts or other devices shall be built into the concrete for the purpose of supporting formwork without the prior approval of the Engineer. The whole or part of any such support embedded in the Reinforced Concrete shall be capable of removal so that no part, remaining embedded in the concrete, shall be nearer than 75mm from the surface. Holes left after removal of such supports shall be neatly filled with well-reamed dry-pack mortar following the procedures described in the relevant Sub-section of this Specification.

All rubbish shall be removed from the interior of the forms before the concrete is placed. After cleaning and prior to placement of reinforcing steel, the formwork in contact with the concrete shall be treated with a suitable non-staining mould oil or suitable approved release agent to prevent sticking of the concrete. Such works shall not discolor or otherwise injure the surface of the concrete. Care shall be taken to prevent the oil from coming in contact with the reinforcement or mixing with the concrete. At construction joints, surface-retarding agents shall be used only where ordered by the Engineer.

All formwork shall be inspected and approved by the Engineer before concrete is placed in it. However, this shall not relieve the Contractor from the requirements as to soundness, finish and tolerances of the concrete specified in this Specification or elsewhere acknowledged as Standard. If, at any period of the work during or after placing the concrete, the forms show signs of sagging or bulging, the concrete shall be removed to the extent directed by the Engineer, the forms brought to the proper position and new concrete placed. No allowance shall be made to the Contractor for such extra works.

**Removal of forms**

Forms shall not be removed without the approval of the Engineer. In the determination of the time for the removal of forms, consideration shall be given to the location and character of the structure, the weather, the materials used in the mix and other conditions influencing the early strength of the concrete. Extreme cares shall be taken to ensure that the method of removal shall not cause overstressing of the concrete or damage to its surface.

Forms shall be removed in such a manner as to permit the structure to uniformly and gradually take the stresses due to its own weight as not to impair safety and serviceability of the structure. All concrete to be exposed by form removal shall have sufficient strength not to be damaged thereby.

Forms shall not be removed in the cases of footing forms where the removal would endanger the safety of the cofferdams, forms from enclosed cells where access is not provided, deck forms in the cells that do not interfere with the future installation of utilities shown on the Drawings, or other works.

Except for concrete being post-tensioned, no concrete shall be subjected to loading which will induce a compressive stress in it exceeding one-third of its compressive strength at the time of loading, or one-third of the specified characteristic strength whichever is less. It may be possible to use shorter periods before striking forms by determining the strength of the concrete in the structural element.
Forms supporting cast-in-situ concrete in flexure may be struck when the strength of the concrete in the element is 10 N/mm² or twice the stress to which it will be subjected, whichever is greater provided that striking at this time will not result in an unacceptable deflection. This strength may be assessed by test on cylinder/cube cured under the same conditions as the concrete in the element as far as possible.

Forms on upper sloping faces of concrete shall be removed as soon as the concrete has attained sufficient stiffness to prevent sagging. Any repair or treatment required on such sloping surfaces shall be performed at once.

If the floor is to be used to support construction loads, props should be retained for 28 days unless the Contractor can prove the requisite concrete strength by tests.

The form shall be removed slowly, as the sudden removal of wedges is equivalent to a shock load on the partly hardened concrete.

Materials and plants shall not be stacked on any newly constructed floor unless sufficient support is maintained to withstand such loads without damaging the floor.

The following table is a guide to the minimum periods that must elapse between the completion of the concreting operations and the removal of formwork. No formwork shall be removed without the permission of the Engineer and such permission shall not relieve the Contractor of his responsibilities regarding the safety of the structure.

<table>
<thead>
<tr>
<th>Type and position of formwork</th>
<th>Approximate period (days)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Side of beams, walls and columns (unloaded)</td>
<td>5</td>
</tr>
<tr>
<td>Slab soffits (props supporting)</td>
<td>14</td>
</tr>
<tr>
<td>Removal of props to slabs</td>
<td>21</td>
</tr>
<tr>
<td>Beam soffits (props supporting)</td>
<td>21</td>
</tr>
<tr>
<td>Removal of props to beams</td>
<td>28</td>
</tr>
</tbody>
</table>

Notwithstanding the foregoing, the Contractor shall be held responsible for any damages arising from removal of formwork before the structure is capable of carrying its own weight and any incidental loading.

10.2.7 Openings

Temporary and permanent openings in concrete shall be framed neatly with provisions for keys or reinforcing steel as shown on the Drawings or as directed by the Engineer.

10.2.8 Defects in formed surfaces

Workmanship in formwork along with concrete placing shall be such that concrete shall normally require no repair to surfaces being perfectly compacted and smooth.

If any blemish is revealed after removal of formwork, the Contractor shall obtain immediately the Engineer’s decision concerning remedial measures to be undertaken. Notwithstanding the specifications and provisions stated under the Sub-section on ‘Finish and Finishing’ of this Specification, such measures may include but shall not be limited to the following:

- Fins, pinholes, bubbles, surface discolouring and mirror defects may be rubbed down with sacks immediately on removal of the form.
Abrupt and gradual irregularities may be rubbed down with carborundum stone and water after concrete has been fully cured.

Deep honeycombed concrete shall be repaired within 24 hours of striking the formwork by cutting back to sound concrete. The concrete shall be cut back at least 50mm behind face reinforcement. Cut edges shall be regular and not feathered. Recasting shall be with the same concrete as the original casting. The Engineer shall approve the formwork and its method of placing in this case also.

Under some circumstances, abrupt and gradual irregularities of shallow honeycombed concrete may be repaired by cutting back and reforming with an approved epoxy resin or mortar in accordance with the manufacturer’s instructions.

Regardless of the above repairing measures, any structure containing excessive honeycomb, as would be termed by the Engineer, shall be subject to rejection by the Engineer. The Contractor, on receipt of written orders from the Engineer, shall remove and rebuild such portions of the structure at his own expenses.

Holes to be filled

Holes on the concrete surfaces formed by formwork supports or the like shall be filled with dry pack mortar made from one part by weight of ordinary Portland cement and three parts of specified fine aggregate approved by the Engineer. The mortar shall be mixed with sufficient water only to make the materials stick together when being moulded in the hands. All construction materials shall conform to the requirements as described previously and under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification.

The Contractor shall thoroughly clean any hole that is to be filled and break out any loose, broken or cracked concrete or aggregate and remove any dry cement from the hole. The surrounding concrete shall be soaked until the whole surface that will come into contact with the dry pack mortar has been covered and darkened by absorption of the free water by the cement. The surface shall then be dried so as to leave a small amount of free water on it.

The dry pack material shall then be placed and packed in layers having a compacted thickness of not more than 10mm. Compaction shall be carried out by using a hardwood stick and a hammer and shall extend over the full area of the layer. Special cares should be taken to compact the dry pack against the sides of the holes.

After compaction, the surface of each layer shall be scratched before further loose material is added. The holes shall be slightly overfilled. The surface shall be finished by laying a hardwood block against the dry pack fill and striking the block several times.

Approval of scaffoldings and form

Plans, Drawings and structural calculations shall be submitted to the Engineer on time so that no construction of such scaffoldings and forms shall take place before the Engineer’s approval is accorded in writing. Such approval shall not relieve the Contractor of his responsibilities for the involved structure.

The Engineer shall have reasonable time for his examination of the Contractor’s plans and calculations, if scaffoldings are introducing temporary loading on new structures in particular. For this purpose, the Contractor shall not be allowed any extension of time beyond the stipulated period of the Contract.
Before concrete is placed, the Engineer shall inspect all formworks and scaffoldings. No concrete shall be placed until inspection is made and approval is given by the Engineer. Such approval shall not relieve the Contractor of any of his responsibilities under the Contract for the successful completion and the soundness of the structure.

10.2.11 Measurement

Formwork and false work shall not be measured separately but shall be deemed to be an integral part of the concrete items.

10.2.12 Payment

The Contractor’s rates for concrete work, inter-alia, shall be inclusive of all costs of all formwork, falsework and centering and for their subsequent removal. No additional payment will be made to the Contractor for these works.
10.3 Water proofing polythene sheet

10.3.1 Description
Works covered under this item shall consist of supplying and laying in place one layer of polythene sheet of weight in accordance with the applicable Drawings, BOQ and these specifications and/or as directed by the Engineer.

10.3.2 Construction requirement
Sheets shall be laid covering the entire inside area under the Cement Concrete. Before laying the sheets, the surface shall be cleaned to give a surface free from damage, tear or other imperfections and shall be laid such that there is a minimum of 300mm overlap of the adjacent strips.

10.3.3 Measurement
Measurement shall be taken for payment in square meter of the actual area covered by the sheets and accepted by the Engineer. No allowance shall be made for overlaps.

10.3.4 Payment
The amount of completed and accepted work measured as provided above shall be made at the Contract unit price per square meter and the payment shall constitute full compensation for furnishing all materials, equipment including their storage, handling and transport and all labours, cleaning, preparing, cutting, laying, fixing and all incidentals necessary to complete the work. No additional payment shall be made for the overlaps.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Supplying and laying of polythene sheet</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
SECTION-11
JOINTS IN CONCRETE
<table>
<thead>
<tr>
<th>SECTION</th>
<th>CONTENTS</th>
<th>PAGE</th>
</tr>
</thead>
<tbody>
<tr>
<td>11.1</td>
<td>Construction joints</td>
<td>118-119</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>118</td>
</tr>
<tr>
<td></td>
<td>Bonding</td>
<td>118–119</td>
</tr>
<tr>
<td></td>
<td>Bonding and doweling to existing structures</td>
<td>119</td>
</tr>
<tr>
<td></td>
<td>Forms at construction joints</td>
<td>119</td>
</tr>
<tr>
<td>11.2</td>
<td>Expansion and contraction joints</td>
<td>119-123</td>
</tr>
<tr>
<td>11.2.1</td>
<td>Expansion joints</td>
<td>119-120</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>119</td>
</tr>
<tr>
<td></td>
<td>Material</td>
<td>119</td>
</tr>
<tr>
<td></td>
<td>Metal armour</td>
<td>120</td>
</tr>
<tr>
<td></td>
<td>Armour assemblies</td>
<td>120</td>
</tr>
<tr>
<td>11.2.2</td>
<td>Contraction joints</td>
<td>120</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>120</td>
</tr>
<tr>
<td></td>
<td>Material</td>
<td>120</td>
</tr>
<tr>
<td>11.2.3</td>
<td>Pourable joint sealants</td>
<td>120</td>
</tr>
<tr>
<td>11.2.4</td>
<td>Compressive filler</td>
<td>121</td>
</tr>
<tr>
<td>11.2.5</td>
<td>Water stops</td>
<td>121</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>121</td>
</tr>
<tr>
<td></td>
<td>Types</td>
<td>121</td>
</tr>
<tr>
<td></td>
<td>Polyvinyl chloride (PVC) water stops</td>
<td>121</td>
</tr>
<tr>
<td></td>
<td>Rubber water stops</td>
<td>121</td>
</tr>
<tr>
<td>11.2.6</td>
<td>Installation</td>
<td>122-123</td>
</tr>
<tr>
<td></td>
<td>Open joints</td>
<td>122</td>
</tr>
<tr>
<td></td>
<td>Filled joints</td>
<td>122</td>
</tr>
<tr>
<td></td>
<td>Sealed joints</td>
<td>122</td>
</tr>
<tr>
<td></td>
<td>Water stops</td>
<td>122-123</td>
</tr>
<tr>
<td>11.3</td>
<td>Measurement</td>
<td>123</td>
</tr>
<tr>
<td>11.4</td>
<td>Payment</td>
<td>123</td>
</tr>
</tbody>
</table>
SECTION-11
JOINTS IN CONCRETE

11.1 Construction joints

General

Construction joints are defined as concrete surfaces upon or against which concrete is to be placed and to which new concrete is to be placed, that have become so rigid that the new concrete cannot be incorporated integrally with that previously placed. Construction joints shall be formed wherever there is a discontinuity in placing concrete in external elements of concrete structures. Formed vertical or inclined construction joints as well as unformed joints, which are due to interruption of concrete placement, shall be made only where located on the Drawings or shown in the pouring schedule or as directed by the Engineer. All exposed faces of construction joints shall be made absolutely straight, leveled or plumbed and normal to the finished surface.

Spacing of construction joints shall be in accordance with good concreting practice as defined in BS 8110 or equivalent and enabling adequate precautions to be taken against shrinkage cracking. Placing of concrete shall be carried out continuously. The joints shall be at right angle to the general direction of the member and shall take due account of shear and other stresses.

All planned reinforcing steel shall extend uninterrupted through joints. Additional reinforcing steel dowels shall be placed across the joints, if and when directed by the Engineer. Such additional steel shall be furnished and placed at the Contractor’s expenses.

Bonding

Unless otherwise shown on the Drawing, horizontal joints may be made without keys and vertical joints shall be constructed with shear keys. Surfaces of fresh concrete at horizontal construction joints shall be rough floated sufficiently to thoroughly consolidate the surface and intentionally left in a rough condition. Shear keys shall consist of formed depressions in the surface covering approximately one-third of the contact surface. The forms for keys shall be beveled so that removal will not damage the concrete.

Surfaces of construction joints shall be prepared as early as possible after casting. The preparation shall consist of the removal of all laitance, loose or defective concrete coatings, sand and other deleterious materials. Preparation shall be carried out preferably when the concrete has set but not hardened by jetting with a fine spray of water or brushing with a stiff brush, just sufficient to remove the outer mortar skin and to expose the larger aggregate without it is being disturbed. Where this treatment is impracticable and work is resumed on a surface, which has set, the whole surface shall be thoroughly roughened or scrapped with suitable tools so that no smooth skin of concrete that may be left from the previous work is visible.

The prepared joint face shall be thoroughly cleaned by compressed air and water jets or other approved means and brushed and watered immediately before depositing concrete. The cleaned and saturated surfaces that also include vertical and inclined surfaces, shall first be thoroughly covered with a thin coating of mortar or neat cement grout against which the new concrete shall be placed before the grout has attained its initial set.

The placing of concrete shall be carried continuously from joint to joint. The face edges of all joints, which are exposed to view, shall be carefully finished true to line and elevation.
Construction joints in floors shall be located within the middle third of spans of slabs, beams and girders. Joints in girders shall be offset a minimum distance of two times the width of intersecting beams.

**Bonding and doweling to existing structures**

When reinforcing dowels grouted into the holes drilled in the existing concrete is required at such construction joints, the holes shall be drilled by methods that will not damage the concrete around the holes. The diameters of the holes shall be 6mm larger than the nominal diameter of the dowels unless shown otherwise on the Drawings. The dowel bars shall be round mild steel bar of the diameter and length as indicated on the Drawings and/or as per the directions of the Engineer. The grout shall be a neat cement paste of Portland cement and water or an epoxy. Immediately prior to placing the dowel bars, the holes shall be cleaned off dust and other deleterious materials, shall be thoroughly saturated with water, have all free water removed and shall be dried to a saturated surface dry condition. Sufficient grout or an epoxy shall be placed inside the holes so as not to remain any void after the dowels are inserted. Grout shall be cured for a period of at least 3 (three) days or until dowel bars are encased in concrete. When an epoxy is used, the mixing and placing shall conform to the manufacturer’s recommendations.

**Forms at construction joints**

When forms at construction joints overlap previously placed concrete, they shall be re-tightened before depositing new concrete. Exposed face edges of all joints shall be neatly formed with straight bulkheads or grade strips, or otherwise properly finished true to line and elevation.

11.2 Expansion and contraction joints

11.2.1 Expansion joints

**General**

Expansion joints are intended to accommodate relative movement between adjoining parts of a structure. Compressible filler shall be placed between the joint faces to provide freedom for expansion for the two adjacent concrete masses. Care shall be taken to ensure that the material fills the joint completely and that no concrete or hard material is left in the joint after the second face of the joint has been cast.

**Material**

One of the following specifications shall be used as pre-mould fillers:

- Specification for Preformed Expansion Joint Fillers for Concrete Paving and Structural Construction, ASTM 1751.

- Specification for Preformed Sponge Rubber and Cork Expansion Joint Fillers for Concrete Paving and Structural Construction ASTM D 1752. Type-II (cork) shall not be used when resiliency is required.

- Specification for Preformed Expansion Joint Filler for Concrete, ASTM D 994.

The bitumen sheet, laid on the horizontal top surface of the expansion joint keys, shall be a 10mm thick material approved by the Engineer.
**Metal armour**

Expansion joint armour assemblies shall be fabricated from steel with the following materials:

- Steel bars, plates and shapes shall conform to the requirements of ASTM A 36.
- Bolts and nuts shall conform to the requirements of ASTM A 307.
- High strength bolts, nuts and washers shall conform to the requirements of ASTM A 325.
- Steel castings shall conform to the requirements of ASTM A 486 or ASTM A 27.
- Grey iron castings shall conform to the requirements of ASTM A 48.
- Sheet metal shall be of commercial quality.

**Armour assemblies**

All assemblies shall be accurately fabricated and straightened at the workshop, as necessary to conform to the concrete sections. The assemblies shall be installed so that their top surface matches the plane of the adjacent finished concrete surface throughout the length of the assembly. Appropriate methods shall be followed in placing the assemblies to keep them in correct position during the placing of concrete. The opening at expansion joints shall be that designated on the Drawings at normal temperature or as directed by the Engineer for other temperatures. Caress shall be taken to avoid impairment of the clearance in any manner.

11.2.2 **Contraction joints**

**General**

Joints placed in structures or slabs to provide for volumetric shrinkage of monolithic unit or movement between monolithic units are defined as contraction joints. Contraction joints shall be constructed so that there will be no bond between the concrete surface forming the joints.

**Material**

Material placed in contraction joints shall consist of asphalt saturated felt paper or other approved bond-breaking materials.

11.2.3 **Pourable joint sealants**

Pourable sealants shall be placed along the top edges of contraction or filled expansion joints. It shall conform to the following considerations:

- Unless otherwise shown on the Drawings and/or ordered by the Engineer, joint sealants shall be a hot poured rubber bitumen compound for horizontal joints and either a bituminous compound or an elastomeric two parts polysulphide sealant for sloping, vertical and soffit joints.
- Bituminous compounds shall comply with BS 2499 for horizontal joints and BS 2499 Type A1 for sloping or vertical joints. Polysulphide compound shall comply with BS 4254.
- Joint sealants and the requisite priming materials shall be obtained from manufacturers approved by the Engineer. The application of joint sealant shall not be commenced without the Contractor obtains its approval by the Engineer.
11.2.4 Compressive filler

Unless otherwise specified, the joint filler shall be of resin or bituminous bonded corks such as ‘Hydrocor’ manufactured by Expandite Ltd. The filler shall be obtained from a manufacturer approved by the Engineer and shall be stored and fixed in accordance with the manufacturer’s instructions.

11.2.5 Water stops

General

Water stops shall be of the type, size and shape shown on the Drawings and/or as directed by the Engineer. They shall be dense, homogeneous and without holes or other defects.

Types

Water stops to be used may be of the following types:

- **Polyvinyl chloride (PVC) water stops**

  Where shown on the Drawings, contraction and expansion joints shall be made watertight by the provision of a continuous Water Stop strip of Poly Vinyl Chloride (PVC) manufactured by the extrusion process from an elastomeric plastic compound, the basic resin of which shall be Poly Vinyl Chloride. Unless otherwise specified or ordered, a two bulb dumbbell section PVC Water Stop shall be used in construction joints and a three bulb section PVC Water Stop shall be used in expansion joints.

  Water Stops shall be of high grade PVC, containing no filler or reclaimed or scrap material. PVC shall comply with the requirements of BS 2571 for PVC Type A, Class 1. The quality of Water Stops shall comply with the following major requirements:

  - Specific gravity 1.30 (maximum)
  - Hardness 80 (minimum) duro
  - Tensile strength 138 kg/cm² (minimum)
  - Elongation 225% (minimum)

- **Rubber water stops**

  Rubber Water Stops shall be manufactured with synthetic rubber made exclusively from neoprene, reinforcing carbon black, zinc oxide, polymerization agents and softeners. The quality shall conform the following major requirements:

  - Neoprene content 70% by volume (minimum)
  - Hardness 50-60 duro
  - Tensile strength 193 kg/cm² (minimum)
  - Elongation 600% (minimum)

  Rubber Water Stops shall be formed with an integral cross section in suitable moulds so as to produce a uniform section with a permissible variation in dimension of 0.8mm plus or minus. No splices will be permitted in straight strips. Strips and special connection pieces shall be well cured in a manner such that any cross section shall be dense, homogeneous, and free from all porosity. Junctions in the special connection pieces shall be full moulded. During the vulcanizing period, the joints shall be securely held by suitable clamps. The material at the splices shall be dense and homogeneous throughout the cross-section.
11.2.6 Installation

Open joints

Open joints shall be constructed by the insertion and subsequent removal of a wood strip, metal plate, or other approved material. The insertion and removal of the template shall be accomplished without chipping or breaking the corners of the concrete. When not protected by metal armour, open joints in slabs shall be finished with an edging tool. Upon completion of concrete finishing work, all mortars and other debris shall be removed from the open joints.

Filled joints

When filled joints are shown on the Drawings or asked by the Engineer, pre-mould type fillers shall be used unless Poly Styrene board is specifically called for. Filler for each joint shall consist of as few pieces of material as possible. Abutting edges of filler material shall be accurately held in alignment with each other and tightly fit or taped as necessary to prevent the intrusion of grout. Joint filler material shall be anchored to one side of the joint by waterproof adhesive or other methods so as to prevent it from working out of the joint but not interfere with the compression of the material.

Sealed joints

Prior to installation of the pourable joint sealants, all foreign materials shall be removed from the joint. The filler material shall be cut back to the depth shown or approved and the surface of the concrete, in contact with the sealant, be cleaned by light sand blasting. When required, a Poly Ethylene foam strip shall be placed in the joint to retain the sealant and isolate it from the filler material. The sealant materials shall then be mixed and installed in accordance with the manufacturer’s directions. Any material that fails to bond the sides of the joint within 24 hours after placement shall be removed and replaced.

Water stops

Water Stops shall be obtained from a manufacturer approved by the Engineer, and shall be fixed and joined according to the manufacturer’s instructions. All strips shall be stored in a place as cool as practicable and shall in no case be exposed to the direct sunlight.

Water Stops shall be installed with approximately half of the width of the material embedded in the concrete on either side of the joint. It shall be firmly supported by split stop-end shuttering and in no case shall Water Stop be pierced to assist in fixing. Special care shall be taken to ensure that the concrete is well worked against the embedded parts of the strips and is free from honeycomb. Precautions are to be taken to protect any projected portions of the strips from damage during the progress of the works and from sunlight and heat.

If, after placing concrete, Water Stops are moved out of position or shape, the surrounding concrete shall be removed, the Water Stop reset, and the concrete replaced at the Contractor’s own expenses. Two 9mm diameter reinforcing bars shall be provided to support the Water Stops and shall be securely held in position by the use of spacers, supporting wires, or other approved devices.

Flexible Water Stops shall be fully supported in the formwork, free from nails and clear of reinforcement and other fixtures. Damaged Water Stops shall be replaced and care shall be taken to place the concrete so that Water Stops do not bend or distort.

Splicing of Poly Vinyl Chloride Water Stop shall be performed in accordance with the manufacturer’s recommendations. A thermostatically controlled electric source of heat shall be used to make all splices. The heat shall be sufficient to melt but not to char the plastic. Splices shall develop at least 90% of the
tensile strength of un-spliced materials and shall withstand bending 180° around a 50mm diameter pin without cracking or separating.

The Contractor, at least before the commencement of concrete work, shall submit to the Engineer for his approval details of the Contractor’s proposals for the installation of water stops. These shall show where joints in the Water Stops are to be located and details of the intersections and changes of direction to a scale that shows the position of any joint or shape of any mould section.

As far as possible, jointing of PVC Water Stops on Site shall be confined to the making butt joints in straight runs of Water Stops. Where it is agreed with the Engineer that it is necessary to make an intersection or change of direction of any joint other than a butt joint in a straight run, a preliminary joint, intersection or change of direction piece shall be made and subjected to such tests as the Engineer may require.

Precautions shall be taken so that the Water Stops shall neither be displaced nor damaged by construction operations or other means. All surfaces of the Water Stops shall be kept free from oil, grease, dried mortar or any other foreign matters while the Water Stop is being embedded in concrete. Means shall be used to ensure that all portions of the Water Stop designed for embedding shall be tightly enclosed by dense concrete.

11.3 Measurement

Construction Joints shall not be measured. Expansion and Contraction joints shall be measured in linear meter of the joints considered satisfactory by the Engineer and accepted by him. There will be no additional measurement for joint fillers, sealed joints, Water Stops, miscellaneous metal devices etc.

11.4 Payment

Payment for construction joints shall be deemed included in the items of concrete and there will be no extra payment for it. For expansion and Contraction joints the amount of completed and accepted works measured as provided above shall be paid at the Contract Unit Price per linear meter and the payment shall constitute the full compensation for furnishing and placing joint fillers, sealed joints, Water Stops, drains, vents, miscellaneous metal devices including all labour and incidentals for full completion of the Work as per Specifications.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Expansion joints</td>
<td>Linear meter</td>
</tr>
<tr>
<td>Contraction joints</td>
<td>Linear meter</td>
</tr>
</tbody>
</table>
SECTION-12
REPAIR OF EXISTING CONCRETE STRUCTURES
REPAIR OF EXISTING CONCRETE STRUCTURES

CONTENTS

SECTION-12  

<table>
<thead>
<tr>
<th></th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>12.1</td>
<td>Description</td>
<td>124</td>
</tr>
<tr>
<td>12.2</td>
<td>Materials</td>
<td>124</td>
</tr>
<tr>
<td>12.3</td>
<td>Construction methods</td>
<td>124-125</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>124</td>
</tr>
<tr>
<td></td>
<td>Repair of existing localized defective concrete</td>
<td>124</td>
</tr>
<tr>
<td></td>
<td>Repair to concrete surfaces</td>
<td>124-125</td>
</tr>
<tr>
<td></td>
<td>Repair of minor cracks in concrete</td>
<td>125</td>
</tr>
<tr>
<td></td>
<td>Anchoring and tying</td>
<td>125</td>
</tr>
<tr>
<td>12.4</td>
<td>Measurement</td>
<td>125</td>
</tr>
<tr>
<td>12.5</td>
<td>Payment</td>
<td>125</td>
</tr>
</tbody>
</table>
SECTION-12
REPAIR OF EXISTING CONCRETE STRUCTURES

12.1 Description
This work shall consist of re-construction of the existing localized defective concrete (including that associated with small extensions of existing concrete structures), rehabilitation of existing concrete facing, repair of minor cracks in structural concrete and anchoring and tying of existing structural members.

The work shall be carried out in accordance with these Specifications and to the locations, lines and dimensions shown on the Drawings and/or as required by the Engineer.

Any extension work associated with repair of existing localized defective concrete is covered under Sections titled ‘Concrete Work’ and ‘Reinforcing Steel’ of this Specification and the extension work shall be measured and paid for under those Sections.

12.2 Materials
Concrete shall conform to the specifications contained in the Sub-section on ‘Concrete for structures’ of this Specification.

Cement mortar shall comply with the specifications contained in the Sub-section on ‘Brick masonry work’ of this Specification except that the mix may vary as shown on the Drawings and/or as directed by the Engineer.

12.3 Construction methods
General
The Contractor and the Engineer shall jointly survey structures to be repaired and the location of all repairs shall be permanently marked in paint on each structure. The repair works shall be carried out by skilled and experienced personnel well conversant with this work.

Repair of existing localized defective concrete
Where existing defective concrete is to be repaired or extended, the existing concrete shall be carefully broken to ensure that all defective materials are removed and that, where necessary, sufficient reinforcement is exposed.

All loose concrete shall be removed, the exposed reinforcement shall be carefully cleaned and the exposed concrete shall be cleaned of all dusts. A construction joint shall be prepared on the exposed face to ensure a good feature between the existing section and the repaired/extension works.

The prepared faces shall be inspected and approved by the Engineer before new work commences.

Repair to concrete surfaces
Defective concrete on the face of substructure walls, in soffits to beams, slabs and other superstructure and on the web faces of main beams and other superstructures shall be carefully removed in a sequence and in accordance with the strict instructions of the Engineer. Such works shall be permanently supervised by a representative of the Engineer and the Contractor shall ensure that technical staffs are permanently available at the Site to receive instructions. The structural integrity of the existing members shall not be impaired and the Contractor shall be fully responsible to ensure that strict procedures are followed. Defective concrete shall be carefully and cleanly removed by manual methods using hammers.
and chisels. The concrete exposed shall be cleaned of all dust and loose materials. Any reinforcement shall be carefully cleaned using wire brushes unless otherwise instructed by the Engineer.

The removed concrete shall be replaced by a method proposed by the Contractor and approved by the Engineer after inspection of the exposed work. The Contractor shall demonstrate that the method he proposes to adopt, is capable of giving a face equivalent to the workmanship standard that would be accepted in new works.

**Repair of minor cracks in concrete**

Minor cracks shall be cleaned to remove all loose materials to expose a sound surface. On approval by the Engineer of the cleared crack, it shall be grouted to full depth with cement mortar and trimmed flush with the face of the concrete.

**Anchoring and tying**

Structural concrete members that exhibit cracking and relative movement may be anchored or tied as instructed by the Engineer. Prior to commencing work, the Contractor shall obtain the approval of the Engineer of the methods to be followed. This shall cover the provision of temporary stages, the drilling methods, safety measures, anchoring methods and subsequent testing for ground anchors to ensure tie bars capable of carrying twice the working load, stressing methods and ultimate grouting of anchor bars. The Contractor shall take instructions from the Engineer on the precise requirements for the provision, installation and anchoring of all tie bars incorporated in the Work.

### 12.4 Measurement

Measurement for repair of defective concrete shall be taken in cubic meters of volume actually marked up, replaced to original lines and accepted by the Engineer.

Measurement of concrete surface repaired shall be taken in square meters of surface areas marked up, repaired and accepted by the Engineer.

Repair of minor cracks shall be measured as the length in linear meters marked up, grouted and accepted by the Engineer.

Anchoring and tying shall be measured as the weight in Kg of anchors and tie bars ordered, installed and accepted by the Engineer.

### 12.5 Payment

The works measured as provided above shall be paid at the relevant Contract unit prices per cubic meter, per square meter, per linear meter and per Kg as applicable. The payment shall constitute the full compensation for all works including all materials, preparatory works and removal of defective materials, temporary works, all labour, equipment, tools and incidentals necessary to complete the Work meeting the prescribed specifications and to the entire satisfaction of the Engineer. For anchoring and tying, the payment shall also be the full compensation for fixing or drilling, installation, grouting in stages and stressing.

<table>
<thead>
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<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Repair of existing defective concrete</td>
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<tr>
<td>Repair of existing concrete surfaces</td>
<td>Square meter</td>
</tr>
<tr>
<td>Repair of minor cracks in existing concrete</td>
<td>Linear meter</td>
</tr>
<tr>
<td>Supplying and fixing anchors and tie bars</td>
<td>Kg.</td>
</tr>
</tbody>
</table>
SECTION-13
REINFORCING STEEL
### REINFORCING STEEL

#### CONTENTS

**SECTION-13**

<table>
<thead>
<tr>
<th>13.1</th>
<th>Reinforcement for RCC</th>
<th>126-140</th>
</tr>
</thead>
<tbody>
<tr>
<td>13.1.1</td>
<td>Description</td>
<td>126</td>
</tr>
<tr>
<td>13.1.2</td>
<td>Materials</td>
<td>126-127</td>
</tr>
<tr>
<td></td>
<td>Reinforcement</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Reference standards</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Deformed reinforcement</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Plain reinforcement</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Smooth steel wire</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Cold – worked steel reinforcement</td>
<td>126</td>
</tr>
<tr>
<td></td>
<td>Mild steel plain round bar</td>
<td>127</td>
</tr>
<tr>
<td></td>
<td>Deformed bars</td>
<td>127</td>
</tr>
<tr>
<td></td>
<td>Other bars</td>
<td>127</td>
</tr>
<tr>
<td>13.1.3</td>
<td>Chemical composition:</td>
<td>127</td>
</tr>
<tr>
<td>13.1.4</td>
<td>Process</td>
<td>127</td>
</tr>
<tr>
<td>13.1.5</td>
<td>Dimensional requirements</td>
<td>127-128</td>
</tr>
<tr>
<td>13.1.6</td>
<td>Tensile properties</td>
<td>128</td>
</tr>
<tr>
<td>13.1.7</td>
<td>Bend test requirement</td>
<td>128-129</td>
</tr>
<tr>
<td></td>
<td>Permissible variation</td>
<td>129</td>
</tr>
<tr>
<td></td>
<td>Length</td>
<td>129</td>
</tr>
<tr>
<td>13.1.8</td>
<td>ASTM code requirements for deformations</td>
<td>129</td>
</tr>
<tr>
<td>13.1.9</td>
<td>Binding wire</td>
<td>130</td>
</tr>
<tr>
<td>13.1.10</td>
<td>Wire mesh</td>
<td>130</td>
</tr>
<tr>
<td>13.1.11</td>
<td>Ordering material</td>
<td>130</td>
</tr>
<tr>
<td>13.1.12</td>
<td>Tests</td>
<td>130</td>
</tr>
<tr>
<td>13.1.13</td>
<td>Construction methods of reinforcing bar</td>
<td>130-133</td>
</tr>
<tr>
<td></td>
<td>Storage and care</td>
<td>130-131</td>
</tr>
<tr>
<td></td>
<td>Fabrication:</td>
<td>131</td>
</tr>
<tr>
<td></td>
<td>Cutting and bending</td>
<td>131-132</td>
</tr>
<tr>
<td></td>
<td>Placing, supporting and fastening</td>
<td>132-133</td>
</tr>
<tr>
<td>13.1.14</td>
<td>Lateral reinforcement for columns</td>
<td>133-135</td>
</tr>
<tr>
<td></td>
<td>Spirals</td>
<td>133-134</td>
</tr>
<tr>
<td></td>
<td>Ties</td>
<td>134</td>
</tr>
<tr>
<td></td>
<td>Lateral reinforcement for beams</td>
<td>134-135</td>
</tr>
<tr>
<td>13.1.15</td>
<td>Spacing of reinforcement</td>
<td>135</td>
</tr>
<tr>
<td>13.1.16</td>
<td>Splicing</td>
<td>135-139</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>135</td>
</tr>
<tr>
<td></td>
<td>Lapped splices</td>
<td>135-136</td>
</tr>
<tr>
<td></td>
<td>Welded splices</td>
<td>136</td>
</tr>
<tr>
<td></td>
<td>Splices of deformed bars in tension</td>
<td>137</td>
</tr>
<tr>
<td></td>
<td>Splices of deformed bars in compression</td>
<td>137</td>
</tr>
<tr>
<td></td>
<td>End bearing splices</td>
<td>138</td>
</tr>
<tr>
<td></td>
<td>Special splice requirements for columns</td>
<td>138</td>
</tr>
<tr>
<td></td>
<td>Lap splices in columns</td>
<td>138</td>
</tr>
<tr>
<td></td>
<td>Welded splices or mechanical connectors in columns</td>
<td>138</td>
</tr>
<tr>
<td></td>
<td>End bearing splices in columns</td>
<td>138</td>
</tr>
<tr>
<td>Section</td>
<td>Pages</td>
<td></td>
</tr>
<tr>
<td>---------------------------</td>
<td>-------</td>
<td></td>
</tr>
<tr>
<td>Splices of plain bars</td>
<td>138</td>
<td></td>
</tr>
<tr>
<td>Mechanical anchorage</td>
<td>138-139</td>
<td></td>
</tr>
<tr>
<td>13.1.17 Substitutions</td>
<td>139</td>
<td></td>
</tr>
<tr>
<td>13.1.18 Concrete cover to reinforcement</td>
<td>139</td>
<td></td>
</tr>
<tr>
<td>13.1.19 Protective coating</td>
<td>139</td>
<td></td>
</tr>
<tr>
<td>13.1.20 Bundled bars</td>
<td>139</td>
<td></td>
</tr>
<tr>
<td>13.1.21 Inspection</td>
<td>140</td>
<td></td>
</tr>
<tr>
<td>13.1.22 Measurement</td>
<td>140</td>
<td></td>
</tr>
<tr>
<td>13.1.23 Payment</td>
<td>140</td>
<td></td>
</tr>
<tr>
<td><strong>13.2 Welding</strong></td>
<td>141-145</td>
<td></td>
</tr>
<tr>
<td>13.2.1 General</td>
<td>141</td>
<td></td>
</tr>
<tr>
<td>13.2.2 Workmanship and visual quality requirements</td>
<td>141</td>
<td></td>
</tr>
<tr>
<td>13.2.3 Welding repairs</td>
<td>141-142</td>
<td></td>
</tr>
<tr>
<td>13.2.4 Peening</td>
<td>142</td>
<td></td>
</tr>
<tr>
<td>13.2.5 Electrodes</td>
<td>142</td>
<td></td>
</tr>
<tr>
<td>13.2.6 Cutting and edge preparation</td>
<td>142-143</td>
<td></td>
</tr>
<tr>
<td>13.2.7 Grinding wheels</td>
<td>143</td>
<td></td>
</tr>
<tr>
<td>13.2.8 Qualification of welders and welding operators</td>
<td>143</td>
<td></td>
</tr>
<tr>
<td>13.2.9 Welding methods</td>
<td>143-144</td>
<td></td>
</tr>
<tr>
<td>General</td>
<td>143</td>
<td></td>
</tr>
<tr>
<td>Welding of stainless steel</td>
<td>143</td>
<td></td>
</tr>
<tr>
<td>Welding of reinforcement</td>
<td>144</td>
<td></td>
</tr>
<tr>
<td>13.2.10 Defects in welded joints</td>
<td>144</td>
<td></td>
</tr>
<tr>
<td>13.2.11 Inspection and testing of welds</td>
<td>144-145</td>
<td></td>
</tr>
<tr>
<td>13.2.12 Measurement and payment</td>
<td>145</td>
<td></td>
</tr>
</tbody>
</table>
SECTION-13
REINFORCING STEEL

13.1 Reinforcement for RCC

13.1.1 Description

Works covered by this item shall consist of supplying and placing of steel reinforcement in different types of concrete structures including board cast-in-situ piles and pre-cast concrete piles but not includes reinforcement for pre-stressed concrete. The works shall conform to the specifications, the types, sizes and positions of reinforcement requirements shown on the Drawings and this specification.

13.1.2 Materials

Reinforcement

Reinforcing bars discussed under this Section shall be made of Mild Steel or High yield Steel, plain or deformed, for all Reinforced Concrete Works but excluding Pre-stressing Concrete.

Bars shall be rolled and produced from steel in the form of new and clean billets directly reduced from ingot of properly identified heats of open hearth, basic oxygen or electric arc furnace steel or lots of acid besmear steel.

Reference standards

Deformed reinforcement

Steel Bars and Wires for the Reinforcement of Concrete – BDS 1313
Rolled Deformed Steel Bars (intermediate grade) for Concrete Reinforcement – BDS 580
Deformed and Plain Billet Steel Bars for Concrete Reinforcement – ASTM A 615
Rail Steel Deformed and Plain Bars for Concrete Reinforcement – ASTM A 616
Axle Steel Deformed and Plain Bars for Concrete Reinforcement – ASTM A 617
Low Alloy Steel Deformed Bars for Concrete Reinforcement – ASTM A 706
Deformed Steel Wire – ASTM A 496
Welded Deformed Steel Wire Fabric – ASTM A 497
Zinc Coated (Galvanized) Steel Bars – ASTM A 767
Epoxy – Coated Reinforcing Steel – ASTM A 775

Plain reinforcement

BDS 1313, ASTM A 615 M, ASTM A 616 M, ASTM A 617 M, ASTM A 185

Smooth steel wire

Cold – Drawn Steel Wire - ASTM A 82

Cold – worked steel reinforcement

IS 1786: 1985, BS 4461: 1978
Mild steel plain round bar

This is a type of bar plain and round in shape of a structural or intermediate grade with yield strength of not less than 280 MPa (N/mm²) i.e. 40 grade.

Deformed bars

Reinforcing steel under this type comprises Mild Steel Grade 40 and High Strength Grade 60 Deformed re-bars with yield strength of not less than 280 MPa (N/mm²) in case of Grade 40 and with yield strength of not less than 410 MPa (N/mm²) in case of Grade 60.

Other bars

Steel welded wire, fabric plain reinforcement conforming to ASTM A 185 may be used, except that for wire with specified yield strength f_y exceeding 410 MPa (N/mm²), f_y will be the stress corresponding to a strain of 0.35 percent.

Smooth steel wire conforming to ASTM A 82 may be used in concrete except that for a wire with a specified yield strength f_y exceeding 410 MPa (N/mm²), f_y will be the stress corresponding to a strain of 0.35 percent.

Fabricated deformed steel bar mats conforming to ASTM A 184 and deformed steel wire complying with ASTM A 496 may be used. Deformed wire for concrete reinforcement shall not be smaller than a nominal diameter of 5.72mm, and for a wire with specified yield strength (f_y) exceeding 410 MPa (N/mm²), f_y shall be the stress corresponding to a strain of 0.35 percent.

Welded deformed steel wire fabric conforming ASTM A 497 may be used for a wire with specified yield strength exceeding (f_y) 410 MPa (N/mm²), f_y will be the stress corresponding to a strain of 0.35 percent.

13.1.3 Chemical composition

The structural grade shall be made from billets. The ends of the bar shall be machine sheared perpendicular to the axis of the bar. The bars shall be free from injurious defects and shall have a workman like finish.

The chemical composition should conform to the requirements of ASTM 706-82.

13.1.4 Process

The steel shall have been made by one or more of the following processes:

- open-hearth
- basic oxygen
- electric furnace
- acid besmear

13.1.5 Dimensional requirements

The nominal diameter, cross sectional areas and perimeter of a deformed bar are equivalent to that of a plain bar having the same standard weight per unit length. Dimensional requirements of such bars have been shown in the Table given below:
### Table: Nominal Dimensions**

<table>
<thead>
<tr>
<th>Bar Designation No.*</th>
<th>Nominal Dimensions**</th>
<th>Nominal weight, lb/ft [Nominal mass, kg/m]</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Diameter, in. [mm]</td>
<td>Cross Sectional Area, in.² [mm²]</td>
</tr>
<tr>
<td>3 [10]</td>
<td>0.375 [9.5]</td>
<td>0.11 [71]</td>
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<tr>
<td>4 [13]</td>
<td>0.500 [12.7]</td>
<td>0.20 [129]</td>
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<tr>
<td>5 [16]</td>
<td>0.625 [15.9]</td>
<td>0.31 [199]</td>
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<tr>
<td>7 [22]</td>
<td>0.875 [22.2]</td>
<td>0.60 [387]</td>
</tr>
</tbody>
</table>

* Bar numbers are based on the number of eighths of an inch including in the nominal diameter of the bars [bar numbers approximate the number of millimeters of the nominal diameter of the bar].

** The nominal dimension of a deformed bar are equivalent to those of a plain round bar having the same weight [mass] per foot [metre] as the deformed bar.

### 13.1.6 Tensile properties

The tensile properties of the Grade 40 and Grade 60 steel have been shown in the Table given below:

<table>
<thead>
<tr>
<th>Item</th>
<th>Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tensile strength, min, psi [MPa]</td>
<td>Grade 40 [300]* 90,000 [620]</td>
</tr>
<tr>
<td>Yield strength, min, psi [MPa]</td>
<td>Grade 40 [300]* 40,000 [300]</td>
</tr>
<tr>
<td>Elongation in 8 in. [203.2 mm], min, %</td>
<td>Grade 40 [300]* 9</td>
</tr>
<tr>
<td>Elongation in 8 in. [203.2 mm], min, %</td>
<td>Grade 40 [300]* 9</td>
</tr>
</tbody>
</table>

* Grade 40 [300] bars are furnished only in sizes 3 through 6 [10 through 19].

### 13.1.7 Bend test requirement

The pin diameter required for performing bend tests shall conform to ASTM A 615. The following table contains such requirements:

<table>
<thead>
<tr>
<th>Bar Designation No.</th>
<th>Pin Diameter for Bend Tests *</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Grade 40 [300]</td>
</tr>
<tr>
<td>3, 4, 5 [10, 13, 16]</td>
<td>3.5d</td>
</tr>
<tr>
<td>6 [19]</td>
<td>5d</td>
</tr>
<tr>
<td>7, 8 [22, 25]</td>
<td>-</td>
</tr>
<tr>
<td>9, 10, 11 [29, 32, 36]</td>
<td>-</td>
</tr>
<tr>
<td>14, 18 [43, 57]</td>
<td>-</td>
</tr>
</tbody>
</table>

* Test bends 180° unless noted otherwise.

d = Nominal diameter of specimen
**Permissible variation**

For lots from standard weights.  
+ 5% for 6mm dia  
+ 3.5% for 10mm dia and above  
Individual  
+ 6% for all sizes

**Length**

Length of the bar shall be maximum possible, but each bar shall not be less than 12m in length or 45.36 kg in weight whichever is greater.

13.1.8 *ASTM code requirements for deformations*

Deformations shall be spaced along the bar at substantially uniform distances. The deformations on the opposite sides of the bar shall be similar in size and shape.

The deformations shall be placed with respect to the axis of the bar so that the included angle is not less than 45°. Where the line of deformation forms an included angle with the axis of the bar from 45° to 70° inclusive, the deformations shall alternately reverse in direction on each side, or those on one side shall be reversed in direction from those on the opposite side. Where the line of deformation is over 70°, a reversal in direction is not required.

Average spacing or distance between deformations on each side of the bar shall not exceed 17 (seventeen) times of the nominal diameter of the bar.

Overall length of deformations shall be such that the gap between the ends of the deformations on the opposite sides of the bar shall not exceed 12.5% of the nominal perimeter of the bar. Where the ends terminate in a longitudinal rib, the width of the longitudinal rib shall be considered as the gap. Where more than two longitudinal ribs are involved, the total width of all longitudinal ribs shall not exceed 25% of the nominal perimeter of the bar. Furthermore, the summation of gaps shall not exceed 25% of the nominal perimeter of the bar. Nominal perimeter of the bar shall be 3.14 times the nominal diameter (d). 

Spacing, height and gap of deformations as to be conformed have been shown in the following table:

<table>
<thead>
<tr>
<th>Bar designation</th>
<th>Maximum average spacing</th>
<th>Minimum average height</th>
<th>Maximum gap (Chord of 12.5% of Nominal Perimeter)</th>
</tr>
</thead>
<tbody>
<tr>
<td>3 [10]</td>
<td>0.262 [6.7]</td>
<td>0.015 [0.38]</td>
<td>0.143 [3.6]</td>
</tr>
<tr>
<td>4 [13]</td>
<td>0.350 [8.9]</td>
<td>0.020 [0.51]</td>
<td>0.191 [4.9]</td>
</tr>
<tr>
<td>5 [16]</td>
<td>0.437 [11.1]</td>
<td>0.028 [0.71]</td>
<td>0.239 [6.1]</td>
</tr>
<tr>
<td>6 [19]</td>
<td>0.525 [13.3]</td>
<td>0.038 [0.97]</td>
<td>0.286 [7.3]</td>
</tr>
<tr>
<td>7 [22]</td>
<td>0.612 [15.5]</td>
<td>0.044 [1.12]</td>
<td>0.334 [8.5]</td>
</tr>
<tr>
<td>8 [25]</td>
<td>0.700 [17.8]</td>
<td>0.050 [1.27]</td>
<td>0.383 [9.7]</td>
</tr>
<tr>
<td>9 [29]</td>
<td>0.790 [20.1]</td>
<td>0.056 [1.42]</td>
<td>0.431 [10.9]</td>
</tr>
<tr>
<td>10 [32]</td>
<td>0.889 [22.6]</td>
<td>0.064 [1.63]</td>
<td>0.487 [12.4]</td>
</tr>
<tr>
<td>11 [36]</td>
<td>0.987 [25.1]</td>
<td>0.071 [1.80]</td>
<td>0.540 [13.7]</td>
</tr>
<tr>
<td>14 [43]</td>
<td>1.185 [30.1]</td>
<td>0.085 [2.16]</td>
<td>0.648 [16.5]</td>
</tr>
<tr>
<td>18 [57]</td>
<td>1.58 [40.1]</td>
<td>0.102 [2.59]</td>
<td>0.864 [21.9]</td>
</tr>
</tbody>
</table>

**Note:** Any bar that fails to satisfy the aforementioned all requirements is to be treated as plain reinforcement.
13.1.9 **Binding wire**

Reinforcement binding wire shall be the best black annealed mild steel wire and not less than 1.6mm in diameter in approximation/18 - 22 BWG or 26 BWG galvanized iron wire.

13.1.10 **Wire mesh**

Wire mesh shall conform to the requirements of AASHTO Standard Specification M 55 Welded Steel Wire Fabric for Concrete Reinforcement.

13.1.11 **Ordering material**

The name of the proposed supplier of the reinforcement shall be submitted as soon possible to the Engineer for his approval. The Contractor shall submit necessary information concerning the supplier as requested by the Engineer.

Copies of orders placed shall be submitted to the Engineer.

The manufacturer shall submit all requested relevant data on the steel, i.e. breaking strength, yield strength, characteristics on elongation, chemical composition etc., to the Engineer for his approval.

No steel shall be delivered without a certificate guaranteeing the yield stress.

The steel shall be stored and marked in a way that it enables identification of the steel corresponding to each certificate later on.

13.1.12 **Tests**

Test results in addition to those to be submitted by the Contractor and specified above shall be required.

The Contractor shall cut out samples as directed by the Engineer.

The samples shall be tested according to the Engineer’s instructions by an approved Testing Institution, provided that the testing facilities are not available in the LGED Laboratories. Approximately three samples shall be tested from each 10 tons of reinforcement delivered at the Site. Expenses incurred in connection with cutting, carrying and testing the samples shall be borne by the Contractor at his own costs.

13.1.13 **Construction methods of reinforcing bar**

**Storage and care**

All reinforcing steel when received at the Site, prior to its use, shall be stacked off the ground on platforms, skids or any other support and shall be kept free from dirt, oil and grease. All cares shall be taken to prevent the steel reinforcement from any mechanical injury and surface loss resulting from its exposition to weather conditions that produce rust. It shall be clean and kept free from loose rust and loose mill scale at the time of fixing in position and subsequent pouring of concrete. However, reinforcement steel may not be rejected on the ground of bonded rust, surface seams, surface irregularities and mill scale so long minimum dimensions, cross-sectional area and tensile properties of a hand wire brushed specimen meet the specified physical requirements for the size and grade of steel.

Reinforcement shall be handled and stored in a manner that will prevent bending out of the desired shape and any accumulation of dirt, oil and paint. When placed in the works, it shall be free from dirt, oil, grease, paint, mill scale and loose or thick rust.
Bar reinforcement shall be shipped in standard bundles, tagged and marked in accordance with the Codes of Practice of the Concrete Reinforcing Steel Institute.

**Fabrication**

All bars shall be fabricated following Specifications, methods and procedures stated below. Fabrication tolerances shall be in accordance with ACI 315.

**Cutting and bending**

All reinforcement bars shall be cut and bent cold to the specified shape and pertinent dimensions shown on the Drawings using a proper bar bender, operated by hand or power to attain proper radii of bends. The equipment used and methods followed for this purpose shall get the approval of the Engineer.

Bars shall not be bent or straightened in a manner that will injure the material.

Bars partially embedded in concrete shall not be field bent unless otherwise shown on the Drawings or directed by the Engineer.

Errors in alignment of reinforcement partially embedded in hardened concrete shall not be corrected by bending in place, except as permitted by the Engineer.

Bars bent during transportation or handling shall be straightened before being used in work. It shall not be heated to facilitate bending.

Fabrication tolerances shall be in accordance with ACI 315.

All plain bars shall have standard hooks at the end, which shall meet the following requirements unless otherwise specified on the Drawings. When the dimensions of hooks or the diameter of bends are not prescribed, they shall be in accordance with ACI 318 ‘Building Code requirements for Reinforced Concrete’. Some of the standard requirements have been specified below:

- 180° turn plus an extension of at least 4 bar diameters but not less than 60mm at the free end of the bar.
- 90° turn plus an extension of at least 12 bar diameters at the free end of the bar.

For stirrup and the anchorage only:

- For 16mm \( \phi \) bar and smaller: 90° bend plus an extension of at least 6 bar diameters or 75mm whichever is greater at the free end of the bar.
- For 20mm \( \phi \) and 25mm \( \phi \) bar: 90° bend plus an extension of at least 12 bar diameters or 150mm whichever is greater at the free end of the bar.
- For 25mm \( \phi \) bar and smaller: 135° bend plus an extension of at least 6 bar diameters at the free end of the bar.
- For closed ties and continuously wound ties: 135° bend plus an extension of at least 6 bar diameters, but not less than 75mm.

The minimum diameter of bend measured on the inside of the bar, for standard hooks other than for stirrups and ties in sizes 10mm \( \phi \) through 16mm \( \phi \), shall not be less than the values shown in the table given below.
Minimum diameters of Bend

<table>
<thead>
<tr>
<th>Bar size</th>
<th>Minimum diameter of bend</th>
</tr>
</thead>
<tbody>
<tr>
<td>10mm (\leq d_b \leq 25\text{mm})</td>
<td>6(d_b)</td>
</tr>
<tr>
<td>25mm (\leq d_b \leq 40\text{mm})</td>
<td>8(d_b)</td>
</tr>
<tr>
<td>40mm (\leq d_b \leq 55\text{mm})</td>
<td>10(d_b)</td>
</tr>
</tbody>
</table>

* \(d_b\) is the nominal diameter of bar, mm

For stirrups and tie hooks, inside diameter of bend shall not be less than 4 bar diameters for 16mm \(\phi\) bar and smaller. For bars larger than 16mm \(\phi\), diameter of bend shall be in accordance with the specifications shown in the above table.

Bends for other bars, where full tension in the bar may occur, shall be made around a pin having a diameter not less than 20 bar diameters. Hooks shall conform to American Concrete Institute Standard Building Code Requirements for reinforced concrete ACI 316-89, or as shown on the Drawings or as directed by the Engineer.

Placing, supporting and fastening

All bar reinforcement shall be accurately placed, supported and secured in position as shown on the Drawings using approved spacer blocks and chairs prior to any concrete pouring. Displacement tolerance may be allowed within the permissible tolerance limit as shown in the table given below unless otherwise specified by the Engineer. The reinforcement shall be checked and approved by the Engineer before pouring of concrete.

<table>
<thead>
<tr>
<th>Tolerances for Placing Reinforcement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Tolerance for depth ((d))</td>
</tr>
<tr>
<td>------------------------------------</td>
</tr>
<tr>
<td>(d \leq 200\text{mm})</td>
</tr>
<tr>
<td>(d &gt; 200\text{mm})</td>
</tr>
</tbody>
</table>

Notwithstanding the above provisions, tolerance for the clear distance to formed soffits shall be minus 6mm and tolerance for cover shall not exceed minus one-third the minimum concrete cover required in the design Drawings or specifications.

Tolerance for longitudinal location of bends and ends of reinforcement shall be \(\pm\ 50\text{mm}\), except at discontinuous ends of members where tolerance shall be \(\pm\ 12\text{mm}\).

Welding of crossing bars shall not be permitted for assembly of reinforcement unless authorized by the Engineer.

The Contractor shall be responsible for the accuracy of cutting, bending and placing of the reinforcement. Reinforcement will be inspected for compliance with the requirements as to grade, size, shape, length, splicing locations, overlapping length and position after it has been placed.

Before the reinforcement is placed, the surfaces of the bars and the surfaces of any metal bar supports shall be cleaned of heavy rust, loose mill scale, dirt, grease and other objectionable foreign substances. Heavy flaky rust, which can be removed in firm rubbing with hessian or equivalent treatment, shall be considered objectionable. After being placed, the reinforcing bars shall be maintained in a clean condition until they are completely embedded in the concrete.
Reinforcement shall be accurately placed in the position shown on the Drawings and/or as directed by the Engineer and shall be securely held by blocking against the forms, by supporting on concrete or approved metal or plastic chairs or by using metal hangers and by wiring together at intersections using annealed wire of specified diameter with the ends turned in to the main body of concrete. Bars shall be tied at all intersections except where spacing is less than 300mm in any direction when alternate intersections shall be tied. Wire ties shall be securely tied and folded so that they do not project beyond the planes formed by the reinforcing bars. The adequacy of the supports and ties to secure the reinforcement properly shall be subject to the approval of the Engineer.

Reinforcement supports shall be strong enough to withstand the imposed loads without movement of the reinforcement. They shall be positively attached to the reinforcement and of such size and number as to maintain the specified cover.

There shall be a clear distance of at least 25mm between the bars and any adjacent embedded metal works. The Contractor shall ensure that there is no disturbance of the reinforcing bars in concrete that has already been placed.

Reinforcement binding wire shall be best black annealed mild steel wire and not less than approximately 1.6mm in diameter/18 - 22 BWG galvanized iron wire.

Cover blocks required for ensuring that the reinforcement is correctly positioned shall be as small as possible, consistent with their purpose, or a shape and material acceptable to the Engineer and designated so that they will not overturn when the concrete is placed. The concrete cover blocks or space blocks shall be made of concrete having 1 part cement, 1 part sand and 2 part coarse aggregate. The coarse aggregate would be 6mm down graded. The blocks would be cast in mould and continuously cured for 21 days before use. Wire shall be cast in the block for the purpose of tying it to the reinforcement. The wire must not be closer than 30mm from the concrete surface. The use of small stones or wood blocks shall not be permitted.

If concrete cylinder blocks are used for proper spacing of vertical bars in column, the height shall be 2.54cm and radius shall be equal to the distance of the centre line of the bar from column face.

Top reinforcement in slabs shall be maintained in position by means of chairs made out of ferrous metal and shall conform to industry practice as described in the Manual on ‘Standard Practice of the Concrete Reinforcing Steel Institute’. The diameter and quantity being sufficient to ensure security of the reinforcement shall be used to support access ways, working platforms, or the placing equipment or for conducting of an electric current.

Platforms for the support of workers and equipment and machines shall be placed directly on the forms without any disturbance of the reinforcing steel during concrete placement.

Before any steel reinforcement is embedded in the concrete, any loose mill scale, loose rust and any oil, grease or other deleterious matter shall be removed. Partially set concrete, which may adhere to the exposed bars during concrete placing operations, shall also be removed.

13.1.14 Lateral reinforcement for columns

Spirals

Spiral reinforcement for columns shall conform to the following:

a) Spirals shall consist of evenly spaced continuous bar or wire of such size and so assembled as to permit handling and placing without distortion from designed dimensions.
b) Size of spirals shall not be less than 10mm diameter for cast-in-place construction.

b) The minimum and maximum clear spacing between spirals shall be 25mm and 75mm respectively.

d) Anchorage of spiral reinforcement shall be provided by 1.5 extra turns of spiral bar or wire at each end of a spiral unit.

e) Splices in spiral reinforcement shall be lap splices of 48 spiral diameter, but not less than 300mm.

f) Spirals shall extend from the top of footing or slab in any story to the level of the lowest horizontal reinforcement in members supported above.

g) Spirals shall extend above termination of spiral to bottom of slab or drop panel, where beams or brackets do not frame in to all sides of a column.

h) Spirals shall extend to a level at which the diameter or width of capital is 2 times that of the column, in case of columns with capitals.

i) Spirals shall be held firmly in place and true to line.

Ties

Tie reinforcement for compression members shall conform to the following:

a) All bars shall be enclosed by lateral ties, at least 10mm diameter in size for longitudinal bars 30mm diameter or smaller, and at least 12mm diameter in size for 35mm diameter to 55mm diameter and bundled longitudinal bars.

b) Vertical spacing of ties shall not exceed 16 longitudinal bar diameters or 48 tie diameters, or the least dimension of the compression members.

c) Ties shall be arranged such that every corner and alternate longitudinal bar shall have lateral support provided by the corner of a tie with an included angle of not more than 135°. No vertical bar shall be farther than 150mm clear on each side along the tie from such a laterally supported bar. Where longitudinal bars are located around the perimeter of a circle, a complete circular tie is allowed.

d) The lowest tie in any story shall be placed within one-half the required tie spacing from the top most horizontal reinforcement in the slab or footing below. The uppermost tie in any story shall be within one-half the required tie spacing from the lowest horizontal reinforcement in the slab or drop panel above.

e) Where beams or brackets provide concrete confinement at the top of the column on all (four) sides, the top tie shall be within 75mm of the lowest horizontal reinforcement in the shallowest of such beams or brackets.

Lateral reinforcement for beams

Compression reinforcement in beams shall be enclosed by ties or stirrups satisfying the size and spacing limitations as stated above. Such ties or stirrups shall be provided throughout the distance where compression reinforcement is required.

Lateral reinforcement for flexural framing members subject to stress reversals or to torsion at supports shall consist of closed ties, closed stirrups, or spirals extending around the flexural reinforcement.
Closed ties or stirrups shall be formed in one piece by overlapping standard stirrup or tie end hooks around a longitudinal bar, or formed in one or two pieces laps, spliced with a lap of development length.

13.1.15 Spacing of Reinforcement

The minimum clear spacing between parallel bars in a layer shall be equal to one bar diameter, but not less than 25mm.

Where parallel reinforcement is placed in two or more layers, bars in the upper layers shall be placed directly above those in the bottom layer with clear distance between layers not less than 25mm.

For compression members, the clear distance between longitudinal bars shall be not less than 1.5 bar diameters or 35mm.

Clear distance limitation between bars shall apply also to the clear distance between a contact lap splice and adjacent splices or bars.

In walls and one-way slabs, the maximum bar spacing shall be three times the wall or slab thickness \(h\) but not more than 450mm.

For two-way slabs, maximum spacing of bars shall be \(2h\) but not more than 450mm.

For temperature steel only, maximum spacing shall be \(5h\) but not more than 450mm.

13.1.16 Splicing

General

All reinforcement shall be furnished in the full lengths indicated on the Drawings unless otherwise permitted by the Engineer. Except for splices shown on the Drawings and splices for No. 5 or smaller bars, splicing of bars shall not be permitted without the written approval of the Engineer. Splices shall be staggered as far as possible.

Where the Drawings do not detail laps that will be necessary, the Contractor shall furnish working Drawings to the Engineer for his approval.

If such additional lap splices are approved, the extra weight occasioned by such lap splices shall not be included in the measurement of reinforcement for payment unless provided for in these Specifications.

Lapped splices

All splices for high yield strength steel bars shall have a lap length as shown on the Drawings or if not shown therein shall be in accordance with the American Concrete Institute Building Code Requirements for Reinforced Concrete (ACI 318-89).

All splices for mild steel shall have a lap length as shown on the Drawings or if not shown therein, of not less than 40 diameters of the smaller bar when hooks are used and 50 diameters for bars without hooks.

Lap splices shall not be used for 35mm diameter bars and larger, except when bars of different diameters are lap spliced in compression, the splice length shall be the larger development length of the larger bar, or the splice length of the smaller bar.
Lap splices of bundled bars shall be based on the lap splice length required for individual bars within the bundle, increased in accordance with development of bundled bars. Individual bar splices within a bundle shall not overlap. Entire bundles shall not be lap spliced.

Bars spliced by non-contact lap splices in flexural members shall not be spaced transversely farther apart than one-fifth the required lap splice length, nor 150mm.

Lap splices shall generally be located at points of minimum tension in bars. Except where otherwise shown on the Drawings, lap splices shall be made with the bars placed in contact and securely wired together.

**Welded splices**

Welding on Site shall be avoided wherever possible, but where suitable safeguards and techniques are employed and provided that the types of steel including high-yield steels to SS 2 have the required welding properties, it may be undertaken with the acceptance of the Engineer. Before welding any reinforcement, the Contractor shall supply to the Engineer a Welding Procedure Specification (WPS) and an example of the weld for the type of steel, connection and weld being proposed. If such evidence is not available, the Contractor shall demonstrate satisfactory performance by means of testing as agreed by the Engineer. Unless satisfactory performance of the proposed welded connection is established by either of the two methods described above, approval for use of the welded connection shall not be given.

In addition and as required by the Engineer, the competence of the operators shall be demonstrated prior to and periodically during welding operations by submission of independent Welder Qualification Records (WQR) for each welder to be used on Site.

Welding may be used in fixing reinforcement in position, for example, by welding between crossing or lapping reinforcement, or between bars and other steel members.

Welded intersections shall not be spaced farther apart than 300mm in the direction of calculated stress, except for wire fabric used as stirrups.

Structural welding shall not be carried out unless specifically shown on the Drawings.

Notwithstanding the above, the Engineer will not permit tack welding of bars, which will be subject to fluctuating stresses in the completed structure.

Welding shall conform to the Structural Welding Code, Reinforcing Steel, AWS D 1.4 of the American Welding Society and applicable special provisions.

Welded splices shall be butted and welded to develop in tension at least 125 percent of specified yield strength $f_y$ of the bar. A full mechanical connection shall develop in tension or compression, as required, at least 125 percent of specified yield strength $f_y$ of the bar. Welded splices and mechanical connections not meeting the above requirements are allowed where area of reinforcement is at least twice that required by analysis shall meet the following:

- Splices shall be staggered at least 600mm and in such manner as to develop at every section at least twice the calculated tensile force at the section but not less than 140 N/mm² for total area of reinforcement provided.

- Spliced reinforcement may be rated at the specified splice strength, in computing tensile force developed at each section. Non-spliced reinforcement shall be rated at that fraction of $f_y$ defined by the ratio of the shorter actual development required to develop the specified yield strength ($f_y$).
**Splices of deformed bars in tension**

The minimum length of lap for tension splices shall be as required for Class A or B splice, but not less than 300mm, where the classification shall be as follows:

<table>
<thead>
<tr>
<th>Class</th>
<th>Requirement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Class A splice</td>
<td>$1.0l_d$</td>
</tr>
<tr>
<td>Class B splice</td>
<td>$1.30l_d$</td>
</tr>
</tbody>
</table>

* $l_d$ is the development length

Lap splices of deformed bars in tension, shall be Class-B splices except that Class-A splices are allowed when the area of reinforcement provided is at least twice that required by analysis over the entire length of the splice, and one-half or less of the total reinforcement is spliced within the required lap length.

Where area of reinforcement provided is less than twice that required by analysis, welded splices or mechanical connections used shall meet the following requirements. This is also applicable in case of splices in tension tie members those shall be made with a full welded splice or full mechanical connection.

- Welded splices shall be butted and welded to develop in tension at least 125 percent of specified yield strength $f_y$ of the bar.
- A full mechanical connection shall develop in tension or compression, as required, at least 125 percent of specified yield strength $f_y$ of the bar.

Welded splices or mechanical connections used where area of reinforcement provided is at least twice that required by analysis shall meet the following:

a) Splices shall be staggered at least 600mm and in such manner as to develop at every section at least twice the calculated tensile force at the section but not less than 140 N/mm² for total area of reinforcement provided.

b) Spliced reinforcement may be rated at the specified splice strength, in computing tensile force developed at each section. Non-spliced reinforcement shall be rated at that fraction of $f_y$ defined by the ratio of the shorter actual development length to $l_d$ required to develop the specified yield strength $f_y$.

Splices in adjacent bars shall be staggered at least 750mm.

**Splices of deformed bars in compression**

The minimum length of lap for compression splice shall be 0.07 $f_yd_b$ for $f_y$ equal to 410 N/mm² or less or $(0.13 f_y – 24)d_b$ for $f_y$ greater than 410 N/mm², but not less than 300mm. For $f'_c$ (specified compressive strength of concrete, N/mm²) less than 20 N/mm², length of lap shall be increased by one-third.

When bars of different diameters are lap spliced in compression, the splice length shall be the larger of the development length of the larger bar, or the splice length of the smaller bar.

Welded splices or mechanical connections used in compression shall also satisfy the following requirements:

- Welded splices shall be butted and welded to develop in tension at least 125 percent of the specified yield strength $f_y$ of the bar.
- A full mechanical connection shall develop in tension or compression, as required, at least 125 percent of the specified yield strength $f_y$ of the bar.
End bearing splices

a) Compression splices for bars required to transmit compressive stress only, may consist of end bearing of square cut ends held in concentric contact by a suitable device.

b) Bar ends shall terminate in flat surfaces within 1.5° of a right angle to the axis of the bars, and shall be fitted within 3° of full bearing after assembly.

c) End bearing splices shall be used only in members containing closed ties, closed stirrups or spirals.

Special splice requirements for columns

Lap splices, buttwelded splices, mechanical connections, or end-bearing splices shall be used with the limitations as stated below. A splice shall satisfy the requirements for all load combinations for the column.

Lap splices in columns

a) Lap splices shall conform to the first two requirements stated above under the Sub-section on ‘Splices of Deformed Bars in Compression’ and where applicable to (d) or (e) below where the bar stress due to factored loads is compressive.

b) Where the bar stress due to factored loads is tensile and does not exceed 0.5fy in tension, lap splices shall be Class B tension lap splices, if more than one half of the bars are spliced at any section, or Class A tension lap splices, if half or fewer of the bars are spliced at any section and alternate lap splices are staggered by 1d (development length).

c) Where the bar stress due to factored loads is greater than 0.5fy in tension, lap splices shall be Class B tension lap splices.

d) If spiral reinforcement confines the splice, the lengths required may be multiplied by 0.75, but lap length shall not be less than 300mm.

Welded splices or mechanical connectors in columns

Welded splices or mechanical connectors in columns shall also meet the following requirements.

➢ Welded splices shall be butted and welded to develop in tension at least 125 percent of specified yield strength fy of the bar.

➢ A full mechanical connection shall develop in tension or compression, as required, at least 125 percent of specified yield strength fy of the bar.

End bearing splices in columns

End bearing splices complying with the requirements stated above under Sub-section on “End Bearing Splices” may be used for column bars stressed in compression provided that the splices are staggered or additional bars are provided at splice locations. The continuing bars in each face of the column shall have a tensile strength at least 0.25fy, times the area of the vertical reinforcement in that face.

Splices of plain bars

For plain bars, the minimum length of lap shall be twice that of deformed bars.

Mechanical anchorage

Any mechanical device capable of developing the strength of reinforcement without damage to concrete is allowed as anchorage.
Mechanical device may be used only when its adequacy can be proven by test results to the satisfaction of the Engineer.

Development of reinforcement may consist of a combination of mechanical anchorage plus additional embedded length of reinforcement between the point of maximum bar stress and the mechanical anchorage.

13.1.17 Substitutions

Substitutions of different size bars shall be permitted only with specific authorization by the Engineer and at no additional cost to the Employer. If bars are substituted, they shall have a cross sectional area equivalent to the design area or larger.

The Contractor shall also provide, also in the case of substitutions, at his own expenses and to the approval of the Engineer, such necessary detailing of the reinforcement as he require for the execution of the work to the Engineer’s satisfaction.

13.1.18 Concrete cover to reinforcement

Unless specified on the Drawings, the clear concrete cover to reinforcement shall be as tabulated below:

<table>
<thead>
<tr>
<th>Description of Concrete Element</th>
<th>Clear Cover (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Normal Exposure</td>
</tr>
<tr>
<td>Wall and footing</td>
<td></td>
</tr>
<tr>
<td>a) contact with earth</td>
<td>60</td>
</tr>
<tr>
<td>b) exposed to weather and water</td>
<td>50</td>
</tr>
<tr>
<td>Piles</td>
<td></td>
</tr>
<tr>
<td>a) cast-in-place</td>
<td>75</td>
</tr>
<tr>
<td>b) pre-cast</td>
<td>40</td>
</tr>
<tr>
<td>Beam, Girder, Column</td>
<td>40</td>
</tr>
<tr>
<td>Building roof and floor slab</td>
<td>25</td>
</tr>
</tbody>
</table>

13.1.19 Protective coating

All exposed reinforcing steel at construction joints shall be protected with a brush coat of neat cement mixed to a consistency of thick paint within one week after the placing of the initial concrete, unless it is definitely known that the steel will be embedded within 30 days. This coating shall be entirely removed, by light tapping with a hammer or other tools, not more than one week before the placing of the final pour.

13.1.20 Bundled bars

a) Groups of parallel reinforcing bars bundled in contact to act as one unit, shall be limited to four in any one bundle.

b) Bundled bars shall be enclosed within stirrups or ties.

c) Bars larger than 35mm diameter shall not be bundled in beams.

d) Individual bars within a bundle terminated within the span of flexural members shall terminate at different points with at least 40 times the nominal diameter of bar staggered.

e) Where spacing limitations and minimum concrete cover are based on nominal bar diameter, a unit of bundled bars shall be treated as a single bar of a diameter derived from the equivalent total area.

f) Minimum concrete cover shall be equal to the equivalent diameter of the bundle, but need not be greater than 50mm.
13.1.21 Inspection

The Contractor shall notify the Engineer when the steel has been placed in position and ready for concrete placing. No concrete shall be placed until the Engineer inspected the steel and given his approval in writing.

13.1.22 Measurement

The quantity of reinforcement to be measured under this Section shall be the computed weight in kilogram of material used and accepted as shown on the Drawings provided that the quantity shall not include the reinforcement in any item of works. In computing the weight to be measured, the theoretical weights of bars of the cross section shown in this Specification shall be used.

The computed weight shall not include the extra materials incurred, when bars larger than those specified are used or the extra materials necessary for splices, when bars shorter than those specified are used even with the permission of the Engineer. It shall not also include the weight of any devices used to support or fasten the reinforcement in correct position.

13.1.23 Payment

This work measured as provided above, shall be paid for at the Contract unit price per kilogram of reinforcement for the particular Bill of Item. The payment shall be considered to be the full compensation for furnishing, fabricating, splicing and placing of the reinforcing steel, supports and binding wire, cutting and bending, all labours, equipment, tools and incidentals necessary to complete the works prescribed in this Section.

No separate payment shall be allowed for chairs, laps, splices, separators etc. The costs of these shall be considered included in the unit rate.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Mild steel reinforcing bars</td>
<td>Kilogram</td>
</tr>
<tr>
<td>High yield steel reinforcing bars</td>
<td>Kilogram</td>
</tr>
</tbody>
</table>
13.2 **Welding**

13.2.1 **General**

All welding shall be performed by certified welders and in accordance with the American Welding Society (AWS) D1.1 ‘Structural Welding Code’ or similar approved standard.

The principal forms of welding metals are as follows:

- Electric arc welding
- Gas welding

The electric arc welding process is the most important and is most extensively used for mild steels ranging from light articles with a wall or thickness of 16 gauge to heavy fabrications. This is a process whereby the metal of the two members to be welded is fused together through heat generated by an electric arc. Fusion should be complete over the whole area of the joint surface.

Gas welding is done using oxy-acetylene flame and is not adapted to structural steel works, but is generally used for small jobs. The flame produced by burning oxy-acetylene is fed through a blowpipe, which is ignited at its tip. The flame is played on the two pieces to be welded until the metal becomes hot enough to fuse together adding additional metal to the joint as necessary by melting in to it a suitable electrode.

Unless otherwise specified, all welding shall be performed following the Shielded Metal Arc Process with low hydrogen electrodes for manual welding.

The Contractor shall be responsible for the quality of the welding performed by his welding organization. All welding by the Contractor shall be carried out by the electric arc method using coated electrodes or other means whereby the air is excluded from the molten metal and where applicable, automatic machines with correct procedure control shall be used.

13.2.2 **Workmanship and visual quality requirements**

In addition to conforming with the procedural and quality requirements set forth in the Structural Welding Code and/or these Specifications, all manual welding shall meet the following requirements for workmanship and visual quality.

(a) Each weld shall be uniform in width and size throughout its full length and each layer of welding shall be smooth, free of slag, cracks, pinholes and undercut and shall be completely fused to the adjacent weld beads and base metal. In addition, the cover pass shall be free of coarse ripples, irregular surface, non-uniform bead pattern, high crown, deep ridges or valleys between beads and shall blend smoothly and gradually into the surface of the base metal.

(b) Butt Welds shall be slightly convex, of uniform height and shall have full penetration.

(c) Fillet Welds shall be of specified size with full throat and with each leg of uniform length.

(d) Repair, chipping or grinding of welds shall be done in such a manner as not to gouge, groove, or reduce the base metal thickness.

13.2.3 **Welding repairs**

All weld defects which are determined unacceptable, shall be removed by chipping, grinding, arc or flame gouging, following which the area shall be properly prepared for welding, repaired by an approved
qualified welding procedure and re-tested as necessary. The Contractor shall establish the cause of all
defects and show that such defects have been corrected before welding will be permitted. All repairing
shall be done by and at the expenses of the Contractor.

13.2.4 Peening

The Contractor shall not be allowed to peen welds without prior approval of the Engineer.

13.2.5 Electrodes

All electrodes shall be purchased in sealed containers and shall be thoroughly dry when used.
Electrodes, taken from sealed containers, shall be used within four hours. Electrodes not used within four
hours shall be stored in electrode storage ovens. The electrode storage oven temperature shall be in
accordance with the electrode manufacturer’s recommendations. Electrodes with wet or damaged
coatings shall not be used.

A simple test indicate the quality of an electrode or welding or welding wire can be made by laying the
wire flat on a clean surface and applying the welding flame to it for a distance of about 8 - 10cm by
moving the flame backward and forward until the wire becomes red and then slowly melting the wire,
moving the flame in such a manner so that the wire melts only half-way thorough its diameter. If the
flame is withdrawn as soon as the rod metal begins to melt, the impurities can readily be seen being
thrown off in the form of sparks, or a boiling action in the case of inferior metal. When cold, an inferior
metal will contain numerous spongy, volcano-like irregularities. A good metal welding rod will melt and
flow evenly without any disturbing actions.

Cracks may occur in welding alloy steels owing to the rapidity with which these harden. This may largely
be avoided by preheating the parent metal at 300°C or above in advance of welding to lower the normal
cooling rate.

The maximum diameter of electrodes for welding have been shown in the following table:

<table>
<thead>
<tr>
<th>Average thickness of plate or section</th>
<th>Maximum gauge or diameter of electrode to be used</th>
</tr>
</thead>
<tbody>
<tr>
<td>Less than 5mm</td>
<td>3.2mm – 10 SWG</td>
</tr>
<tr>
<td>5mm to less than 8mm</td>
<td>4mm – 8 SWG</td>
</tr>
<tr>
<td>8mm to less than 10mm</td>
<td>5mm – 6 SWG</td>
</tr>
<tr>
<td>10mm to less than 16mm</td>
<td>6mm – 4 SWG</td>
</tr>
<tr>
<td>16mm to less than 25mm</td>
<td>9mm</td>
</tr>
<tr>
<td>25mm and over</td>
<td>9mm</td>
</tr>
</tbody>
</table>

The maximum width of any bead of welding, other than a cover pass, shall not exceed 3 times the
diameter of the electrode being used.

Subject to the approval of the Engineer, electrodes shall be carefully selected in order to provide metal
welds with mechanical properties similar to those of the metal being welded, except that for welding
higher strength steel to lower strength steel, the electrodes shall be chosen to provide metal welds with
mechanical properties comparable to those of the lower strength material.

13.2.6 Cutting and edge preparation

Members of structural steel and miscellaneous metal works, which are to be joined by welding shall be
cut accurately to size and where required, shall be rolled or pressed to the proper curvature in
accordance with dimensions shown. The edges of these members shall be sheared, flame-cut or
machined to suit the required type of welding and to allow thorough penetration. The cut surfaces shall expose sound metal, free from laminations, surface defects caused by shearing or flame-cutting operations, or other injurious defects. The surface to be welded shall be free from rust, grease, paint and other foreign matter for a distance of at least 150mm back from the edge of the weld.

13.2.7 Grinding wheels

Grinding wheels, which leave a deposit detrimental to subsequent welding will not be permitted. Grinding wheels, which are determined by the Engineer to be detrimental to welding shall not be used.

13.2.8 Qualification of welders and welding operators

All welders and welding operators assigned to the work shall have passed the qualification test for welding operators as specified in the AWS Structural Welding Code. If, as determined by the Engineer, the work of any welder appears questionable, such welder will be required to pass additional qualification tests to determine his ability to perform the type of work on which he is engaged. Such additional qualification tests for welders and the physical tests of the welded specimens shall be made in the presence of the Engineer. If required, the Contractor shall furnish to the Engineer a certified copy of reports of the results of physical tests of specimens welded in the qualification tests. Fulfillment of such qualification shall be at the expenses of the Contractor.

13.2.9 Welding methods

General

Methods which are essentially required to be followed while welding are as follows:

- Welds should be made in the flat position as far as practicable.
- Freedom of movement of one member should be allowed as far as possible.
- The work should be securely held in position by means of spot welds, service bolts, clamps or jigs before commencing welding so as to prevent any relative movement due to distortion, wind or other causes.
- The parts to be welded must be thoroughly cleaned and proper flux used. Any paint or rust and loose mill scales, etc. should be removed from the surfaces to be welded and surrounding materials for a distance of at least 12mm from the weld. A coating of boiled linseed oil may be permitted. Steel to be welded should not be painted or oiled until after erection, unless all ends to be welded are left bare.
- The sequence of welding should be such that when possible the members, offering the highest resistance to compression, are welded first.

Extreme care shall be taken to ensure that correct welding sequences and procedures are observed to avoid any strains and internal stresses arising in welding.

Welding of stainless steel

Unless otherwise specified, all welding shall conform with AWD D1.1. Electrodes used for welding of stainless steel shall be Series E308 and electrodes used for welding of stainless steel to carbon steel shall be Series E309.

Welders and welding operators assigned to the work shall have passed the qualification test for welding operators as specified above under ‘Qualification of Welders and Welding Operators’ of this Sub-section.
Welding of reinforcement

Electric Arc Butt-welding is most suitable for bars of diameter greater than 20mm and lap welding for smaller diameters and lap welding with longitudinal beads for 6mm to 40mm diameters. However, reinforcement, specified to be welded, shall be welded by any process the Contractor can demonstrate by bend and tensile tests, which will ensure that the strength of the parent metal is not reduced and that the weld possesses a strength no less than that of the parent metal. The welding procedure established by the successful weld tests shall be maintained and no departure from this procedure shall be permitted. Following the establishment of a satisfactory welding procedures, each welder to be employed on the work shall carry out welder performance qualification tests on reinforcing bars of the same metal and size as those on the works.

Welds in positions other than those shown on the Drawings and/or as directed by the Engineer shall not be permitted.

13.2.10 Defects in welded joints

The usual defects in welded joints are:

- Lack of penetration or fusion of the metal to the bottom of the joint or welded members.
- Laps in the metal of the weld not properly fused together.

Defects are most likely to occur at the root of the weld and in this position they are liable to have the maximum effects in reducing the strength of the weld.

13.2.11 Inspection and testing of welds

The metal in a good weld when cold should show its original colour. If the metal has a rusty or dull red colour or appears crystallized, it is an indication that the heat has become too high and the metal has been burnt. A good weld will show an evenness of ripples or waves and well formed beads with good fusion along the edges of the welds. There should be no unfilled cavities, small pockets of slags or burnt metal and small air or gas pockets.

The strength of a welded joint may be taken only about 75 per cent of the stress usually allowed for common works, although tests have shown that if the welding is properly done it is possible to develop the full strength of the members jointed.

The following tests shall be carried out on the procedure, qualification, test plates and production test plates:

- Tensile and bend tests: all welds shall be subject to visual inspection.
- The procedures of visual examination shall conform to the requirements of the ASME Boiler and Pressure Vessels Code.

The following defects are unacceptable unless otherwise noted:

- Dimensional defects such as insufficient throat or leg length, excess convexity, excess or insufficient reinforcement.
- Undercuts, overlap, blowholes, slag inclusion, seams and excess weave.
- Any crack or liner indication.
Plates with laminations discovered during gas cutting, welding or any other time shall be rejected, unless approval to repair the plate is obtained from the Engineer.

Welds may also be subject to anyone or a combination of the examinations as may be required to establish the soundness of welds.

The inspection procedures for testing of all welds shall be prepared on the above basis by the Contractor and submitted to the Engineer for approval before any fabrication work is started.

13.2.12 Measurement and payment

Welding shall not be measured and no direct payment shall be made. All costs of welding shall be deemed included in the related items of the Bill of Quantities unless otherwise it has been specifically mentioned in the BOQ.
SECTION-14
SUB-SOIL BORING AND TESTING
# SUB-SOIL BORING AND TESTING

## CONTENTS

<table>
<thead>
<tr>
<th>SECTION-14</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>14.1 General</td>
<td>146</td>
</tr>
<tr>
<td>14.2 Boring</td>
<td>146</td>
</tr>
<tr>
<td>14.3 Disturbed samples</td>
<td>146</td>
</tr>
<tr>
<td>14.4 Undisturbed samples</td>
<td>147</td>
</tr>
<tr>
<td>14.5 Handing and labeling of samples</td>
<td>147</td>
</tr>
<tr>
<td>14.6 Standard penetration test</td>
<td>147</td>
</tr>
<tr>
<td>14.7 Dispatch of samples</td>
<td>147</td>
</tr>
<tr>
<td>14.8 Laboratory tests</td>
<td>147-149</td>
</tr>
<tr>
<td>General</td>
<td>147</td>
</tr>
<tr>
<td>Preparation of the test specimens</td>
<td>148</td>
</tr>
<tr>
<td>Unconfined compression test</td>
<td>148</td>
</tr>
<tr>
<td>Triaxial test</td>
<td>148</td>
</tr>
<tr>
<td>Consolidation test</td>
<td>148</td>
</tr>
<tr>
<td>Routine test</td>
<td>149</td>
</tr>
<tr>
<td>Report and records</td>
<td>149</td>
</tr>
<tr>
<td>14.9 Measurement</td>
<td>149</td>
</tr>
<tr>
<td>14.10 Payment</td>
<td>149</td>
</tr>
</tbody>
</table>
SECTION-14
SUB-SOIL BORING AND TESTING

14.1 General
Confirmatory Sub-Soil investigation shall be carried out at the actual locations of each foundation. The objective of the subsoil investigation is to ascertain the actual soil strata at the location, the engineering properties at each stratum and to ascertain the level at which the foundation can be laid. The investigation shall be carried out as per the following specifications and as directed by the Engineer.

14.2 Boring
Boring shall be carried out in accordance with the specifications of ASTM D 1586 and D 1587. The bore holes shall have a minimum diameter of 100mm and shall be lined throughout. Minimum depths shall be 20m unless otherwise directed by the Engineer. The toe of the lining shall at no time be more than 1m above the level to which the soil has been removed from the bore hole.

Before taking any undisturbed sample or making any in-situ test, the lining shall be carried down to the bottom of the bore hole at the test depth.

Auger of proper size shall be used in very soft to soft clays and silts to avoid suction. The use of shell shall only be restricted to moderately stiff to very stiff and hard clays and also in sandy strata below water table. The use of a chisel would be permitted only in case of boulder or rock formation or through local obstructions or other situation demanding its use as would be decided by the Engineer.

Uncased bore holes may be permitted only upto a depth where the sides of the hole can stand unsupported. In case of side fall or squeezing, steps shall be taken immediately to stabilize the sides of the bore hole by casing pipes as directed by the Engineer. Use of Bentonite slurry of 5% concentration may be permitted to stabilize the bore hole.

No water shall be added while boring through cohesive soils and non-cohesive soils above the water table. While boring through non-cohesive soil below water table, water level in the casing shall always be maintained at or above the water table.

The cutting brought up by the auger shell or the split-spoon or undisturbed sampler shall be carefully examined and the soil description duly recorded after performing field identification tests.

On completion of boring at any bore hole, a bore log shall be prepared in an approved standard format in consultation with the Engineer and submitted to the Engineer in triplicate. Position of the water table shall be observed after 24 hours and back filling of the bore hole shall be carried out with approved materials in a manner as directed by the Engineer.

14.3 Disturbed samples
Disturbed samples shall be taken from bore hole cuttings and split-spoon for visual classification tests at the Site. The samples shall be taken at 1.5m interval or at every identifiable change of strata, whichever is met earlier to give a reliable record of the variation in the conditions of the soils. Disturbed samples shall be sent to the laboratory in airtight plastic container with proper label for the purpose of record and laboratory testing.
14.4 **Undisturbed samples**

Collection of undisturbed samples from cohesive soil layers shall be conducted as per ASTM D 1587 and/or any other equivalent.

14.5 **Handing and labeling of samples**

The following conditions of handling and protection of undisturbed samples shall be undertaken on undisturbed sample.

- Immediately after being taken from the bore hole, the ends of the sample shall be cut and removed to a depth of about 2.5cm (or more in the top to cover any obviously disturbed soil). Several layers of molten wax should then be applied to each end to give a plug about 2.5cm thick. If the sample is very porous, a layer of waxed paper should first be placed over the ends of the sample.

- Any space left between the end of the sample tube and the top of the wax should be tightly packed with saw dust or other suitable materials and a close fitting lid or screwed cap shall be placed on each end of the sample tube.

- The lids should, if necessary, be held in position by adhesive tape.

- A label bearing the number of the sample, bore hole number, depth of sample, date, etc. preferably typed, shall be placed inside the container just under the lid. It shall be placed at the top of the sample. In addition, the number of the sample shall be painted on the outside of the container and the top or bottom of the sample shall be indicated.

- Undisturbed soil sample tubes shall be placed in a strong wooden box and packed with moist saw dust, paper, etc. to prevent damage during dispatch to the laboratory.

14.6 **Standard penetration test**

Standard penetration test shall be conducted as per ASTM D 1586 at an interval of 1.5m or at every identifiable change of strata, whichever is earlier.

The driving of split-spoon shall be recorded for every 150mm penetration till the total penetration is 450mm.

Driving of the split-spoon shall be terminated when standard penetration resistance value, \( N > 100 \) blows / 30cm of penetration is received, unless otherwise directed by the Engineer. The test shall be conducted after driving the casing to the bottom of the bore hole and after cleaning it. \( N \)-values, as observed in the field, shall be reported in the bore logs without any correction.

14.7 **Dispatch of samples**

Samples shall be dispatched to the laboratory as soon as possible after being obtained and shall not be allowed to accumulate at Site. In the event a danger of sample’s deterioration through further storage is noticed, the Contractor shall dispatch such samples immediately on receiving direction from the Engineer.

14.8 **Laboratory tests**

**General**

Laboratory tests shall be carried out as per relevant ASTM or BS Procedures or by any other procedures approved under equivalent recognized standards. The results of all tests shall be submitted in the format as approved by the Engineer.
Preparation of the test specimens

Preparation of test specimens for the various tests shall be carried out as per the procedures laid down in the various relevant ASTM or BS Codes or by any other procedures approved under equivalent recognized standards.

In case of soft to firm cohesive undisturbed soil samples, test samples for all types of shear tests shall be prepared strictly by hand trimming on soil lathe. Care shall be taken against bending of soil samples at the time of horizontal ejection of the samples from the sampling tubes. Samples shall be ejected from the sampling tubes preferably in the same direction of travel in which the samples entered the sampling tubes.

Similarly test specimens for consolidation tests shall also be prepared to the required size by hand trimming only and the ring of the consolidation apparatus shall be inserted by pressing gently with the hands and carefully removing the material around the ring. In no case the ring should be forced into the soil. Great cares shall be taken during trimming of the sample from the top and the bottom of the ring. The test specimen shall be prepared in the same orientation as that to the actual strata so that the laboratory test load compresses the soil in the same direction relative to the soil strata as the applied load in the field.

Unconfined compression test

Unconfined compression test shall be conducted both on natural and remoulded soil samples. Remoulded soil specimen shall be prepared by the dynamic method of compaction.

Each unconfined compression test (natural or remoulded) shall comprise tests on minimum of three soil specimens, not less than 38mm diameter and a height to diameter ratio of 2 together with the determination of natural moisture content and density. Water content of the specimen shall be taken from the failure zone of the specimen. Test results shall be observed and reported as per the standard practice.

Triaxial test

Triaxial test shall be conducted on the undisturbed samples selected by the Engineer. Each test shall be conducted on a minimum of three specimens tested at different cell pressures (0.5 kg/cm², 1.0 kg/cm² and 1.5 kg/cm²). The moisture content before and after the test and the density shall be determined.

The stress-strain diagrams as well as the Mohr circle envelop for these tests shall be submitted.

Consolidation test

Consolidation tests shall be conducted on undisturbed samples selected by the Engineer. The coefficient of consolidation (Cv.), the coefficient of volume compressibility (Mv.), Laboratory Compression Index (Ccl.), Field Compression Index (Ccf.) including field virgin slope and the coefficient of permeability (k) shall be determined and results shall be submitted.

The loading on the test specimens shall be applied at the stages of 0.1 kg/cm², 0.25 kg/cm², 0.5 kg/cm², 1.0 kg/cm², 2.0 kg/cm², 4.0 kg/cm² and 8.0 kg/cm².

Unloading of the test specimens shall be done at suitable stages.
**Routine test**

All routine tests like natural moisture content, bulk density, liquid and plastic limits, grain size distribution, specific gravity, shall be conducted on selected representative samples as directed by the Engineer.

**Report and records**

On completion of each bore hole, three copies of a bore hole log shall be submitted to the Engineer together with one copy of the list of disturbed and undisturbed samples taken from the bore hole.

These bore logs shall show:

1. Ground level referred to the reduced level.
2. Locations of the bore holes on a plan.
3. Detailed description of each stratum.
4. Position, type and identification of each sample and SPT value.
5. Any other Site test results available.
6. Levels at which each separate ground water level is first encountered and at which it comes to rest (standing water level).

On completion of all field and laboratory tests, all results shall be submitted to the Engineer in 3 (three) copies in the form of reports with comments and views.

**14.9 Measurement**

The work will be measured for payment as an item on a lump sum basis or as specified in the Schedule of Items and BOQ.

**14.10 Payment**

Payment shall only be admissible on implementation of the item as measured and provided above and on being certified by the Engineer that the investigations have been carried out as per specifications as contained herein. Payment shall be made as lump sum rate or as specified in the Schedule of Items and BOQ, which shall cover the full costs of boring, collection and dispatch of samples, standard penetration test and all necessary Laboratory tests, preparation and submission of records, cost of all labour, equipment, materials, tools, test fees and all incidentals required for undertaking the test and submission of requisite reports to the Engineer in its totality. No payment shall be made until the testing results and other information in the form of reports with requisite number of copies are submitted to the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Sub-soil investigation</td>
<td>Lump sum</td>
</tr>
</tbody>
</table>
SECTION-15
FOUNDATION PILES
## Contents

### Section 15

<table>
<thead>
<tr>
<th>15.1</th>
<th>Bored cast in place piles</th>
<th>150-162</th>
</tr>
</thead>
<tbody>
<tr>
<td>15.1.1</td>
<td>Description</td>
<td>150</td>
</tr>
<tr>
<td>15.1.2</td>
<td>Accessories</td>
<td>150-151</td>
</tr>
<tr>
<td></td>
<td>Steel casing</td>
<td>150-151</td>
</tr>
<tr>
<td></td>
<td>Temporary steel casing</td>
<td>150</td>
</tr>
<tr>
<td></td>
<td>Permanent steel casing</td>
<td>150-151</td>
</tr>
<tr>
<td></td>
<td>Tremie pipe</td>
<td>151</td>
</tr>
<tr>
<td>15.1.3</td>
<td>Materials</td>
<td>151-152</td>
</tr>
<tr>
<td></td>
<td>Concrete</td>
<td>151</td>
</tr>
<tr>
<td></td>
<td>Reinforcement</td>
<td>151</td>
</tr>
<tr>
<td></td>
<td>Welding electrodes</td>
<td>151</td>
</tr>
<tr>
<td></td>
<td>Drilling fluid</td>
<td>151-152</td>
</tr>
<tr>
<td></td>
<td>Supply</td>
<td>152</td>
</tr>
<tr>
<td></td>
<td>Mixing</td>
<td>152</td>
</tr>
<tr>
<td></td>
<td>Tests</td>
<td>152</td>
</tr>
<tr>
<td>15.1.4</td>
<td>Construction methods</td>
<td>152-159</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>152-153</td>
</tr>
<tr>
<td></td>
<td>Preparation</td>
<td>152</td>
</tr>
<tr>
<td></td>
<td>Drilling</td>
<td>153</td>
</tr>
<tr>
<td></td>
<td>Pile cluster</td>
<td>153</td>
</tr>
<tr>
<td></td>
<td>Obstruction during drilling</td>
<td>153</td>
</tr>
<tr>
<td></td>
<td>Depth of hole</td>
<td>153</td>
</tr>
<tr>
<td></td>
<td>Assumed procedure</td>
<td>153</td>
</tr>
<tr>
<td></td>
<td>Approval of construction method</td>
<td>154</td>
</tr>
<tr>
<td></td>
<td>Setting out piles</td>
<td>154</td>
</tr>
<tr>
<td></td>
<td>Diameter of piles</td>
<td>154</td>
</tr>
<tr>
<td></td>
<td>Tolerances</td>
<td>154</td>
</tr>
<tr>
<td></td>
<td>Boring</td>
<td>155</td>
</tr>
<tr>
<td></td>
<td>Methods</td>
<td>155</td>
</tr>
<tr>
<td></td>
<td>Boring near recently cast piles</td>
<td>155</td>
</tr>
<tr>
<td></td>
<td>Steel casings</td>
<td>155</td>
</tr>
<tr>
<td></td>
<td>Stability of pile excavation using drilling fluid</td>
<td>155</td>
</tr>
<tr>
<td></td>
<td>Disposal of excavated material</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Pumping from bore holes</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Obstructions</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Unexpected ground conditions</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Boring records</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Final pile toe level</td>
<td>156</td>
</tr>
<tr>
<td></td>
<td>Inspection and cleaning of bottom of excavation</td>
<td>156-157</td>
</tr>
<tr>
<td></td>
<td>Placing reinforcement</td>
<td>157</td>
</tr>
<tr>
<td></td>
<td>Placing concrete</td>
<td>157-159</td>
</tr>
<tr>
<td></td>
<td>Approval</td>
<td>157</td>
</tr>
<tr>
<td></td>
<td>Methods</td>
<td>157-158</td>
</tr>
<tr>
<td></td>
<td>Workability of concrete</td>
<td>158</td>
</tr>
<tr>
<td></td>
<td>Placing concrete under water or drilling fluid</td>
<td>158-159</td>
</tr>
<tr>
<td>Section</td>
<td>Page</td>
<td></td>
</tr>
<tr>
<td>------------------------------------------------------------------------</td>
<td>------</td>
<td></td>
</tr>
<tr>
<td>15.1.5 Extraction of temporary casing</td>
<td>159-160</td>
<td></td>
</tr>
<tr>
<td>Workability of concrete</td>
<td>159</td>
<td></td>
</tr>
<tr>
<td>Concrete level</td>
<td>159-160</td>
<td></td>
</tr>
<tr>
<td>Vibrating extractors</td>
<td>160</td>
<td></td>
</tr>
<tr>
<td>Reinforcement cage</td>
<td>160</td>
<td></td>
</tr>
<tr>
<td>Supervision</td>
<td>160</td>
<td></td>
</tr>
<tr>
<td>15.1.6 Temporary support</td>
<td>160</td>
<td></td>
</tr>
<tr>
<td>15.1.7 Records</td>
<td>160</td>
<td></td>
</tr>
<tr>
<td>15.1.8 Measures in case of rejected piles</td>
<td>161</td>
<td></td>
</tr>
<tr>
<td>15.1.9 Other requirements</td>
<td>161</td>
<td></td>
</tr>
<tr>
<td>15.1.10 Measurement</td>
<td>161</td>
<td></td>
</tr>
<tr>
<td>15.1.11 Payment</td>
<td>161-162</td>
<td></td>
</tr>
<tr>
<td>15.2 Pre-cast Reinforced Concrete Piles</td>
<td>163-172</td>
<td></td>
</tr>
<tr>
<td>15.2.1 Description</td>
<td>163</td>
<td></td>
</tr>
<tr>
<td>General</td>
<td>163</td>
<td></td>
</tr>
<tr>
<td>15.2.2 Materials</td>
<td>163</td>
<td></td>
</tr>
<tr>
<td>Concrete</td>
<td>163</td>
<td></td>
</tr>
<tr>
<td>Reinforcement</td>
<td>163</td>
<td></td>
</tr>
<tr>
<td>15.2.3 Formwork</td>
<td>163-164</td>
<td></td>
</tr>
<tr>
<td>Pile shoes/helmet</td>
<td>164</td>
<td></td>
</tr>
<tr>
<td>15.2.4 Production of pre-cast reinforced concrete piles</td>
<td>164-165</td>
<td></td>
</tr>
<tr>
<td>Length of piles</td>
<td>164</td>
<td></td>
</tr>
<tr>
<td>Pile dimensions</td>
<td>164</td>
<td></td>
</tr>
<tr>
<td>Casting of piles</td>
<td>164-165</td>
<td></td>
</tr>
<tr>
<td>Casting tolerances</td>
<td>165</td>
<td></td>
</tr>
<tr>
<td>Curing and removal of formwork</td>
<td>165</td>
<td></td>
</tr>
<tr>
<td>15.2.5 Marking of piles</td>
<td>165</td>
<td></td>
</tr>
<tr>
<td>15.2.6 Protection of finished piles</td>
<td>165</td>
<td></td>
</tr>
<tr>
<td>15.2.7 Handling and storage of piles</td>
<td>165-166</td>
<td></td>
</tr>
<tr>
<td>15.2.8 Pilot (Test) piles</td>
<td>166</td>
<td></td>
</tr>
<tr>
<td>15.2.9 Driving of piles</td>
<td>166-170</td>
<td></td>
</tr>
<tr>
<td>Driving equipment</td>
<td>167</td>
<td></td>
</tr>
<tr>
<td>Driving appurtenances</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Hammer cushion</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Pile drive head</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Pile cushion</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Leads</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Followers</td>
<td>168</td>
<td></td>
</tr>
<tr>
<td>Driving procedures</td>
<td>168-169</td>
<td></td>
</tr>
<tr>
<td>Jets</td>
<td>169</td>
<td></td>
</tr>
<tr>
<td>Driving records</td>
<td>170</td>
<td></td>
</tr>
<tr>
<td>15.2.10 Repair of pile heads</td>
<td>170</td>
<td></td>
</tr>
<tr>
<td>15.2.11 Extension of piles</td>
<td>170</td>
<td></td>
</tr>
<tr>
<td>15.2.12 Defective piles</td>
<td>170-171</td>
<td></td>
</tr>
<tr>
<td>15.2.13 Cutting off pile heads and bonding</td>
<td>171</td>
<td></td>
</tr>
<tr>
<td>15.2.14 Measurement</td>
<td>171</td>
<td></td>
</tr>
<tr>
<td>15.2.15 Payment</td>
<td>171-172</td>
<td></td>
</tr>
<tr>
<td>Section</td>
<td>Description</td>
<td>Pages</td>
</tr>
<tr>
<td>-----------</td>
<td>--------------------------------------</td>
<td>---------</td>
</tr>
<tr>
<td>15.3</td>
<td>Sand compaction piles</td>
<td>173-174</td>
</tr>
<tr>
<td>15.3.1</td>
<td>General</td>
<td>173</td>
</tr>
<tr>
<td>15.3.2</td>
<td>Construction process</td>
<td>173</td>
</tr>
<tr>
<td>15.3.3</td>
<td>Measurement</td>
<td>173</td>
</tr>
<tr>
<td>15.3.4</td>
<td>Payment</td>
<td>174</td>
</tr>
<tr>
<td>15.4</td>
<td>Timber piles</td>
<td>175-177</td>
</tr>
<tr>
<td>15.4.1</td>
<td>Description</td>
<td>175</td>
</tr>
<tr>
<td>15.4.2</td>
<td>Materials</td>
<td>175</td>
</tr>
<tr>
<td>15.4.3</td>
<td>Preservatives and treatments</td>
<td>175</td>
</tr>
<tr>
<td>15.4.4</td>
<td>Handling and storage of piles</td>
<td>175</td>
</tr>
<tr>
<td>15.4.5</td>
<td>Pile driving</td>
<td>175-177</td>
</tr>
<tr>
<td>15.4.6</td>
<td>Pile cut-off</td>
<td>177</td>
</tr>
<tr>
<td>15.4.7</td>
<td>Measurement</td>
<td>177</td>
</tr>
<tr>
<td>15.4.8</td>
<td>Payment</td>
<td>177</td>
</tr>
</tbody>
</table>
SECTION-15
FOUNDATION PILES

15.1 Bored cast in place piles

15.1.1 Description

This work shall comprise of boring and construction of bored cast in place piles for foundation of the building structures. The item includes the provisions of all labour, materials, equipment, boring and all incidentals necessary to complete the work in accordance with these Specifications, in conformity with the requirements of the Drawings, as required in the other places of the Contract Document and/or as per direction of the Engineer.

Piles through the water and soft upper soil layers shall be provided with permanent steel casing, if shown on the Drawings.

The pile boring shall be carried out using a temporary steel casing driven up to the pile toe or to a level approved by the Engineer. The temporary casing shall be withdrawn.

Under certain circumstances the Contractor may be permitted to bore all or part of the pile without casing under water or using drilling fluid to stabilize the borehole.

Concrete and reinforcement of the piles shall be strong enough to resist pile loads and horizontal forces on the pile caps.

15.1.2 Accessories

Steel casing

Temporary steel casing

Temporary steel casing pipe of required diameter shall be used at least for the upper 6m from the ground level during drilling to stabilize the hole. The casing pipe shall be fabricated to the specified size and shape from mild steel. It shall be smooth, clean, water tight and sufficiently strong to withstand both handling and driving stresses and the pressure of both concrete and the surrounding earth materials. It shall be free from significant distortion and shall have uniform cross-section throughout each continuous length. Size of the shaft shall be less than the inside diameter of the casing. However, it shall not be less than 10mm. During pouring of concrete, it shall be free from internal projections and encrusted concrete, which might prevent the proper formation of piles.

Permanent steel casing

The steel shall conform to the ASTM A 36 or approved equivalent having sufficient strength and rigidity to prevent distortion by soil pressure or for drilling of adjacent piles.

Minimum wall thickness of the permanent steel casing shall be 6mm. Minimum length shall be from 100mm above the bottom of the pile cap to 5m inside the ground or into firm strata or as shown on the Drawings or as directed by the Engineer. In the case the permanent casing is used in the boring operation and if the handling and transport require a greater thickness to avoid deformation or buckling, the increase in thickness shall be provided by the Contractor at his own expenses.
The steel casing shall be furnished in appropriate length and the joints shall be approved by the Engineer.

Casing pipes may be transported to the Site at suitable lengths in pieces and shall be welded as per specifications to fabricate the designed length. The casing shall be handled and stored in a manner that shall prevent buckling and other deformation as well as accumulation of dirt, oil and paint. When placed in the work, it shall be free from dirt, oil, grease, paint, mill scale and loose or thick rust or any deleterious substance that may affect the concrete.

**Tremie pipe**

A Tremie shall consist of a steel tube having a diameter of not less than 150mm, sufficiently long to reach the bed of water keeping its one end above the water level, constructed in sections having flanged couplings fitted with gaskets. The tube shall be fitted with a hopper at its upper end for pouring concrete inside the tube. The Tremie shall be supported so as to permit free movement of the discharge end over the entire top surface of the work so as to permit rapid lowering when necessary to retard or stop the flow of concrete.

### 15.1.3 Materials

**Concrete**

The concrete for bored cast-in-place piles shall conform to all requirements as described under the Section on ‘Concrete Work’ of this Specification. Cement Type 1 shall be used and the characteristic cylinder strength shall be 250 kg/cm² at 28 days.

Concrete placed under water or drilling mud by Tremie shall have cement content of not less than 370 kg/m³.

The density and consistency of the concrete shall conform to the Tremie Casting Method. Sufficient workability (slump) of all concrete shall be maintained during the casting and casing handling period. Reasonable calculated delays shall be secured by a design mix (including the necessary retarders and plasticisers) which is tested by trial mixes prior to the pile construction.

All relevant concrete properties such as slump, time of setting, temperature and strength shall be measured on the trial mixes.

**Reinforcement**

Reinforcement bar and binding wires used in the construction of bored cast-in-place piles shall conform to the requirements stated under the Sub-section on ‘Reinforcement for RCC’ of this Specification.

**Welding electrodes**

Where welding is specified for fabrication of the reinforcement, the electrodes shall conform to the American Welding Society (AWS) Standards and shall be of the size and classification number recommended by the manufacturer.

**Drilling fluid**

The following instructions shall be complied with, if Bentonite mud is used to stabilize the bore hole.
Supply

Bentonite, if required and supplied at the Site, shall be in accordance with the specifications conforming to DFCP 4 of the Oil Companies Materials Associations.

The Contractor shall obtain a certificate from the manufacturer of the Bentonite powder showing the properties of the consignment delivered at the Site. This certificate shall be made available to the Engineer on request. The properties to be given by the manufacturer are the apparent viscosity range and the gel strength range for solids in water.

Any other materials for the drilling fluid shall receive approval from the Engineer.

Mixing

Bentonite and any other materials shall be mixed thoroughly with clean water to make a suspension, which shall maintain the stability of the pile excavation for a period necessary to pour concrete and complete construction. Where saline or chemically contaminated groundwater occurs, special precautions shall be taken to modify the Bentonite in fresh water so as to render it suitable in all respect for the construction of piles.

Tests

The frequency of testing drilling fluid and the methods and procedures of sampling shall be proposed by the Contractor and approved by the Engineer prior to the commencement of the work. The frequency may subsequently be varied as required depending upon the consistency of the results obtained. The control tests shall cover the determination of density, viscosity, gel strength and pH values.

For average soil conditions, the results shall generally be within the ranges stated in the Table given below. The tests shall be carried out until a consistent working pattern has been established.

<table>
<thead>
<tr>
<th>Property to be measured</th>
<th>Range of Results at 20°C</th>
<th>Test Method</th>
</tr>
</thead>
<tbody>
<tr>
<td>Density</td>
<td>1.03 – 1.1 g/ml</td>
<td>Mud density balance</td>
</tr>
<tr>
<td>Viscosity</td>
<td>30-90s or less than 20 cP</td>
<td>Marsh cone method Fann viscometer</td>
</tr>
<tr>
<td>Shear strength (10 minute gel strength)</td>
<td>1.4 - 10.0 N/m² or 4.0 - 40.0 N/ m²</td>
<td>Shearometer Fann Viscometer</td>
</tr>
<tr>
<td>pH value</td>
<td>9.5 - 12.0</td>
<td>pH indicator paper strips or electrical pH meter</td>
</tr>
</tbody>
</table>

The Contractor shall supply all equipment and engage experienced operators required for carrying out tests on the drilling mud. No additional payment shall be made for these tests, which shall be considered as an essential part of the drilling operations.

15.1.4 Construction method

General

Preparation

Before starting drilling operation, the Contractor shall plan the sequence and stages of operation for different piles and establish levels, grades and alignment of all piles with reference to Bench Mark (BM) previously established at Site. The Contractor shall have fabricated all casing pipes and reinforcing bars as per design and shall be ready for lowering the pipes on completion of drilling. All necessary equipment such as pump, welding set, etc. and materials for concrete work including Tremie pipe shall be made available before the start of drilling operation.
Drilling

The Engineer shall approve the drilling method and the equipment to be used for this purpose. The Contractor shall prepare all suitable cofferdam/artificial island/staging or any other approved means, if required, for the drilling operation and pouring concrete of the piles in water. Bentonite slurry, if required, shall be used to stabilize the hole.

Pile cluster

Where there are more than 4 (four) piles in a cluster, the centre pile shall be installed first. All piles in a cluster shall be of the same depth.

Obstruction during drilling

When obstructions make it extremely difficult to drill certain holes in the location shown and upto the proper bearing strata, the Contractor shall take all usual methods to install piles as required including jetting, cutting, drilling or other feasible means. If in the judgment of the Engineer the Contractor is unable to complete properly any pile by resorting to such methods, the Engineer may order for an additional hole drilled at another selected location at the Contractor’s own expenses.

Depth of hole

The Engineer shall check the depth of hole by lowering suitable drop in order to determine the length of pile. Immediately after approval of the bore, the permanent steel casing pipe shall be installed upto the design depth, if provided in the BOQ and then the reinforcement cage shall be lowered.

Pile type and construction methods shall ascertain that the pile shaft shall not be weakened by contamination of the concrete, by sectional reduction, by washing out of cement, by breaking during pulling of temporary casings or by any other way including construction of neighbouring piles.

Assumed Procedure

The following construction procedures shall be assumed in the Design. The Engineer shall approve the final construction procedures or any subsequent modification prior to commencing piling operations.

- If required, placing the permanent steel casing in position and embedding the casing toe into the firm strata. If no permanent steel casing is specified, a sufficient length of temporary steel casing shall be used to stabilize the upper part of the bore hole.
- Boring and excavating the inside of the steel casing down to the casing toe level or to a level approved. Excavating upto the final pile tip level using either temporary casing under water or using drilling mud. Water level inside the casings shall, at all times, be at least 2m higher than the outside of the casings.
- Cleaning carefully all mud or sediments from the bottom of the bore hole.
- Placing reinforcement cage, inspecting pipes, etc.
- Pouring concrete continuously under water or drilling fluid following the Tremie method.
- Withdrawing the temporary casing concurrently with pouring of concrete upto the instructed level.
- Breaking the top section of the concrete pile after hardening in order to reach sound concrete.
Approval of construction method

In the Tender, the Contractor shall describe his proposed construction methods, which shall include information on boring equipment, materials, methods of work, quality control and bearing capacity and also the name of the Sub-contractor (as and when allowed). The Contractor shall submit references from similar jobs carried out by him or by the Sub-contractor.

Prior to making Contract, the Contractor shall submit all requested supplementary information in writing.

After the Contract has been awarded to the Contractor, he shall prepare a detailed programme and establish a procedure for the pile construction in accordance with the above information.

The detailed programme shall contain all information as requested on materials, equipment, methods of work, etc. and be approved in writing by the Engineer. Such approval shall not relieve the Contractor of his full responsibilities for the entire pile construction.

No boring equipment or material shall be imported at Site before the Contractor has received the approval of the Engineer as stated above.

Setting out piles

Before starting drilling operation the Contractor shall plan the sequence and stages of operation for different piles and establish levels, grades and alignment of all piles with reference to Bench Mark, previously established at Site. The Contractor shall have all casing pipes and reinforcing bars fabricated as per design and ready for lowering on completion of drilling. All necessary equipment and materials for concrete work including Tremie pipe shall be made available before the start of drilling operation.

The positions of the piles shall be set out in accordance with the Drawings from established Bench Mark. The position of each pile shall be approved by the Engineer before drilling commences.

Where there are more than four piles within a cluster, the center pile shall be constructed first. No concrete shall be placed until all drilling within a radius of 2.5m has been completed. If this is not possible, no drilling shall be done within 2.5m radius of a cast-in-situ pile until the concrete has set for at least 96 hours after pouring.

Diameter of piles

The diameter of a pile shall be not less than the specified diameter.

Tolerances

Bores shall be accurately drilled in the locations as shown on the Drawings. All piles shall be drilled with a lateral tolerance of not more than 75mm from the point specified. Pile that deviates by more than 75mm in lateral location or pile whose slope deviate from the vertical by more than 2%, shall be rejected. Additional piles shall then be furnished and installed by the Contractor in such locations as the Engineer may direct. The Contractor shall provide suitable equipment, such as an inverted pendulum, to check the verticality of the bore holes at intervals during drilling and prior to pouring concrete. All costs for such additional piles as required to suit the changed pile locations, shall be borne by the Contractor at his own costs.
**Boring Method**

Generally two methods are followed while excavation. One is Percussion Drilling Method and the other is Rotary Drilling Method. However, method of excavation shall be proposed by the Contractor and approved by the Engineer. Water or air jetting for boring of the piles shall not be allowed.

**Boring near recently cast piles**

Piles shall not be bored so close to other piles which have recently been cast and which contain workable or unset concrete so that flow of concrete could be induced from or damage caused to any of the piles. Boring and excavation for a pile shall not be commenced until 96 hours after completion of any pile within a radius of 2.5m center to center.

**Steel casing**

A temporary steel casing pipe of approved quality and specifications stated earlier shall be used and lowered simultaneously with the progress of drilling for the purpose of stabilizing at least the top 6m of the hole. Where a permanent steel casing pipe is specified in the Drawings, this shall either be lowered as drilling progresses instead of the temporary pipe, or installed immediately on completion of drilling. The inside of the casing pipe shall be cleaned of oil, grease, paint and other deleterious substances before lowering.

A pile constructed in a stable cohesive soil without the use of temporary casing or other form of support shall be bored and concreted without prolonged delay and in any case soon enough to ensure that the soil characteristics are not significantly impaired.

**Stability of pile excavation using drilling fluid**

Where a bore hole is formed without casing under water or using drilling fluid for maintaining the stability of a boring, the level of the water or fluid in the excavation shall be maintained so that the water fluid pressure always exceeds the pressure exerted by the soils and external ground water. The water or fluid level shall be maintained at a level not less than 2m above the level of the outside water level or any artesian pressure level.

Drilling mud shall be used at least from the level of sub-soil water or from the level of the bottom of the guide casing depending upon the Site conditions and the hole shall then always be kept almost full with fluid, which should preferably be kept in motion. The density and composition of the fluid shall be such as to suit the requirements of the ground condition and to maintain the fine materials from the boring in suspension. A five percent Bentonite suspension would be generally suitable.

Where saline or chemically contaminated ground water occurs, special precautions shall be taken to modify the Bentonite suspension or pre-hydrate the Bentonite in fresh water so as to render it suitable in all respect for construction of the piles.

In the event of a rapid loss of water or Bentonite suspension from the pile excavation, the excavation shall be back-filled without any delay and the instructions of the Engineer shall be obtained before excavation at the location is resumed.
Disposal of excavated material

No excavated material shall be dumped into the river or any connecting waterway without the written approval of the Engineer. Excavated materials shall be removed from the Site and dumped either beyond areas affected by dredging, or taken to the Contractors dumping areas on land. The Contractor shall be fully responsible for all costs involved in removing the excavated materials to spoil.

Pumping from bore holes

Pumping from a bore hole shall not be permitted unless a casing has been placed into a stable stratum, which prevents the flow of water from other strata in significant quantities into the boring, or unless it can be shown that pumping will not have a detrimental effect on the surrounding soils and/or properties.

Obstructions

Where boulders or other obstructions render it impossible to bore the pile, excavation operations inside the pile casing, as directed by the Engineer, shall be carried out to remove the obstructions. The Contractor shall be reimbursed for such operations only when the largest dimension of the obstruction exceeds 200mm and the obstruction is found more than 2m below the ground level or water bed. However, the amount of compensation shall be proposed by the Contractor and agreed by the Engineer beforehand the removal works start.

Unexpected ground conditions

The Contractor shall report immediately to the Engineer any circumstances, which indicates that in the Contractor’s opinion the ground conditions differ from those expected by him from his interpretation of the Site Investigation Reports.

Boring records

During the boring of the pile, the Contractor shall compile a boring log indicating depths and types of the various soil layers encountered. Disturbed samples shall be submitted to the Engineer, as per requests.

The Contractor shall carry out sampling and tests to check soil strengths and shall not be reimbursed for this work.

Final pile toe level

The final pile toe level shall be as indicated on the Drawing or as instructed by the Engineer after due consideration of the Contractor’s proposals, boring logs and test results.

The final toe level of other piles may subsequently be altered according to the results of the test loading detailed under Section on ‘Pile Load Testing’ of this Specifications.

Inspection and cleaning of bottom of excavation

The time between final excavation and bottom cleaning and the start of pouring concrete shall be reduced as much as possible and shall not exceed six hours. To achieve this, the final 2m of excavation shall not start until all preparations for cleaning, reinforcing and pouring concrete are finished. In case of unexpected delay, the Contractor shall dump sand or gravel in the bore upto 2m above the toe level.

Immediately after excavation, the bottom of the excavation shall be carefully cleaned for mud and sediments and other soft materials. A short interruption is recommended to allow the fine materials to settle.
The cleaning shall be made by an approved method. Before cleaning of every pile, notice shall be given to the Engineer.

The Contractor shall carry out Sedimentation Tests in presence of the Engineer.

For boring without casing, the diameter of the bore hole for a representative number of piles shall be measured by a Caliper prior to the pouring of concrete. The verticality of the bore holes shall be maintained by the Contractor using approved equipment and no reimbursement shall be made in this regard.

**Placing reinforcement**

The reinforcing steel cage consisting of the steel shown on the Drawings along with cage stiffener bars, spacers, centralizers, and other necessary appurtenance shall be completely assembled and placed as one unit immediately after the excavation is inspected and accepted and prior to concrete placement.

The reinforcement shall be placed as indicated on the Drawings. Reinforcement in the form of a cage shall be assembled with additional support, such as spreader forks and laciness, necessary to form one rigid cage. Hoops, links or helical reinforcement shall fit closely around the main longitudinal bars and be bound by approved wire, the ends of which shall be turned into the interior of the pile or pour. Hoops, links or helical reinforcement may also be placed and fitted with main longitudinal bars by staggered spot or line welding of approved quality.

The reinforcing steel shall be tied and supported so that it will remain within allowable tolerances until the concrete will support the reinforcing steel.

The cover to all reinforcement shall be not less than 75mm.

Joints in longitudinal steel bars shall be permitted unless otherwise specified. Joints in reinforcement shall be such that the full strength of the bar is effective across the joint and shall be made so that there is no relative displacement of the reinforcement during construction of the pile.

Joints in longitudinal steel bars in piles with tension (for instance for test loading) shall be carried out by welding unless another method has been approved by the Engineer.

In case the final pile toe level, instructed by the Engineer, is deeper than that indicated on the Drawings, the section of the pile, deeper than the toe level indicated on the Drawings, will not require any reinforcement.

**Placing concrete**

- **Approval**

  No pouring of concrete shall take place before the bottom of the excavation has been cleaned, the bore hole inspected and approval has been obtained in writing from the Engineer.

- **Methods**

  The method for mixing the concrete shall be as specified under the relevant Sub-section of the Section on ‘Concrete Work’ of this Specification. The concrete shall be placed using a Tremie pipe long enough to reach the bottom of the hole and having an internal diameter of not less than 150mm. The Tremie pipe shall be gradually withdrawn as the pouring of the concrete progresses but shall always be kept below the surface of the poured concrete.
The method of placing and the workability of the concrete shall be such that a continuous monolithic concrete pile of the full cross-section is formed.

The concrete shall be placed continuously and without such interruption as would allow the previously placed batch to have hardened. In this respect the Contractor shall submit details of his contingency plans, standby plant, etc. to be utilized in the event of any equipment breakdown.

The use of pumped concrete and the methods in its use shall be approved by the Engineer.

The Contractor shall take all precautions in the design of the mix and placing of the concrete to avoid arching of the concrete in a casing. No spoil, liquid or other foreign matters shall be allowed to contaminate the concrete.

- **Workability of concrete**
  - Slump measured at the time of discharge into the pile boring shall be minimum 100mm and maximum 150mm.

- **Placing concrete under water or drilling fluid**
  - Concrete to be placed under water or drilling fluid shall be placed by Tremie and shall not be discharged freely into the water or drilling fluid.

  The internal diameter of the pipe of the Tremie shall be not less than 150mm for concrete made with 25mm aggregate and not less than 200mm for concrete made with 32mm aggregate. It shall be so designed that external projections are minimized allowing the Tremie to pass through reinforcing cages without causing damage. The internal face of the pipe of the Tremie shall be free from projections.

  Before placing concrete, all measures shall be taken to ensure that there is no accumulation of silt or other materials at the base of the boring and the Contractor shall ensure that heavily contaminated Bentonite suspension that could impair the free flow of concrete from the pipe of the Tremie, has not accumulated at the bottom of the hole.

  A sample of the Bentonite suspension shall be taken from the base of the boring by using an approved sampling device. If the specific gravity of the suspension exceeds 1.25, pouring of concrete shall not proceed. In this event, the Contractor shall modify the mud quality.

  The concrete shall be a rich coherent mix of high workability as shown on the Drawing, in accordance with the provisions stated under the relevant Sub-section on ‘Concrete Work’ as stated in the BOQ and/or as directed by the Engineer.

  The concrete shall be placed in such a manner that segregation does not occur.

  During and after pouring concrete, all cares shall be taken to avoid damage to the concrete from pumping and de-watering operations.

  The hopper and pipe of the Tremie shall be clean and watertight throughout. The pipe shall be sufficiently long to reach the base of the boring and a sliding plug or barrier shall be placed in the pipe to prevent direct contact between the first charge of concrete in the pipe of the Tremie and the water or drilling fluid. The discharge end shall be sealed closed at the start of work so as to prevent water from entering the tube before the tube is filled with concrete. After placement of concrete has started the Tremie pipe shall be kept full of concrete up to the bottom of the hopper. The pipe shall,
at all times, penetrate the concrete, which has previously been placed and shall not be withdrawn from the concrete until the concrete pouring is completed. The bottom of the Tremie pipe shall be kept at least 1.5m under the surface of the concrete. At all times a sufficient quantity of concrete shall be maintained within the pipe to ensure that the pressure from it exceeds that from the water or drilling fluid. If water enters the tube after placement of concrete has started, the Tremie shall be withdrawn, the discharge end resealed and the placement restarted. When a batch is dumped into the hopper, the flow of concrete shall be induced by slightly raising the discharge end, always keeping it in the deposited concrete. The flow shall be continuous until the work is completed.

The Contractor shall maintain a continuous record of the volume of the concrete used and the level of the concrete in the pile. Any deviation from the theoretical or expected volume/level relationship shall immediately be reported to the Engineer.

- Placing concrete in dry

When the top of the pile elevation is above the ground, portion of the pile above the ground shall be formed with a removable form or permanent casing when specified.

The concrete shall be vibrated or rodded to a depth of 1.5m below the ground surface except where soft uncased soil or slurry remaining in the excavation will possibly mix with the concrete.

After placement, the temporarily exposed surfaces of the shaft concrete shall be cured in accordance with the provisions of curing of concrete described under the Section ‘Concrete Work’ of this Specification.

For at least forty-eight hours after concrete has been placed, no construction operation other than mild vibration shall be conducted that would cause soil movement adjacent to the shaft.

Portions of the pile exposed to a body of water shall be protected from the action of water by keeping the forms in place for a minimum of seven days after concrete placement.

15.1.5 Extraction of temporary casing

Workability of concrete

Temporary casing shall be extracted while the concrete within it remains sufficiently workable to ensure that the concrete is not lifted.

Concrete level

When the casing is being extracted, a sufficient quantity of concrete shall be maintained within it to ensure that pressure from external water, drilling fluid or soil is exceeded and that the pile is neither reduced in section nor contaminated. The toe of the temporary casing shall be kept minimum 2m under the outlet of the Tremie.

No concrete shall be placed in the boring once the bottom of the casing has been lifted above the top of the concrete. It shall be placed continuously as the casing is extracted until the desired head of concrete is obtained.

Adequate precautions shall be taken in all cases where excess head of water or drilling fluid could be caused as the casing is withdrawn because of the displacement of water or fluid by the concrete as it flows into its final position against the walls of the shaft.
The pile shall be concreted at least one pile diameter above the designed cut off level to allow for chiseling off the top concrete down to sound hard concrete.

The pile top shall, after clean cutting, be embedded 75mm in the foundation.

**Vibrating extractors**

The use of vibrating casing extractors shall be permitted.

**Reinforcement cage**

When concrete is placed by Tremie method, temporary hold-down devices shall be used to prevent uplifting of the steel cage during concrete placement. Concrete spacers or other approved non-corrosive spacing devices shall be used at sufficient intervals not exceeding 1.5m along the drilled depth to insure concentric location of the cage within the boring. When the size of the longitudinal reinforcing steel exceeds 25mm, such spacing shall not exceed 3m.

**Supervision**

The execution of the pouring of concrete in the pile shall be supervised by a qualified person of the Contractor’s staff in addition to the Engineer’s representative, who will keep records on the relation between quantity of concrete used, level of concrete and withdrawal of casing.

**15.1.6 Temporary support**

The Contractor shall ensure that free standing piles are temporarily braced or stayed immediately after driving to prevent loosening of the piles in the ground and to ensure that no damage resulting from oscillation, vibration or movement of any free-standing pile length can occur.

**15.1.7 Records**

The Contractor shall keep records as indicated below for the installation of each pile and shall submit two signed copies of these records to the Engineer no later than noon of the next working day after the pile has been installed. The signed records shall form a record of the work.

- Contract
- Pile reference number (location)
- Pile type
- Nominal cross-sectional dimensions or diameter
- Date and time of boring
- Date and time of pouring of concrete
- Level of the ground at commencement of installation of pile
- Working level
- Pile toe level
- Pile head level
- Length of temporary casing
- Length of permanent casing
- Soils samples taken and in-situ test carried out
- Standing water level
- Length and details of reinforcement
- Concrete mix
- Volume of concrete supplied to pile and corresponding levels of concrete and casings
- All information regarding obstructions, delays and other interruptions to the sequence of work
15.1.8 **Measures in case of rejected piles**

If any pile is found unsatisfactory in the opinion of the Engineer for utilization in the structure, it shall be cut off below the pile cap when so ordered by him.

The pile shall be replaced as directed by the Engineer. All additional expenses shall be borne by the Contractor, which would not be reimbursable.

When the safe bearing value of any pile is found by tests to be less than the design load, longer pile or additional piles shall be installed as ordered in writing by the Engineer.

15.1.9 **Other requirements**

Reinforcement cages of piles selected by the Engineer shall be fitted at the Contractors expenses with watertight 50mm diameter G.I pipe from datum level down to pile toe level. Bottom of the pipes shall be closed waterproof. Piles with diameter above 800mm shall have four G.I pipes while piles with diameter below or equal to 800mm shall have three G.I pipes.

The Contractor shall test piles by an electronic ultrasonic device as per the instruction of the Engineer. The Contractor shall submit a report in two samples within two days of testing. Ultrasonic testing will not be paid separately and the Contractor shall make provisions that piles shall be tested in several lots as required by the work.

The Contractor shall cut off pile heads carefully but shall not cut, bend or damage starter bars. If required, the Engineer may instruct the Contractor to cut starter bars to the top level indicated on the Drawings.

15.1.10 **Measurement**

The unit of measurement shall be the linear meter in case of boring. The payable length shall be measured from the bottom of the pile cap upto the toe level of each pile.

The unit of measurement for concrete shall be in cubic meter. The payable length of the satisfactory bored piles shall be measured from the toe level to pile to including the broken portion.

The mass measured for injection of grout material shall not include the mass of water.

Breaking of pile heads shall be measured in cubic meter.

Permanent casing shall be measured in linear meter for each size.

The above measurements shall be taken for payment only on completion of works in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as accepted by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

15.1.11 **Payment**

The amount of completed and accepted boring works, as measured above, shall be paid at the Contract unit price per linear meter. The payment shall constitute the full compensation for furnishing all related accessories and equipment related to boring with all support arrangements like Rigs, Crane, Jets, Frames, Leads etc. including temporary casing, drilling and removal of obstruction in course of drilling, drilling fluid circulation, disposal of excavated materials, all labour and incidentals necessary to complete
the Work as per requirements described under this item of work, the Bill of Quantities, as shown on the Drawings and/or as directed by the Engineer.

The amount of completed and accepted work, as measured above in cubic meter of concrete, shall be paid at the Contract unit price. The payment shall constitute the full compensation for all costs for furnishing all materials of concrete including setting out piles, placing of reinforcement cage and keeping it in proper position, placing concrete by Tremie casting method, testing of all construction materials all costs of labour, equipment and plants, inspection and control, all related tools and all incidentals necessary to complete the work as per requirements described under this item of work, the Bill of Quantities, as shown on the Drawings and/or as directed by the Engineer. The payment shall exclude the costs for reinforcement and the broken portion of pile head.

Breaking of Pile Heads shall include removal of the dismantled materials such as concrete to a safe distance including scrapping and removing concrete from steel/M.S. rods, preparation and making platform where necessary, leveling and dressing the Site and clearing as well, etc.

Permanent casing shall be paid for at the Contract unit price per linear meter for permanent casing. Such payment shall be the full compensation for furnishing and placing the casing above the costs attributable to the work paid for under associated pay items.

No payment shall be made for unauthorized, defective, unsound or unsatisfactorily piles or for any costs incurred by the Contractor for such piles.

<table>
<thead>
<tr>
<th>Item of Payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Bored cast in place piles (Percussion method) (diameter as stated in the Bill of Quantities/Drawings)</td>
<td></td>
</tr>
<tr>
<td>a) Boring</td>
<td>Linear Meter</td>
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<tr>
<td>b) Concrete</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Bored cast in place piles (Rotary Drilling method) (Diameter as stated in the Bill of Quantities/Drawings)</td>
<td></td>
</tr>
<tr>
<td>a) Boring</td>
<td>Linear Meter</td>
</tr>
<tr>
<td>b) Concrete</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Breaking of pile heads</td>
<td>Cubic Meter</td>
</tr>
<tr>
<td>Permanent casing</td>
<td>Linear Meter</td>
</tr>
</tbody>
</table>
15.2 Pre-cast reinforced concrete piles

15.2.1 Description

This work shall consist of the production and driving of pre-cast reinforced concrete piles in accordance with this Specification and of the types and dimensions designated on the Drawings or as directed by the Engineer.

General

Pre-cast piles shall be manufactured in a casting yard in accordance with the Drawings.

Concrete shall be placed in one continuous pour for each pile. Pouring of concrete shall begin at the head and be progressed to the driving end of the pile.

Each pile shall be indelibly marked with its sequential number and date of manufacture.

15.2.2 Materials

Concrete

The concrete for pre-cast piles shall conform to the requirements illustrated under the Section on ‘Concrete Work’ of this Specification with a minimum concrete strength (cylinder) of 250 - 300 kg/cm² at 28 days.

Reinforcement

Reinforcing bar and binding wire used in the production of pre-cast piles shall conform to the requirements illustrated under the relevant Sub-section of the Section on ‘Reinforcing Steel’ of this Specification.

The reinforcement shall be assembled before placing in the moulds and all hoops and links shall be of uniform length firmly wired into position. Ends of helical reinforcement shall be firmly secured. Diagonal fork spacers shall be of an approved pattern.

Joints in main longitudinal bars will be permitted only where, in the opinion of the Engineer, each bar cannot be supplied in one complete length. Where permitted, joints shall be provided at agreed centers, designed to develop the full strength of the bar across the joint provided with adequate links or stirrups and staggered in position from those of adjacent longitudinal bars, all to the acceptance of the Engineer.

The main longitudinal reinforcing bars in piles, not exceeding 12m in length, shall be in one continuous length unless otherwise specified. In piles exceeding 12m long, joints will be permitted in main longitudinal bars at 12m nominal intervals. Joints in adjacent bars shall be staggered at least 1m apart along the length of the pile.

Joints in reinforcement shall be such that the full strength of the bar is effective across the joint.

Welding of joints in main longitudinal bars will not be permitted unless agreed in writing by the Engineer.

Concrete cover shall be maintained at the joints.

15.2.3 Formwork

Formwork shall comply with the provisions under the item on ‘Formwork’ of the Sub-section ‘False Work and Forms’ of this Specification except as specified below.
When the sides of adjacent piles are used as formwork, an approved method shall be used to prevent adhesion between concrete surfaces.

Holes for toggle bolts shall be at right angles to the faces of the pile and lined with steel tubes or other approved materials. Holes for lifting, handling and pitching shall be formed in the positions and according to the details shown on the Drawings or otherwise approved by the Engineer and lined with steel tubes.

Details of all pile shoes shall be submitted to the Engineer for approval prior to fabrication or supply. All shoes shall be fitted to the reinforcement as shown on the Drawings.

Pile shoe/helmet

Where applicable, pile shoes shall be manufactured by an approved supplier and consist of cast iron, cast steel or fabricated steel as shown on the Drawings.

Cast iron shoes shall be formed from chill hardened iron grade 10 in accordance with BS 1452 “Specification for Grey iron castings”. Cast steel shoes shall be formed from steel to grade A of BS 3100 “Specification for steel castings for general engineering purposes”. Fabricated steel shoes shall be formed from steel to grade 43 A1 and steel straps and fastenings to Grade 43A of BS 4360 “Weldable structural steels”. Other equivalent established standards will also be applicable in this case.

Castings shall be free from sand, honeycomb, porosity, blowholes or other defects. For cast shoes, straps and fastenings shall be of mild steel or wrought iron, cast in to and running continuously through the base.

15.2.4 Production of pre-cast reinforced concrete piles

Length of piles

The pile lengths shown on the Drawings are based on Site investigations prior to driving of test piles. The lengths of the piles shall be finally determined and ordered by the Engineer after driving of pilot piles. Pilot piles shall be produced to the lengths shown on the Drawings.

Pile dimensions

Piles shall be cast to the cross-sectional dimensions shown on the Drawings. The cross-section on dimensions shall not be less than those specified and shall not exceed them by more than 6mm.

The head of each pile shall be square to the longitudinal axis. The edges of the head and of the pile for a distance of 30mm from the head, shall be chamfered 25mm x 25mm. Any face of a completed pile shall not deviate by more than 1/1000th of the length of the pile from the straight line connecting the centroids of the end faces.

Casting of piles

All pre-cast concrete piles shall be cast on the Site or at the Contractor's pile casting yard. In case where piles are manufactured off-Site, the Contractor shall ensure that adequate notices have been given to the Engineer and he has been provided with appropriate facilities for inspection of the manufacturing process.

Piles shall be cast in a horizontal position on an accurately leveled casting platform. The formwork shall conform to the requirements defined under the relevant clause of this Specification.
May be deleted because of repetition reinforcing bar shall be of the types and dimensions and shall be placed, as shown on the Drawings. The construction method for reinforcement shall conform in all respect to the requirements defined under the relevant Sub-section (Reinforcing Steel) of this Specification.

The formwork and reinforcement for each pile shall be inspected and approved by the Engineer before pouring of concrete commences.

Concrete shall be placed continuously and shall be compacted by mechanical vibration. Special cares shall be taken to produce a pile free from air pockets or honeycomb.

The forms shall be slightly overfilled, the surplus shall be scraped off and the top surface shall be finished to a uniform texture similar to that specified to be produced by the forms. The pile surfaces shall be true, smooth and even.

**Casting tolerances**

The cross sectional dimensions of piles shall not be less than those specified or shown on the Drawings and shall not exceed such dimensions by more than 6mm.

Unless otherwise directed by the Engineer, any face of a pile shall not deviate by more than 6mm from a straight edge 3m long laid on the face and the centroid of any cross section of the pile shall not deviate by more than 10mm from the straight line connecting the centroid of the end faces of the pile.

**Curing and removal of formwork**

Curing shall conform to the requirements defined in the relevant portion of the Section on ‘Concrete Work’ of this Specification. Side forms may be removed not less than 24 hours after placing the concrete, but the entire pile shall remain fully supported for at least seven days. When accelerated curing is used, the curing procedures shall have to be accepted by the Engineer.

15.2.5 **Marking of piles**

The head of each pile shall be permanently marked with its date of casting and reference number. The pile shall be indelibly marked at 1m interval along its length showing the distance from pile shoe. The top 3m of the pile shall be marked at 250mm interval.

15.2.6 **Protection of finished piles**

Protection of finished piles against aggressive soil conditions shall be provided by one of the following methods:

(a) Impervious liners to LWL – 2m

(b) Tanking/waterproofing of the piles to the depth referred under (a) above

(c) Painting with an approved two parts coal tar epoxy paint product suitable for mixing at Site immediately before application.

15.2.7 **Handling and storage of piles**

Pre-cast piles shall be lifted, handled, transported and stacked so that no damage occurs. The lifting points for each size of pile shall be marked as shown on the Drawings and as stated above with
waterproof paint and to be approved by the Engineer. The piles shall be transported and stacked with supports at the lifting points.

Piles are to be handled only when concrete has reached its characteristic strength as determined by field control test cylinders. Piles shall be handled carefully to avoid being dropped or severely jarred.

15.2.8 Pilot (Test) piles

The Contractor shall construct and drive pilot piles prior to commencement of piling for the permanent works. The permanent work piles shall not proceed until the testing of the pilot piles has been completed meeting all requirements of acceptance to the satisfaction of the Engineer.

Pilot piles shall be furnished for the lengths ordered and driven at the locations and to the elevations directed by the Engineer. In general, the ordered length of pilot piles will be greater than the estimated length of production piles in order to provide for variation in soil conditions.

The driving equipment used for driving pilot piles shall be identical to that which the Contractor shall propose to use in case of production piling.

Pilot piles shall be driven in positions specified by the Engineer. The Contractor shall notify the Engineer in advance of driving and shall supply the Engineer daily with a detailed record of the driving of the pilot piles.

Pilot piles shall be driven to a hammer blow count established by the Engineer at the estimated tip elevation. Pilot piles which do not attain the hammer blow count specified above at a depth of 0.3m above the estimated tip elevation shown on the plans, shall be allowed to “set-up” for a period of 12 to 24 hours as determined by the Engineer, before being re-driven. If the specified hammer blow count is not attained on re-driving, the Engineer may direct the Contractor to drive a portion or all of the remaining pilot pile length and repeat the “set-up”– re-drive procedure.

Driving of a pilot pile shall continue until the Engineer directs that it shall cease, in order to demonstrate that driving resistance continues to increase.

15.2.9 Driving of piles

The Contractor shall establish all lines, levels and be responsible for the correct positioning of all piles. Setting out shall be carried out from the main grid lines of the proposed structure. Immediately before installation, the pile position shall be marked with suitable identifiable pins or markers.

The position of the piles shall be set out in accordance with the Drawing from the established Bench Mark.

The pre-cast piles shall be driven to a pre-planned sequence approved by the Engineer and in the presence of the Engineer’s authorized representative in order to minimize the detrimental effects of heave and lateral displacement of the ground. No pile driving will be allowed at night unless prior permission is obtained from the Engineer.

Piles shall be protected with an approved cushion and cap while being driven. Pile driving shall be stopped when the maximum blows per 0.3m or the number specified on the Drawings are reached or if the pile head is damaged due to improper driving.
Piles shall be rigidly secured by leads or temporary guide structure against lateral movement during driving and shall be driven without interruption right from the first blow of the hammer until the required penetration has been attained.

Piles shall be driven to the positions, lines and elevations shown on the Drawings so that the pile center is within 75mm of the specified location point and with a deviation from the vertical of not more than two percent. If any pile is damaged or driven out of the specified tolerance, the Contractor shall immediately submit proposals for remedial measures to the Engineer for his written approval. Notwithstanding the Engineer’s approval, the Contractor shall be solely responsible for the design and cost of the remedial measures.

The Contractor shall keep a pile driving register in a format approved by the Engineer, where he shall record all data covering dimensions, elevation of point, top elevation after cut off, type, make and weight of hammer, height of fall of hammer, average penetration per blow under the last 20 blows and blow count per 0.3m throughout the full length of that pile. Five copies of the report shall be submitted to the Engineer before any payment will be made for this work.

**Driving equipment**

Before any driving takes place, the Contractor shall submit to the Engineer, for his approval, full details of all pile driving equipment, including the driving hammer, hammer cushion, drive head, pile cushion and other appurtenances and the proposed methods to be followed. The Contractor’s proposal shall not be on using water-jetting method.

Piles shall be driven by continuous vibratory percussion using steam, air, diesel or gravity hammers. The equipment shall have sufficient capacity to drive the pile to the design depth and set without damaging the pile.

Pile driving hammers, other than gravity hammers, shall be of the size needed to develop the energy required to drive piles at a penetration rate of not less than 2.5mm per blow at the required bearing value.

Gravity hammers shall not be used for concrete piles or for piles where design load capacity exceeds 30 Metric Tons. When gravity hammers are permitted, the ram shall weigh not less than 900 kg and the height of drop shall not exceed 4.5m. In no case the ram weight of a gravity hammer shall be less than the combined weight of the drive cap and pile. All gravity hammers shall be equipped with hammer guides to insure concentric impact on the drive head or pile cushion.

Open-end (single acting) diesel hammers shall be equipped with a device to permit the Engineer to determine hammer stroke at all times during pile driving operations. Closed-end (double acting) diesel hammers shall be equipped with a bounce chamber pressure gauge, in good working order and mounted near the ground level, to facilitate easy reading by the Engineer. The Contractor shall provide a correlation chart on bounce chamber pressure and delivered hammer energy.

Vibratory or other pile driving methods may be used only when specifically allowed by the special provisions or in writing by the Engineer. Except when pile lengths have been determined from load test piles, the bearing capacity of piles driven with vibratory hammers shall be verified. Such verification shall be carried out by re-driving the first pile driven in each group of 10 piles with an impact hammer of suitable energy to measure the pile capacity before driving the remaining piles in the group.

In case the required penetration is not obtained by the use of a hammer complying with the above minimum requirements, the Contractor shall be required to provide a hammer of greater energy or, when permitted, resort to supplemental methods such as jetting or pre-boring.
Driving appurtenances

Hammer cushion

All impact pile driving equipment, except gravity hammer, shall be equipped with a suitable thickness of hammer cushion material to prevent any damages to the hammer or pile and to insure uniform driving behaviour. Hammer cushions shall be made of durable manufactured materials, which will retain uniform properties during driving. Wood, wire rope, and asbestos hammer cushions shall not be used. The Contractor shall replace the hammer cushion before driving is permitted to continue whenever there is a reduction of hammer cushion thickness exceeding twenty-five percent of the original thickness.

Pile drive head

Pile, driven with impact hammer, shall be fitted with an adequate drive head to distribute the hammer blow to the pile head. The drive head shall be axially aligned with the hammer and the pile. The drive head shall be guided by the leads and not be free-swinging. The drive head shall fit around the pile head in such a manner as to prevent transfer of torsion forces during driving while maintaining proper alignment of hammer and pile. The pile head shall be plain and perpendicular to the longitudinal axis of the pile to prevent eccentric impacts from the drive head.

Pile cushion

When the nature of the driving of a concrete pile is such as to unduly injure it, shall be protected by a pile cushion. When plywood is used, the minimum thickness placed on the pile head, prior to driving, shall not be less than 100mm. A new pile cushion shall be provided, if the cushion is either compressed more than one-half the original thickness or begins to burn during driving. The pile cushion dimensions shall be such, as to distribute the blow of the hammer throughout the cross-section of the pile.

Leads

Pile driving leads, which support the pile and the hammer in proper positions throughout the driving operation, shall be used. Leads shall be constructed in a manner that allows movement of the hammer while maintaining alignment of the hammer and the pile to insure concentric impact for each blow. The leads shall be of sufficient length to make the use of a follower redundant and shall be so designed as to permit proper alignment of battered piles.

Followers

Followers shall only be used when approved in writing by the Engineer, or when specifically allowed as special provision. The follower and pile shall be held and maintained in equal and proper alignment during driving. The follower shall be of such material and dimensions to permit the piles to be driven to the length determined necessary from the driving of the full-length piles.

Driving procedures

Pile heads shall be squared up prior to driving. In addition, pile shoes may be used to protect the piles when hard driving is anticipated. The pile shoes shall be of the types and quality as shown on the Drawings and as specified in this Specifcation and/or as directed by the Engineer. They shall be used at the locations specified or ordered by the Engineer either.

Each pile shall be driven continuously until the specified set or depth has been reached, except that the Engineer may permit the suspension of driving, if he is satisfied that the rate of penetration prior to the
cessation will be substantially re-established on resumption or if he is satisfied that the suspension of driving is beyond the control of the Contractor.

Pile shall be driven to the minimum tip elevations and bearing capacity shown on the plans, specified in the special provisions or approved by the Engineer. Piles that heave more than 6mm upward during the driving of adjacent piles shall be re-driven.

Piles shall be driven with a variation of not more than 6mm per 0.3m from the vertical or from the batter shown on the Drawings, except that piles for trestle bents shall be so driven that the cap may be placed in its proper location without inducing excessive stresses in the piles. Foundation piles shall not be out of the position as shown on the Drawings by more than one-fourth of their diameter or 150mm, whichever is greater after driving. Any increase in footing dimensions or reinforcing due to out-of-position piles shall be at the Contractor’s own expenses.

At the start of work and in new sections, sets shall be taken at intervals during the last 3m of the driving to establish the behavior of the piles.

The Contractor shall give adequate notice and provide all facilities to enable the Engineer to check the driving resistance. A set shall be taken only in the presence of the Engineer unless otherwise approved.

The final set of each pile shall be recorded either as the penetration in millimeter per 10 blows or as the number of blows required to produce a penetration of 250mm. The exposed part of the pile and the driving equipment shall be in good condition when the final set is measured.

The Contractor shall inform the Engineer immediately in the event of an unexpected change in driving characteristics is noted.

When required, levels and measurements shall be taken to determine the movement of the ground or any pile resulting from the driving process.

When problems are encountered in the resistance to the pile being driven or with a pile rising as a result of driving of an adjacent pile, the Contractor shall seek and comply with the instructions of the Engineer on methods and procedures to overcome the problem. One of the methods may be that the Contractor shall provide a heavier hammer as decided by the Engineer or resort to jetting at his own expenses. The drop hammers shall be equipped with proper leads and hoisting equipment to handle the work efficiently. The fall of hammer shall not be more than 2.43m.

**Jets**

Jetting shall only be carried on, if the Engineer approves it in writing or when specifically allowed under the special provisions. The Contractor shall be responsible for all damages caused to the Site by the jetting operations.

When water jetting is followed, the number of jets along with the volume and pressure of water at the jet nozzles shall be adequate to freely erode the material adjacent to the pile. The plant shall have sufficient capacity to deliver at all times a minimum pressure of 293 kg/cm² at two number 20mm jet nozzles. In either case, unless otherwise indicated by the Engineer, jet pipes shall be removed when the pile tip remains at a minimum of 1.52m above the prescribed tip elevation and the pile shall be driven to the required bearing capacity with an impact hammer to secure the final penetration.

All jet water shall be controlled, treated if necessary and disposed of by the Contractor in a manner satisfactory to the Engineer.
Driving records

The Contractor shall keep a record of the installation of each pile and shall submit two signed copies to the Engineer, not later than noon of the next working day after the pile is installed. The record shall include the following data:

(a) Pile location
(b) Pile reference number
(c) Pile type
(d) Nominal cross-sectional dimensions
(e) Length of preformed pile
(f) Date and time of driving or re-driving
(g) Ground level at the commencement of installation of pile
(h) Working level
(i) Pile toe level
(j) Type, weight, drop and mechanical condition of hammer and similar information for other equipment
(k) Numbers and type of packing and type and condition of dolly used.
(l) Final set of pile
(m) If required, the sets taken at interval during the last 3m of driving
(n) If required, temporary compression of ground and pile from the time of a marked increase in driving resistance until the pile reaches its final level
(o) All information regarding obstructions, delays and interruptions to the sequence of work.

15.2.10 Repair of pile heads

When repairing the head of a pile, the head shall be cut off square at sound concrete and all loose particles shall be removed by wire brushing followed by washing with water.

If the pile is to be subjected to further driving, the head shall be replaced with concrete of an approved grade. Repaired piles shall not be driven until the added concrete has reached the specified strength of the concrete of the pile.

If the driving of a pile has been accepted but the sound concrete remained below the cut-off level, the pile shall be made good up to the cut-off level with concrete of a grade not inferior to that of the pile.

15.2.11 Extension of piles

Where it becomes necessary to extend a pile, the concrete at the end of the pile shall have to be broken in order to expose the reinforcing bars for a length of 40 bar diameters. Additional reinforcement shall be used in line with the pile axis as per the relevant Section/Sub-section of this Specification and/or in the way the Engineer directs. The additional concrete shall be of the same quality as that used in the pile. Prior to placing concrete, a construction joint shall be made in accordance with the specifications of the relevant Section/Sub-section of this Specification. Forms shall remain in place for a minimum of seven days.

15.2.12 Defective piles

The driving procedures shall not subject the piles to excessive abuse producing crushing and spalling of the concrete or deformation of the steel. Manipulation of piles to force them into proper position, considered by the Engineer to be excessive, shall not be permitted. Any piles, being damaged from internal defects or improper driving or driven out of its proper location or below the specified elevation,
shall be corrected at the Contractor’s own expenses by one of the following methods approved by the Engineer.

(1) The pile shall be withdrawn and replaced by a new and if necessary, a longer pile.

(2) A second pile shall be driven adjacent to the defective or low pile.

(3) The pile shall be spliced or built up or a sufficient portion of the footing shall be extended to properly embed the pile.

(4) All piles, pushed up by the driving of adjacent piles or by any other causes, shall be driven down again.

**15.2.13 Cutting off pile heads and bonding**

On completion of installation of piles, they shall be cut off to the required level as shown on the Drawings and to a tolerance of + 20mm or otherwise instructed by the Engineer. For pre-cast reinforced concrete piles, the main reinforcement shall be exposed and left reasonably straight for bonding into the pile cap. The minimum bond length of main reinforcement to be exposed shall be as given in the following table.

<table>
<thead>
<tr>
<th>Grade of Pile Cap Concrete</th>
<th>30</th>
<th>40 or more</th>
</tr>
</thead>
<tbody>
<tr>
<td>High Tensile Steel (f_y = 460 N/mm²)</td>
<td>37D</td>
<td>32D</td>
</tr>
<tr>
<td>Mild Steel (f_y = 250 N/mm²)</td>
<td>30D</td>
<td>25D</td>
</tr>
</tbody>
</table>

D = nominal diameter of bar.

In the stripping of pile heads, the concrete shall be stripped up to such a level that the remaining concrete will project 75mm into the pile cap.

Where a pile has been formed below the required cut-off level, it shall be built-up and the reinforcement shall project for such length as given above.

The method of cutting the pile heads shall have to be accepted by the Engineer.

**15.2.14 Measurement**

This work shall be measured separately for production of piles, pile driving and providing pile shoe.

Measurements of production of piles for concrete shall be taken for payment in cubic meters of pre-cast reinforced concrete produced in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer on the basis of specified pile length. Only the completed works as accepted by the Engineer will be eligible for payment.

Driving of pile shall be measured in linear meters of the length of pile driven complete and accepted. Cut off length shall not be measured for payment. The length of pile driven shall be measured from the pile toe to the cut-off level.

Pile shoe shall be measured by number that has been produced in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.
15.2.15 Payment

The amount of completed and accepted concrete work measured as provided above shall be paid for at the Contract unit prices per cubic meter of pile produced and linear meter of pile driven, as shown in the Bill of Quantities. The payment shall constitute the full compensation for the production of concrete, transportation of piles, all materials used, arranging all equipment, machinery and all labour, tools and incidentals necessary to complete the work satisfactorily true to the Specifications but excluding the cost of reinforcement with its fabrication and pile shoe. Reinforcement and pile shoe shall be paid separately as shown in the Bill of Quantities. No payment shall be made for pre-cast pile concrete until concrete test results demonstrate that the piles have achieved the specified strength.

The amount of completed and accepted pile driving work measured as provided above shall be paid for at the Contract unit prices per linear meter of pile driven. The payment shall be the full compensation for the driving of the piles including, pile head breaking and repair, squaring up pile heads, construction and removal of any cofferdam, arranging rigs, cranes, hammers, leaders and all other necessary driving equipment, driving and all labour, tools and incidentals necessary to complete the work satisfactorily true to the Specifications but excluding the cost of reinforcement with its fabrication and pile shoe. No payment shall be made for driving piles until and unless the piles have been driven to the specified depth.

The pile shoe as accepted and measured as provided above shall be paid for at the Contract unit price per number, as contained in the Bill of Quantities and the payment shall constitute fabrication, furnishing and filling of the pile shoes according to prescribed Specification.

When pilot piles are incorporated in the foundation as working piles, no additional payment shall be made for the piles so utilized other than as for a pilot pile.

No payment shall be made for unauthorized, defective, unsound or unsatisfactory driven piles for any costs incurred by the Contractor for such piles.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Supply of pre-cast reinforced concrete piles (excluding cost of reinforcement and pile shoe)</td>
<td>Cubic meter</td>
</tr>
<tr>
<td>Driving of pre-cast reinforced concrete piles including pile head breaking and repair</td>
<td>Linear meter</td>
</tr>
<tr>
<td>Supply and fitting of pile shoe for pre-cast piles</td>
<td>Each</td>
</tr>
</tbody>
</table>
15.3 **Sand compaction piles**

15.3.1 **General**

Sand compaction pile shall generally be used for improvement of sandy as well as clayey soil in alluvial plain, mostly plains, reclaimed land and marshy land.

The equipment used for sand compaction piles are:

- Derrick/winch
- Casing pipe
- Drop hammer

Drop hammer weighing 1 ton and above shall be of solid cylinder and sectional area shall be such that it can play within the casing pipe. Diameter of hammer section shall be about 6mm to 8mm lower than the opening of the casing pipe. The casing pipe shall be of Mild Steel.

15.3.2 **Construction process**

A small hole of about 50mm depth shall be made on the ground with the hammer. Casing pipe shall be installed at that point and kept vertical. The casing pipe shall be held in position with the wire rope connected to the winch i.e. downward movement of the casing pipe shall be restrained.

Coarse sand of F.M. between 1.5 and 2.5 or sand gravel mix as provided in the design shall be placed within the casing pipe (250mm to 300mm dia) upto a depth of about 1.0 to 1.5m.

The coarse sand or sand gravel mix placed at the tip of casing pipe shall then be compacted to form a solid mass by the hammer holding the casing pipe in position.

Casing pipe shall then be allowed to go downward with application of hammer blows on the shoe formed at the tip and shall be penetrated to the desired depth.

As the casing pipe reaches the desired depth, the pipe shall be pulled back by about 30cm from the lowest position and then held firmly from the winch. Artificial shoe shall then be detached from the casing pipe with the hammer blows.

The casing shall be drawn to a desired height and sand be discharged into the casing pipe upto a depth of about 1.5 to 2 times the drawn up height of the casing pipe.

The discharged sand shall be compacted by the hammer and the process of compaction shall continue until the casing pipe reaches the ground surface.

The removed volume of sand or sand gravel mix, measured on the ground, shall be between 120% and 130% of the designed volume.

Effect of improvement of the foundation soil shall be confirmed by SPT, CPT or any other method as approved by the Engineer between and at the centre of piles so as to satisfy the desired bearing capacity.
15.3.3 Measurement

Measurement shall be taken for payment in linear meter of pile constructed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

15.3.4 Payment

Sand compaction pile as measured above shall be paid at the Contract unit price per linear meter. The rate shall constitute the full compensation of furnishing all materials like sand, gravel and other materials, driving pile holes, lowering casing pipe, placing of materials and their compaction, SPT/CPT or any other method and tests for determining the improvement in the bearing capacity, arranging all equipment and machinery, labour, tools and incidentals necessary to complete the Work as per requirements described under this item of work, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

No payment shall be made for unauthorized, defective, unsound or unsatisfactorily piles or for any costs incurred by the Contractor for such piles.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Driving and production of sand compaction pile</td>
<td>Linear meter</td>
</tr>
</tbody>
</table>
15.4 Timber piles

15.4.1 Description

This work shall consist of supplying and driving foundation piles made of timber of the type and dimension in accordance with these Specifications and as shown on the Drawings and/or as directed by the Engineer.

15.4.2 Materials

Timber piles shall adhere to the requirements of the Specifications for Wood Products, AASHTO M 168. The grades of timber to be used shall be as shown in the Plans or in the BOQ or in the special provisions. They should be matured, sound and free from large or loose knots, cracks sharp crooks and bends or decay or other defects and sufficiently straight so that a line drawn from the centre of the butt to the centre of the tip shall be contained entirely within the pile.

15.4.3 Preservatives and treatments

Timber piles shall be treated or untreated. When Specifications demand treatment of timber piles it should follow strictly the requirements and methods conforming AASHTO M 133 or BDS 819 : 1975. Unless and otherwise specially required on the design Drawings, timber piles those are required to be treated should be impregnated with solignum, creosote or treated with some such anti-rot compounds. Timber piles those are required to be painted may be treated with pentachlorophenol with a Type C solvent or with a water-borne preservative. Preservative treatment may not be necessary for piles, which will be completely and permanently submerged in waterlogged ground. In this case seasoning is not necessary and piles may be stored in water prior to use.

15.4.4 Handling and storage of piles

Timber piles shall be lifted, handled, transported and stacked in a way, which will keep the damages to the piles at minimum.

Untreated material shall be open-stacked on supports at least 300mm above the ground surface to avoid absorption of ground moisture and allow free circulation of air. When necessary, the Contractor shall provide shade or appropriate protection from weather by a suitable covering. The storage area shall be such that no water shall collect under or near the stored timber piles.

15.4.5 Pile driving

Pile driving equipment shall secure that it will not damage the pile during driving. List of all driving equipment and appurtenances shall be furnished by the Contractor, which shall be approved by the Engineer in advance of any driving taking place. However, approval of pile driving equipment shall not relieve the Contractor of his responsibilities of driving piles without damage.

Collars, bands or other devices shall be provided to protect piles against splitting and brooming as and when it would be necessary under the driving conditions.

The Contractor shall establish all lines, levels and be responsible for the correct positioning of all piles. Setting out shall be carried out from the main grid lines of the proposed structure. Immediately before installation of the pile, the pile position shall be marked with suitable identifiable pins or markers.

The positions of the piles shall be set out in accordance with the Drawings from the established Bench Mark.
The bark of the piles shall be removed before driving. The bottom is shaped conically for a length of 1½ to 2 times the diameter or about 0.3m and where the ground is hard it is protected with an iron shoe of ’V’ shape or it should be of a design as specified on the Drawings and/or approved by the Engineer. Piles protected by shoes should have a blunt end 100mm to 200mm in diameters. The points of the piles shall be carefully shaped to secure an even and uniform bearing of the shoes.

The piles shall be driven to a pre-planned sequence approved by the Engineer and in presence of the Engineer’s authorized representative in order to minimize the detrimental effects of heave and lateral displacement of the ground. No pile driving shall be allowed at night without prior permission from the Engineer.

Piles shall be rigidly secured by leads or temporary guide structure against lateral movement during driving and shall be driven without interruption right from the first blow of the hammer until the required penetration has been attained.

Each pile shall be driven continuously until the specified set or depth has been reached except that the Engineer may permit the suspension of driving, if he is satisfied that the rate of penetration prior to the cessation will be substantially re-established on resumption or if he is satisfied that the suspension of driving is beyond the control of the Contractor.

Piles shall be driven to the positions, lines and elevations shown on the Drawings and/or as per the direction of the Engineer so that the pile centre remains within 75mm of the specified location point and with a deviation from the vertical of not more than 2%. If any pile is damaged or driven out of the specified tolerance, the Contractor shall submit his proposed remedial measures to the Engineer for his written approval. Notwithstanding the Engineer’s approval, the Contractor shall be solely responsible for the design and all costs of the remedial works.

At the start of work and in new sections, sets shall be taken at intervals during the last 3m of the driving to establish the behavior of the piles.

The Contractor shall give adequate notice and provide all facilities to enable the Engineer to check the driving resistance. A set shall be taken only in the presence of the Engineer, unless otherwise approved.

The final set of each pile shall be recorded either as the penetration in millimeter per 10 blows or as the number of blows required to produce a penetration of 250mm. The exposed part of the pile and the driving equipment shall be in good condition when the final set is measured.

Piles should never be driven to “refusal”. Piles shall be considered sufficiently driven, when five blows fail to drive more than 12mm or when the last blow does not sink the head more than 7mm.

The Contractor shall inform the Engineer immediately in the event of an unexpected change in driving characteristics is noted.

When required, levels and measurements shall be taken in order to determine the movement of the ground or any piles resulting from the driving process.

When problems are encountered in the resistance to the pile being driven or with a pile rising as a result of driving of an adjacent pile, the Contractor shall seek and comply with the instructions of the Engineer on methods and procedures to overcome the problem.

The Contractor shall keep a pile driving register in a format approved by the Engineer recording all data covering dimensions, elevation of point, top elevation after cut off, type, make and weight of hammer,
height of fall of hammer, average penetration per blow under the last 5 blows. Five copies of the report shall be submitted to the Engineer before any payment will be made for this work.

15.4.6 Pile cut-off

Timber piles shall be cut-off to a true plane at the elevations required and anchored to the structure, as shown on the Drawings. All cut-off lengths of piling shall remain the property of the Contractor and shall be properly disposed of.

Timber piles, which support timber caps or grillage, shall be sawed to conform to the plane of the bottom of the superimposed structure. In general, the length of piles above the elevation of cut-off shall be sufficient to permit the complete removal of all materials injured by driving, but piles driven to very nearly the cut-off elevation shall be carefully adzed or otherwise freed from all broomed, splintered, or otherwise injured materials.

Immediately after making final cut-off on treated timber foundation piles, the cut area shall be given two liberal applications of preservative followed by a heavy application of approved sealer. Treated timber piles which will have the cut-off exposed in the structure shall have the cut area treated with three coats of a compatible preservative material meeting the requirements of AWPA Standard M4-A minimum time period of 2 hours shall elapse between each application.

If concrete cap is provided, the piles should be embedded for a depth sufficient to ensure transmission of load. The concrete should be at least 150mm outside the piles and be suitably reinforced to prevent splitting.

15.4.7 Measurement

This work shall be measured separately as the length in linear meters of timber pile supplied and accepted and the length in linear meters of pile driven complete upto the required depth in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment. The length of pile furnished shall be measured in accordance with the specified length. The length of piles driven shall be measured from the pile toe to the cut-off level.

15.4.8 Payment

The amount of completed and accepted work measured as provided above shall be paid for at the Contract unit prices per linear meter of pile furnished and linear meter of pile driven, as shown in the Bill of Quantities. The payment shall constitute the full compensation for the supply, treatment, handling and driving of piles including all driving, arranging all required equipment and machinery and all incidentals to complete the Work as per requirements described under this item of work, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer. Pile shoe shall be paid separately as shown in the Bill of Quantities.

No payment shall be made for unauthorized, defective, unsound or unsatisfactorily piles or for any costs incurred by the Contractor for such piles.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Supply of timber piles</td>
<td>Linear meter</td>
</tr>
<tr>
<td>Driving of timber pile</td>
<td>Linear meter</td>
</tr>
<tr>
<td>Supply and fitting of pile shoe</td>
<td>Each</td>
</tr>
</tbody>
</table>
SECTION-16
PILE LOAD TESTING
## CONTENTS

### SECTION-16

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>16.1</td>
<td>Description</td>
<td>178</td>
</tr>
<tr>
<td>16.2</td>
<td>General</td>
<td>178</td>
</tr>
<tr>
<td>16.3</td>
<td>Definitions</td>
<td>178-179</td>
</tr>
<tr>
<td></td>
<td>Allowable load</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Compression pile</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Kentledge</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Maintained load test</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Pilot pile</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Proof load</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Reaction system</td>
<td>178</td>
</tr>
<tr>
<td></td>
<td>Tension pile</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Test pile</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Ultimate bearing capacity</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Working load</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Working pile</td>
<td>179</td>
</tr>
<tr>
<td>16.4</td>
<td>Supervision</td>
<td>179</td>
</tr>
<tr>
<td>16.5</td>
<td>Safety Precautions</td>
<td>179-180</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Kentledge</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Tension piles and ground anchors</td>
<td>179</td>
</tr>
<tr>
<td></td>
<td>Testing equipment</td>
<td>180</td>
</tr>
<tr>
<td>16.6</td>
<td>Construction of a pilot pile to be test loaded</td>
<td>180</td>
</tr>
<tr>
<td></td>
<td>Notice of construction</td>
<td>180</td>
</tr>
<tr>
<td></td>
<td>Method of construction</td>
<td>180</td>
</tr>
<tr>
<td></td>
<td>Boring or driving record</td>
<td>180</td>
</tr>
<tr>
<td></td>
<td>Cut off level</td>
<td>180</td>
</tr>
<tr>
<td></td>
<td>Pile for compression tests</td>
<td>180</td>
</tr>
<tr>
<td>16.7</td>
<td>Preparation of a working pile to be tested</td>
<td>181</td>
</tr>
<tr>
<td>16.8</td>
<td>Reaction system</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Compression tests</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Working piles</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Spacing</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Adequate reaction</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Care of piles</td>
<td>181</td>
</tr>
<tr>
<td></td>
<td>Loading arrangement</td>
<td>181</td>
</tr>
<tr>
<td>16.9</td>
<td>Equipment for applying load</td>
<td>181</td>
</tr>
<tr>
<td>16.10</td>
<td>Measurement of load</td>
<td>182</td>
</tr>
<tr>
<td>16.11</td>
<td>Adjustment of Loading Equipment</td>
<td>182</td>
</tr>
<tr>
<td>16.12</td>
<td>Measuring of pile head movement</td>
<td>182-183</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>182</td>
</tr>
<tr>
<td></td>
<td>Leveling method</td>
<td>182</td>
</tr>
<tr>
<td></td>
<td>Independent reference frame</td>
<td>182-183</td>
</tr>
<tr>
<td></td>
<td>Other methods</td>
<td>183</td>
</tr>
</tbody>
</table>
16.13 Protection of testing equipment
   Protection from weather
   Prevention of disturbance
   183
16.14 Supervision
   Notice of test
   Records
   183
16.15 Test procedures
   Loading procedures
   Safe load through initial test (Single pile)
   Pile foundation design and construction
   Presentation of results
   183-186
16.16 Completion of a test
   Measuring equipment
   Kentledge
   Temporary piles
   186
16.17 Measurement
   186
16.18 Payment
   186
SECTION-16
PILE LOAD TESTING

16.1 Description
In carrying out the Initial Test, the works shall consist of the application of compression load tests to pre-cast and driven or cast in-situ piles selected by the Engineer to determine the potential bearing capacity and adequacy of the piles by measurement of their settlement behavior under the test loads.

16.2 General
This Section deals with the testing of a pile by the application of an axial load or force. It covers vertical piles tested in compression.

16.3 Definitions

Allowable load
The load which may be safely applied to a pile after taking into account its ultimate bearing capacity, negative friction, pile spacing, overall bearing capacity of the ground below and allowable settlement.

Compression pile
It is a pile, which is designed to resist an axial force that would cause it to penetrate further into the ground.

Kentledge
The dead weight used in a load test.

Maintained load test
A loading test in which each increment of load is held constant either for a defined period of time or until the rate of movement (settlement or uplift) falls to a specified value.

Pilot pile
A pile installed before the commencement of the main piling works or a specific part of the works for the purpose of establishing the suitability of the chosen type of pile and for confirming its design, dimensions and bearing capacity. Pilot piles may be utilized as working piles, subject to the Engineer’s approval.

Proof load
A load applied to a selected pile to confirm that it is suitable for the load at the settlement specified. A proof load should not normally exceed 200% of the working load on a pile except under the circumstances where special provisions are provided for the testing of pre-cast piles driven to a set. In these circumstances 300% is specified.

Reaction system
The arrangement of kentledge, piles, anchors or rafts that provides a resistance against which the pile is tested.
Tension pile
It is a pile, which is designed to resist an axial force that would cause it to be extracted from the ground.

Test pile
Any pile to which a test loading is or is to be applied.

Ultimate bearing capacity
It is the load at which the resistance of the soil becomes fully mobilized.

Working load
It is the load, which the pile is designed to carry.

Working pile
It is one of the piles forming the foundation of a structure.

16.4 Supervision
All tests shall be carried out under the direction of an experienced and competent supervisor conversant with the test equipment and test procedures. All personnel, operating the test equipment, shall have been trained in the use.

16.5 Safety precautions
General
When preparing for conducting and dismantling a pile test, the Contractor shall carry out the requirements of the various regulations and other statutory requirements those are applicable to the work for the provision and maintenance of safe working conditions. In addition, it shall make such other provisions as may be necessary to safeguard against any hazard that is involved in the testing or preparations for testing.

Kentledge
Where kentledge is used, the Contractor shall construct the foundations for the kentledge and any crib work, beams or other supporting structure in such a manner that there will be no differential settlement, bending or deflection of an amount that constitutes a hazard to safety or impairs the efficiency of the operation. The kentledge shall be adequately bonded, tied or otherwise held together to prevent it falling apart or becoming unstable because of deflection of the supports.

The weight of kentledge shall be greater than the maximum test load. If the weight is estimated from the density and volume of the constituent materials, an adequate factor of safety against error shall be allowed.

Tension piles and ground anchors
Where tension piles or ground anchors are used, the Contractor shall ensure that the load is correctly transmitted to all the rods or bolts. The extension of rods shall not be permitted by welding, unless it is known that the steel will not be reduced in strength by welding. The bond stresses of the rods in tension shall not exceed normal permissible bond stresses for the type of steel and grade of concrete used.
Testing equipment

In all cases the Contractor shall ensure that when the hydraulic jack and load measuring devices are mounted on the pile head, the whole system shall be stable up to the maximum load to be applied. Means shall be provided to enable dial gauges to be read from a position clear from the kentledge stack or test frame in conditions where failure in any part of the system due to overloading, buckling, loss of hydraulic pressure and so on might constitute a hazard to personnel.

The hydraulic jack, pump, hoses, pipes, couplings and other apparatus to be operated under hydraulic pressure shall be capable of withstanding a test pressure of 1.5 times the maximum working pressure without leaking.

The maximum test load or test pressure, expressed as a reading on the gauge in use, shall be displayed and all operators shall be aware of this limit.

16.6 Construction of a pilot pile to be test loaded

Notice of construction

The Contractor shall give the Engineer at least 48 hours notice of the commencement of construction of any pilot pile, which is to be test loaded.

Method of construction

Each pilot test pile shall be constructed in a manner similar to that to be used for the construction of the working piles and by the use of similar equipment and materials. Any variation shall only be permitted with prior approval of the Engineer.

Extra reinforcement and concrete of increased strength shall be permitted in the shafts of pilot pile at the discretion of the Engineer.

Boring or driving record

For each pilot pile that is to be tested, a detailed record of the soils encountered during boring or of the progress during driving shall be made and submitted to the Engineer daily not later than noon on the next working day.

Cut-off level

The pile shaft shall terminate at the normal cut off level or at a level required by the Engineer.

The pile shaft shall be extended where necessary above the cut-off level of working piles so that gauges and other apparatus to be used in the testing process are not damaged by water or falling debris.

Pile for compression tests

A pile shall not be tested until the curing period is over. In the case of a driven pile, the period shall not be earlier than 72 hours after the driving of the pile is complete.

For a pile that is tested in compression, the head of the test pile shall be cut off, leveled and capped with a steel plate to produce a level bearing surface square to the axis of the pile and sufficiently large to accommodate the loading and settlement measuring equipment. It shall also be adequately reinforced or protected to prevent damage from the concentrated application of load from the loading equipment.
16.7 Preparation of a working pile to be tested

If a test is required on a working pile, the Contractor shall cut down or otherwise prepare the pile for testing as required by the Engineer in accordance with the provisions of the relevant Sub-sections of this Specification.

16.8 Reaction system

Compression tests

Compression tests shall be carried out using kentledge, tension piles or specially constructed anchorage.

Where kentledge is to be used, it shall be supported on crib work disposed around the pile head so that its centre of gravity is on the axis of the pile. The bearing pressure under supporting cribs shall be such as to ensure stability of the kentledge stack. Kentledge shall not be carried directly on the pile head, except when directed by the Engineer.

Working piles

Where working piles are used as reaction piles, their movement shall be measured to within an accuracy of 0.5mm.

Spacing

Where kentledge is used for loading vertical piles in compression, the distance from the edge of the test pile to the nearest part of the crib supporting the kentledge stack in contact with the ground shall be not less than 1.3m.

The center to center spacing of vertical reaction piles including working piles used as reaction piles, from a test pile shall be not less than three times the diameter of the test pile or the reaction piles or 2m, whichever is the highest.

Adequate reaction

The size, length and number of the piles or anchors, or the area of rafts shall be adequate to transmit the maximum test load to the ground in a safe manner without excessive movement or influence on the test pile.

Care of piles

The method employed in the installation of any reaction pile, anchor or raft shall be such as to prevent damage to any test pile or working pile.

Loading arrangement

The loading arrangement used shall be designed to transfer safely to the test pile the maximum load required in testing. Full details shall be submitted to the Engineer prior to any work related to the testing process being carried out at the Site.

16.9 Equipment for applying load

The equipment used for applying load shall consist of one or more hydraulic rams or jacks. The total capacity of the jacks shall be at least equal to the required maximum load. The jack or jacks shall be arranged in conjunction with the reaction system to deliver an axial load to the test load required for the test.
16.10 Measurement of load

The Contractor shall supply measuring devices to determine the load on the pile, which shall require the Engineer’s approval. Certificates of calibration shall be supplied to the Engineer.

In addition, large diameter (exceeding 1.2m) test piles shall be instrumented at 5 (five) different depths to measure the load distribution along the piles. The instrumentation shall consist of both a mechanical system and strain gauges for measuring the pile deformation. The mechanical system shall consist of 6mm steel rods or high tensile steel wires (Gauge No. 23) placed in steel tubes down to the various depths and connected to dial gauges at the top. The strain gauges shall be of a stable type, wholly protected by a steel capsule. They shall be welded to the steel reinforcement, 2 (two) gauges at each depth.

The approval of the Engineer shall be obtained on the type of gauges to be used and other details on the instrumentation.

16.11 Adjustment of loading equipment

The loading equipment shall be capable of adjustment throughout the test to obtain a smooth increase of load or to maintain each load constant at the required stages of a maintained loading test.

16.12 Measuring of pile head movement

General

In a maintained load test, movement of the pile head shall be measured by two of the methods as described below. One method for settlement measurements and the other for control.

Leveling method

An optical or any other leveling method by reference to an external datum may be used.

Where a level and staff are used, the level and scale of the staff shall be chosen to enable readings to be made within an accuracy of 0.5mm. A scale attached to the pile or pile cap may be used instead of a leveling staff. At least two datum points shall be established on permanent objects or other well-founded structures, or deep datum points shall be installed. Each datum point shall be situated so that only one setting up of the level is needed.

No datum point shall be affected by the test loading or other operations at the Site.

The written approval of the Engineer shall be required in the case any other method of leveling is proposed.

Independent reference frame

An independent reference frame may be set up to permit measurement of the movement of the pile. The supports for the frame shall be founded in such a manner and at such a distance from the test pile, kentledge support cribs, reaction piles, anchorage and rafts so that movements of the ground in the vicinity of the equipment do not cause movement of the reference frame during the testing. Observations of any movement of the reference frame shall be made and a check shall be made on the movement of the pile head in relation to an external datum during the progress of the test. In no case shall the supports be less than 3 (three) test pile diameters or 2m, whichever is greater, from the center of the test pile.
The measurement of pile movement shall be made by two dial gauges rigidly mounted on the reference frame that bear on surfaces normal to the pile axis fixed to the pile cap or head. Alternatively, the gauges may be fixed to the pile and bear on the surfaces on the reference frame. The dial gauges shall be placed in diametrically opposite positions and be equidistant from the pile axis. The dial gauges shall enable the readings to be made within an accuracy of 0.1mm.

The reference frames shall be protected from sun and wind.

Other methods

The Contractor may submit for approval of the Engineer any other method for measuring the movement of pile heads.

16.13 Protection of testing equipment

Protection from weather

Throughout the test period, all equipment for measuring load and movement shall be protected from weather.

Prevention of disturbance

Construction equipment and persons who are not involved in the testing process shall be kept away at a sufficient distance from the test to avoid disturbance to the measurement apparatus.

16.14 Supervision

Notice of test

The Contractor shall give the Engineer at least 24 (twenty-four) hours notice of the commencement of the test.

Records

During the progress of a test, the testing equipment and all records of the test, required under this Specification, shall be made available for inspection by the Engineer.

16.15 Test procedures

Loading procedures

The test load shall be concentrically applied to the pile by such a method that the load acting on the pile at any time may be determined, adjusted and controlled. The load shall be applied to the pile as near to the ground surface as possible.

The load shall be applied and removed in increments based on the anticipated working load of the pile with the following schedule.

<table>
<thead>
<tr>
<th>Load as percentage of working load</th>
<th>Minimum time of holding load</th>
</tr>
</thead>
<tbody>
<tr>
<td>Board piles</td>
<td>Driven piles</td>
</tr>
<tr>
<td>25</td>
<td>50</td>
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<tr>
<td>50</td>
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<td>125</td>
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<tr>
<td>100</td>
<td>150</td>
</tr>
<tr>
<td>75</td>
<td>125</td>
</tr>
<tr>
<td>Load as percentage of working load</td>
<td>Minimum time of holding load</td>
</tr>
<tr>
<td>-----------------------------------</td>
<td>-----------------------------</td>
</tr>
<tr>
<td>Board piles</td>
<td>Driven piles</td>
</tr>
<tr>
<td>50</td>
<td>100</td>
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<td>25</td>
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<td>100</td>
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<td>125</td>
<td>200</td>
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<td>175</td>
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<td>75</td>
<td>150</td>
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<tr>
<td>50</td>
<td>100</td>
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<tr>
<td>25</td>
<td>50</td>
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<tr>
<td>0</td>
<td>0</td>
</tr>
</tbody>
</table>

Following application of each increment of load, the load shall be held for not less than the period shown in the Table and until the rate of settlement is less than 0.25mm per hour and is slowing. Readings of settlement and of the time at which they are made, shall be taken immediately before and after the application of each load increment and at intermediate intervals 20 (twenty) minutes apart for the first four hours and 60 (sixty) minutes apart thereafter.

Each stage of unloading shall proceed to the schedule shown in the Table. During unloading, readings of rebound and of the time at which they are made, shall be taken immediately after each increment of load is removed and at intermediate intervals of 20 (twenty) minutes. A final rebound reading shall be taken 24 (twenty-four) hours after the entire test loads have been removed.

A check for the accuracy of the measurement of settlement shall be made from a fixed reference point on a permanent object or well founded structure. This reference point shall be at least 3m off from the nearest point of the test pile.

During the progress of the test, all readings taken shall be available for inspection by the Engineer.

**Safe load through initial test (Single pile)**

The safe load on single pile shall be the smallest of the following:

(a) Two-thirds of the final load at which the total settlement attains a value of 12mm, unless it is established that a total settlement different from 12mm is permissible in a given case on the basis of nature and type of structure. In the later case, the actual total settlement permissible shall be used for assessing the safe load instead of 12mm.

(b) Two-thirds of the final load at which the net settlement attains a value of 6mm.

(c) 50 percent of the final load at which the total settlement equals one-tenth of pile diameter.

**Pile foundation design and construction**

The safe load on groups shall be the smallest of the following:

(a) The final load at which the total settlement attains a value of 25mm, unless a total settlement different from 25mm is specified in a given case on the nature and type of structure.
(b) Two-thirds of the final load at which the total settlement attains a value of 40mm.

**Presentation of results**

Within 24 hours of completion of the test, the Contractor shall submit a summary of results to the Engineer stating, for each stage of loading and unloading, the period for which the load was held and the maximum settlement or rebound.

Within seven days of completion of the test, the Contractor shall submit a full schedule of test date to the Engineer, which shall include the following:

a) **General**
   - Project.
   - Contract identification.
   - Proposed structure.
   - Date of test.

b) **Pile details**
   - Date of casting.
   - Pile reference number and location.
   - Type of pile.
   - Length on ground.
   - Level of toe.
   - Ground level at pile position.
   - Head level at which test load applied.
   - Condition of pile head.
   - Details of permanent casing.

c) **Installation details**
   - Dates and times of boring, driving and pouring concrete of test pile and adjacent piles.
   - Date and time of casting concrete.
   - Driven length of pile or temporary casing at final set.
   - Hammer type, size or weight.
   - Dolly and packing, type and condition before and after driving.
   - Driving length (depth, blows per 250mm, interruptions or breaks in driving).
   - At final set and at re-drive set, for drop of single action hammer the length of the drop or stroke, for diesel hammer the length of the stroke and the blows per minutes, for double-acting hammers the number of blows per minute.
   - Condition of pile head or temporary casing after driving.

d) **Test procedure**
   - Weight of kentledge
   - Tension pile, ground anchor or compression pile details
   - Plan of test arrangements showing position and distances of kentledge supports
   - Rafts, tension or compression plies and reference frame to test pile
   - Jack capacity
   - Method of Load measurement
   - Method of penetration measurement
   - Relevant dates and times
e) **Full test results**

- In tabular and graphical form detailing loads, time and movement. The graphical presentation shall consist of:
  - Load verses Time
  - Load verses gross settlement and rebound for each cycle of loading
  - Load verses net settlement and rebound for each cycle of loading.

### 16.16 Completion of a test

**Measuring equipment**

On completion of a test, all equipment and measuring devices shall be dismantled, checked and either stored so that they are available for use in further tests or removed from the Site.

**Kentledge**

Kentledge and its supporting structure shall be removed from the test pile and stored so that they are available for use in further tests or removed from the Site.

**Temporary piles**

On completion of a preliminary test, temporary tension piles shall be cut off below ground level, removed from the Site and the ground made good with approved materials as specified.

### 16.17 Measurement

Measurement shall be taken for payment as the number of pile compression load tests satisfactorily conducted and completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 16.18 Payment

The amount of completed and accepted work measured as provided above shall be paid for at the Contract unit prices per load test as shown in the BOQ. The payment shall constitute the full compensation for conducting the test including staging, all test equipment, provision for kentledge, carrying out tests, dismantling of equipment and removal from Site, cleaning of Site, preparation of all reports, materials, personnel, tools and all incidentals necessary to complete the entire tests.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Load test on piles</td>
<td>Each</td>
</tr>
</tbody>
</table>
17.1 Patent stone floor       187-188
  17.1.1 Description        187
  17.1.2 Materials        187
    Cement        187
    Coarse aggregate        187
    Fine aggregate        187
    Water        187
  17.1.3 Construction method        187-188
  17.1.4 Measurement        188
  17.1.5 Payment        188

17.2 Mosaic floor        189-191
  17.2.1 Description        189
  17.2.2 Materials        189
    Marble chips        189
    White cement        189
    Grey cement        189
    Water        189
    Glass strips        189
  17.2.3 Construction methods        189-191
  17.2.4 Measurement        191
  17.2.5 Payment        191

17.3 Non-skid floor tiles        192-193
  17.3.1 Description        192
  17.3.2 Materials        192
    Tiles        192
    Mortar        192
    Grout        192
    Water        192
  17.3.3 Construction methods        192-193
  17.3.4 Measurement        193
  17.3.5 Payment        193
SECTION-17
FLOOR

17.1 Patent stone floor

17.1.1 Description
Works covered under this item shall consist of constructing steel-trowelled concrete topping of specified thickness, prepared with stone/picked jhama brick chips, sand and cement in specified proportion in panels not exceeding 900mm on floor slabs and elsewhere including finishing the top with neat cement in accordance with these specifications or as contained in the BOQ or as shown on the Drawings and/or as directed by the Engineer.

17.1.2 Materials

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Coarse aggregate
Coarse aggregate shall conform to the requirements of ASTM C 330 or BDS 243 and to the requirements specified in the relevant Section titled ‘Construction Materials’ of this Specification.

Coarse aggregate shall be hard, durable, clean, free from dust and other deleterious materials prepared from stone chips (or picked jhama bricks conforming BDS 208 when specifically allowed) and 10mm down graded.

Fine aggregate
Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus obtained by mixing 50% sand of FM 1.2 and 50% of minimum FM 2.5. It shall conform to the requirements of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

17.1.3 Construction methods
Preparation of concrete mixture shall also be similar to what have been stated under the relevant Sub-sections of the Section on ‘Concrete Work’ of this Specification. The concrete mixture shall be prepared with one part cement, two parts sand and four parts stone chips/brick chips.

Before proceeding with the work, a sample panel of artificial Patent Stone Floor shall be prepared with specifications as required under the item of the work of the BOQ for approval of the Engineer.

The flooring shall be laid preferably no later than 24 hours after the floor slab is installed.
When the floor is to be laid on an old sub-floor, the base surface shall be thoroughly cleaned of laitance and all other loose foreign materials by stiff wire brush, roughened, if deemed necessary and washed and soaked with clean water. All excess water shall be removed ahead of the application of the bonding slurry so that the concrete surfaces become uniformly damp but not glisten-wet.

Creamy bonding slurry of neat cement shall be applied and well scrubbed on to the surface with stiff bristle brush. Bonding slurry shall be mixed and applied in a quantity as will be required to cover by the succeeding coat before the slurry dries out. In general, slurry shall be made in a quantity sufficient for an area not over 10 square meter at one time in order to maintain a “Live Glue” for bonding. The slurry shall be applied by brush in small areas not exceeding 0.5 square meter. Excess or dead slurry shall be constantly removed from the base by broom.

The ingredients of the artificial Patent Stone Floor should be mixed in a similar way as have been described in the relevant Sub-sections of the Section on ‘Concrete Work’ of this Specification. The concrete mix of the specified proportion shall be applied promptly with the specified thickness after putting the cement slurry and before the slurry becomes hard and dry.

All constituents shall be thoroughly mixed. No re-tampered materials and no partially set materials shall be used in the work.

The floor shall be divided into panels of specified size by means of wooden battens. The top of the battens shall be at the level of the finished floor surface.

The mixture shall be laid evenly between the battens in alternate panels and shall be uniformly consolidated and leveled by a strike off. When the moisture has disappeared from the surface, the surface shall be steel-trowelled under firm pressure to produce a dense uniform smooth surface free from trowel marks.

The dividing battens shall be removed carefully after 16 hours and the remaining panels shall be completed in the aforesaid manner.

The neat cement application shall be minimum 2mm thick. A little extra thickness shall be allowed for polishing. The floor shall be polished on expiry of at least 7 days after it is laid and a perfectly smooth glazed surface is obtained. The work shall be cured and protected for 21 days on finishing.

17.1.4 Measurement

Artificial Patent Stone Floor including neat cement finishing in place completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

17.1.5 Payment

The amount of completed and accepted works measured as provided above shall be paid for at the Contract unit prices per square meter of Patent Stone Floor as shown in the BOQ. The payment shall be the full compensation for the cost of furnishing, transportation and storage of all materials and equipment, all labours for mixing and laying concrete in alternate panels, compacting, curing and all other works and incidentals necessary to complete the Work as per requirements described under this item of Work, the BOQ, as shown on the Drawings and as directed by the Engineer. The item shall also include all costs of materials, labours and equipment for neat cement finishing for which no separate claim from the Contractor will be entertained.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Patent Stone Floor including neat cement finishing</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
17.2 **Mosaic floor**

17.2.1 **Description**

Works covered under this item shall consist of providing in-situ terrazzo toping of required thickness in accordance with these specifications over an artificial patent stone floor constructed beforehand or on any locations as required in the Bill of Quantities or as shown on the Drawings and/or as directed by the Engineer.

17.2.2 **Materials**

**Marble chips**

Best quality machine crushed marble chips of Chinese, Pakistani or Indian origin as specified.

**White cement**

White cement shall be ‘Onoda’ brand or of an equivalent standard approved by the Engineer.

**Grey cement**

Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C150 Type 1 or BDS 232 or of an equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

**Water**

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

**Glass strips**

Glass dividers shall be of 5mm thickness and 20mm deep.

17.2.3 **Construction methods**

Terrazzo or mosaic floor shall be prepared with best quality Chinese/Pakistani/Indian origin marble chips of requisite grades and colour as required (generally 90% white and 10% coloured), gray cement and white cement. The cement and marble mixture shall be in the proportion of one part marble chips and one part cement or as specified in the Bill of Quantities or as directed by the Engineer. The cement mix shall consist of nine parts white cement and one part gray cement. The requisite quantity of aggregates shall be thoroughly mixed. However, the required proportions of the mixes shall be determined at Site by the Engineer.

White cement shall be ‘Onoda’ brand or an approved equal. Colouring materials shall be of best quality of mineral pigment of high purity and shall be finely grounded, sun-proof and with specific gravity not exceeding 5% by weight of cement used.

The Contractor shall submit three sets of samples of all type of marble chips to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.
The terrazzo work shall be polished by machine and carborundum stone of 80 grit followed by 120 and 200 grits and polishing stone until the finished surface becomes acceptable to the Engineer.

The Contractor shall prepare sample terrazzo work, which must receive the approval of the Engineer. Before such approval is received, no full-scale work shall start. The floor layout shall fully conform to the design as shown on the Drawing or as instructed by the Engineer.

The thickness of terrazzo topping shall be as shown on the Drawing or as specified in the Bill of Quantities or as directed by the Engineer. The setting bed shall be 25mm thick artificial patent stone floor prepared with one part ordinary Portland cement, two parts coarse sand and four parts 12mm down graded coarse aggregate obtained from picked jhama chips/pea gravel/stone chips or in accordance with the specifications of the BOQ.

Terrazzo floors shall be laid over previously roughened and wetted patent stone. The panels for terrazzo floor shall be of the size as indicated in the relevant item of the BOQ, or as directed by the Engineer. Glass dividing strips shall be used, if required under the items of the BOQ.

The sub-floors, over which the terrazzo flooring will be laid, shall be prepared in the same way as have been stated under the Sub-section on ‘Patent Stone Floor’, but it will exclude the portion of the neat cement finishing.

Following the preparatory works, a creamy bonding cement paste, prepared with ordinary Portland cement, shall be applied and scrubbed with stiff bristle brushes on the surface, not exceeding an area of about 0.4 square meter. In general, slurry shall be prepared in a quantity sufficient for an area not over 100 square meter at one time in order to maintain live-glue for bonding.

The terrazzo-topping layer shall then be well trowelled and compacted into the setting bed in the required thickness with desired camber and levels.

A layer of cement paste of same composition as used in topping shall be well trowelled next to leave a smooth surface.

The curing must be done after laying the mosaic work by wet sacking for 7 days.

After terrazzo topping has hardened enough to withstand any dislodgment, polishing works shall commence, which shall be started with first grinding the surface by an approved type of grinding machine with carborundum stone of 80 grits to expose the marble chips. The floors shall be kept wet during grinding. Sweeping and flushing with clean water shall be done to remove all grind-off materials.

Air holes, pits and other blemishes shall than be filled with a thin grout of cement paste of same composition as used in the topping.

On hardening of patch fillers, the floor shall receive successive grinding with carborundum stone upto 240 grit in a final finish. It shall then be cleaned and washed of all surplus materials.

The floor shall be kept undisturbed for a period of 2 weeks on even exposure of marble chips. On expiry of this period the floor shall be cleaned of dirt and dust by rubbing gently with pumic stone (No. 40, No. 80, No. 120) using sufficient water. Stains shall be removed by moistening with Oxalic Acid and rubbing with warm water, if required. No acid solutions shall be used.

The surface shall receive bee wax polishing on drying.
All materials shall be mixed in dry state and shall be protected from harmful effects of moisture. Water shall be added to only such amounts as may be consumed in quantities in less than 30 minutes as required to produce workability. Mixing shall be done on watertight platform.

17.2.4 Measurement

Measurement shall be taken for payment in square meter of finished visible floor surface in place completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

17.2.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which shall constitute the full compensation for furnishing all materials, equipment and labour, including transport, storage and handling of materials, batching, mixing, pouring, compacting and curing of terrazzo, providing glass divider strips, grinding, polishing of terrazzo and all incidentals necessary to complete the Work in all respect as per requirements described under this item of work, the BOQ, as shown on the Drawings and as directed by the Engineer. However, this item shall not include the costs for the item on Artificial Stone Floor.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Cast-in-situ mosaic floor</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
17.3 Non-skid floor tiles

17.3.1 Description
Works covered under this item shall consist of supplying, fitting and fixing approved foreign/local made homogeneous quartz/embossed, non-skid floor tiles or special quality fibrous floor tiles laid on cement mortar base of proportion in accordance with the applicable Drawings, requirements of the BOQ, and these Specifications and/or as directed by the Engineer.

17.3.2 Materials

Tiles
Tiles shall be either homogeneous quartz/embossed non-skid or special quality fibrous. The tiles shall be local/foreign made and of the sizes as shown on the Drawings, described in the Schedule of Works and/or as directed by the Engineer. They should be free from all wrapage blemishes and dimensional defects.

Mortar
Mortar for installation shall consist of 1 part cement and 2 parts sand (FM 1.2). The specification for cement and sand shall conform to those stated under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification and/or as directed by the Engineer.

Grout
All grout for tile joints be prepared with white cement or coloured with inert pigments as and where specified. The specification for white cement shall conform to those stated under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification and/or as directed by the Engineer.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

17.3.3 Construction methods
The Contractor shall submit three sets of samples of all types of tiles to the Engineer for his approval before procuring the materials. One set will be kept in the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

The Contractor shall prepare sample tile work and he should obtain its approval from the Engineer. Before such approval is received, no full-scale work shall start. The design of the floor layout shall conform to what have been shown on the Drawing and/or as directed by the Engineer.

The tiles shall be laid over previously roughened and wetted patent stone floor. The panels shall be of the size as shown on the Drawings and/or as indicated in the relevant item of the BOQ and/or as directed by the Engineer.
The sub-floor on which the tiles will be laid, shall be prepared in the same way as have been stated under the Sub-section on ‘Patent Stone Floor’ of this Section, but it will exclude the portion of neat cement finishing.

The tiles are to be fitted and fixed on the floor on a base of 20mm thick cement mortar prepared with 1 part ordinary Portland cement and 2 parts sand of FM 1.2. The mortar bed shall be cut through horizontally and vertically every 425mm to 600mm.

If the surface needs leveling, a scratch cost of plaster shall be applied, leveled and scratched for key and be allowed to dry out for 12 hours before installing tiles. The setting mortar shall be applied evenly and a neat cement paste to a thickness of about 2mm shall be trowelled to the back of the tiles. The tiles will then be set on firmly tapped into place to ensure full contact. The joints shall be in specified pattern and shall not exceed 2mm in width. The joints shall be raked with grout prepared with white cement and coloured pigment and damp-cured for at least 3 days.

The tiles shall be soaked in water for at least 6 hours before laying. Installation shall be controlled by strings, pages, spacers, levels or other suitable methods so as to ensure their correct laying and uniform leveled joints.

17.3.4 Measurement

Measurement shall be taken for payment in square meter of finished tiled surface in place completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

17.3.5 Payment

The amount of completed and accepted works measured as provided above shall be paid at the Contract unit price per square meter, which shall constitute the full compensation for furnishing all materials, equipment and labour, including transport, storage and handling of materials, cleaning, preparing and laying bed with cement mortar and cutting and laying the tiles with neat cement paste, grouting and curing tiles ranking our joints, high quality finishing and all other works and all incidentals necessary to complete the Work as per requirements described under this item of work, the requirements of the BOQ, as shown on the Drawings and as directed by the Engineer. However, this item shall not include the costs for the item on Artificial Stone Floor.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Non-skid floor tiles</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
SECTION-18
CEMENT PLASTER, POINTING AND WALL TILES
# CEMENT PLASTER, POINTING AND WALL TILES

## CONTENTS

### SECTION-18

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>18.1</td>
<td>12mm thick cement - sand plaster on brick masonry wall</td>
<td>194-196</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.1 Description</strong></td>
<td>194</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.2 Materials</strong></td>
<td>194</td>
</tr>
<tr>
<td></td>
<td>- Cement</td>
<td>194</td>
</tr>
<tr>
<td></td>
<td>- Fine aggregate</td>
<td>194</td>
</tr>
<tr>
<td></td>
<td>- Water</td>
<td>194</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.3 Construction method</strong></td>
<td>194-196</td>
</tr>
<tr>
<td></td>
<td>- Preparation of mortar</td>
<td>194-195</td>
</tr>
<tr>
<td></td>
<td>- Preparation of surface</td>
<td>195</td>
</tr>
<tr>
<td></td>
<td>- Application of plaster</td>
<td>195-196</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.4 Scaffolding</strong></td>
<td>196</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.5 Protection and curing</strong></td>
<td>196</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.6 Measurement</strong></td>
<td>196</td>
</tr>
<tr>
<td></td>
<td><strong>18.1.7 Payment</strong></td>
<td>196</td>
</tr>
<tr>
<td>18.2</td>
<td>6mm thick cement - sand plaster on R.C.C surfaces</td>
<td>197-199</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.1 Description</strong></td>
<td>197</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.2 Materials</strong></td>
<td>197</td>
</tr>
<tr>
<td></td>
<td>- Cement</td>
<td>197</td>
</tr>
<tr>
<td></td>
<td>- Fine aggregate</td>
<td>197</td>
</tr>
<tr>
<td></td>
<td>- Water</td>
<td>197</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.3 Construction methods</strong></td>
<td>197-198</td>
</tr>
<tr>
<td></td>
<td>- Preparation of mortar</td>
<td>197-198</td>
</tr>
<tr>
<td></td>
<td>- Preparation of surface</td>
<td>198</td>
</tr>
<tr>
<td></td>
<td>- Application of plaster</td>
<td>198</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.4 Scaffolding</strong></td>
<td>198</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.5 Protection and curing</strong></td>
<td>198-199</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.6 Measurement</strong></td>
<td>199</td>
</tr>
<tr>
<td></td>
<td><strong>18.2.7 Payment</strong></td>
<td>199</td>
</tr>
<tr>
<td>18.3</td>
<td>12mm thick cement - sand skirting/dado</td>
<td>200-202</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.1 Description</strong></td>
<td>200</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.2 Materials</strong></td>
<td>200</td>
</tr>
<tr>
<td></td>
<td>- Cement</td>
<td>200</td>
</tr>
<tr>
<td></td>
<td>- Fine aggregate</td>
<td>200</td>
</tr>
<tr>
<td></td>
<td>- Water</td>
<td>200</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.3 Construction methods</strong></td>
<td>200-202</td>
</tr>
<tr>
<td></td>
<td>- Preparation of mortar</td>
<td>200-201</td>
</tr>
<tr>
<td></td>
<td>- Preparation of surface</td>
<td>201</td>
</tr>
<tr>
<td></td>
<td>- Application of skirting/dado</td>
<td>201-202</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.4 Protection and curing</strong></td>
<td>202</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.5 Measurement</strong></td>
<td>202</td>
</tr>
<tr>
<td></td>
<td><strong>18.3.6 Payment</strong></td>
<td>202</td>
</tr>
</tbody>
</table>
18.4 Rule pointing on brick masonry wall joints 203-205
   18.4.1 Description 203
   18.4.2 Materials 203
       Cement 203
       Fine aggregate 203
       Water 203
   18.4.3 Construction methods 203-204
       Preparation of mortar 203-204
       Preparation of surface 204
       Making rule points 204
   18.4.4 Scaffolding 204
   18.4.5 Protection and curing 204-205
   18.4.6 Re-pointing of existing brick masonry work joints 205
   18.4.7 Measurement 205
   18.4.8 Payment 205

18.5 Terrazzo skirting 206-208
   18.5.1 Description 206
   18.5.2 Materials 206
       Marble chips 206
       White cement 206
       Grey cement 206
       Water 206
   18.5.3 Construction methods 206-207
   18.5.4 Measurement 208
   18.5.5 Payment 208

18.6 Glazed wall tiles 209-210
   18.6.1 Description 209
   18.6.2 Materials 209
       Glazed ceramic tiles 209
       Mortar 209
       Grout 209
       Water 209
   18.6.3 Construction methods 209-210
   18.6.4 Measurement 210
   18.6.5 Payment 210
SECTION-18
CEMENT PLASTER, POINTING AND WALL TILES

18.1 12mm thick cement - sand plaster on brick masonry wall

18.1.1 Description
This item of work shall consist of making 12mm thick cement plaster on brick masonry wall surfaces and at any other appropriate locations in cement mortar with specified proportion. The Work shall consist of furnishing all materials, its transportation and storage, supply of all labours, tools and equipment and the performance of all other allied works that would be required to complete the Work in all respect. All works shall be carried out in accordance with these specifications and conforming to the levels, dimensions and designs as shown on the Drawings, provisions of the BOQ and/or to carry out the Work following the directions of the Engineer.

18.1.2 Materials

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate
Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of plastering and conform to the requirements of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification. Sand, to be used for plastering work, will be normally of F.M. 1.2 or as directed by the Engineer.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

18.1.3 Construction methods

Preparation of mortar
Unless otherwise specified on the Drawings or in the BOQ or instructed by the Engineer, cement mortar for plaster works on brick masonry walls shall consist by volume of one part Ordinary Portland cement and six parts screened sand of specified F.M. In each mortar, requisite quantity of water shall be added and the components mixed and thoroughly incorporated together to give a workability, appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding mixing
water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform. Cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only to the minimum as would be necessary to produce a workable mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as directed by the Engineer.

Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30 minutes after water is added. The adding of additional water to and re-tempering (cement mortar that stiffened because of evaporation of water), shall be permitted only within 30 minutes from the time of adding water at the time of initial mixing.

Mortar that has taken initial set shall not be used in the work with or without addition of fresh materials.

**Preparation of surface**

Before application of plaster, all joints in brick masonry walls shall be adequately raked out with a hooked tool made for the purpose whilst the mortar is still green and not later than 48 hours of the time of lying and smooth concrete surfaces shall be roughened to provide key. Joints should not be raked out with a trowel or a hammer to avoid the edges of the bricks getting chipped. The brickwork should be brushed down with stiff wire brush so as to remove all loose dust from the joints. Surfaces to be rendered shall also be scrubbed clean of all loose materials and be made free from all dust, grease, etc. and be well wetted with water and kept dampen for 24 hours before applying plaster (the walls should not be soaked but only damped evenly). On old walls it would be required to ensure a good key for the new rendering, to destroy the smooth surface of the brick masonry work with appropriate tools.

**Application of plaster**

Plaster shall consist of two coats when applied over brick masonry i.e. under and finish. The under and finish coats shall be applied without an interval.

The undercoat shall have a minimum thickness of 6mm and shall be leveled with straight edge and scratched for key. The finish coat shall be troweled over with care and leveled with a straight edge to obtain a flat smooth surface. All edges and corners, unless otherwise shown on the Drawings, shall be rounded or chamfered as directed by the Engineer. All moulds shall be neat, clean and true to template.

Plaster shall be floated and troweled to a true and plumbed surface and tested frequently during the progress of the work with a straight edge sufficiently long. There shall be no overlaps or construction joints in single unbroken surface unless the area is over 28 square meter or prior permission is taken from the Engineer for a deviation. Plaster shall be stopped only at corners, construction or expansion joints.

If any crack appears in the plaster or any part sound hollow when tapped, or found to be soft or otherwise defective after the plaster has dried, the defects shall be mended by cutting out and re-plastering at the Contractor's own costs. Such woks should not leave any visible impression on the places mended.

The methods and equipment used for transporting and placing mortar shall be such, as not to damage or delay the use of mixed mortar. All equipment and tools used for mixing or vehicles used for transporting mortar shall be kept clean and free from set mortar, dirt or other deleterious foreign substances.

All plastering works shall be placed only after all brick masonry surfaces have been prepared satisfactorily in accordance with the specifications and the Engineer's instructions.
The plaster shall not be applied during rain sufficiently heavy or prolonged to wash the mortar. Mortar already applied, but becomes diluted by rain, shall be removed and replaced at the expenses of the Contractor before continuing any further works.

18.1.4 Scaffolding

The scaffolding shall be sound and strong enough to withstand all loads likely to be imposed upon it and subject to the Engineer’s approval. Pole, going into the masonry should be at a place, which can be filled with a header brick. The holes, which provide resting space for horizontal members shall not be left in masonry under 1m in width or immediately near the skewbacks of arches. The holes left in the masonry work for supporting the scaffolding shall be filled, made good and to be properly finished with plaster.

18.1.5 Protection and curing

All plasters shall be kept moist throughout the progress of work and protected for a minimum 10 days immediately following completion against harmful effects of weather by suitable covering. During hot weather, all finished or partly completed works shall be covered or wetted in such a manner as will prevent rapid drying of the plaster.

On completion of works, all visible surfaces shall be free from damage or debris and shall look clean. All cares shall be taken so that the plaster surfaces are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.

18.1.6 Measurement

Measurement shall be taken for payment in square meter of the surface of the finished plaster works completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

18.1.7 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter of plaster which price shall constitute full compensation for furnishing all materials including their transportation and storage, mixing of mortar, plastering surfaces and corners, rounding and/or chamfering preparing, cleaning and watering the surfaces to be plastered, watering and protecting the plaster after completion, providing scaffolding and its erection and removal, all other works and all incidentals necessary to complete the Work as per requirements described under this item of work, the BOQ, as shown on the Drawings and/or as directed by the Engineer.

<table>
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<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>12mm thick cement–sand plaster</td>
<td>Square meter</td>
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</table>
18.2 6mm thick cement - sand plaster on R.C.C surfaces

18.2.1 Description

This item of work shall consist of making 6mm thick cement plaster on R.C.C surfaces and at any other appropriate locations in cement mortar with specified proportion. The Work shall consist of supply of all materials, labour, tools and equipment, carriage and the performance of all other allied works. All works shall be carried out in accordance with these specifications and conforming to the levels, dimensions and designs as shown on the Drawings, provisions of the BOQ and/or to carry out the Work following the directions of the Engineer.

18.2.2 Materials

Cement

Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate

Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of plastering and conform to the requirements of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section of the Section on ‘Construction Materials’ of this Specification. Sand, to be used for plastering work, will be normally of F.M. 1.2 or as directed by the Engineer.

Water

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other deleterious substances and shall not contain any visibly solid materials. All requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer.

18.2.3 Construction methods

Preparation of mortar

Unless otherwise specified on the Drawings, or in the BOQ or instructed by the Engineer, cement mortar for plaster works on R.C.C surfaces shall consist of one part Ordinary Portland cement and four parts screened sand of specified F.M. by volume. In each mortar, right quantity of water shall be added and the components mixed and thoroughly incorporated together to give a workability, appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding mixing water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform, Cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only the minimum necessary to produce a workable mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as directed by the Engineer.
Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30 minutes after water is added. The adding of additional water to and re-tempering (cement mortar that stiffened because of evaporation of water), shall be permitted only within 30 minutes from the time of adding water at the time of initial mixing.

Mortar that has taken initial set shall not be used in the work with or without addition of fresh materials.

**Preparation of surface**

Before application of plaster, smooth concrete surfaces shall be roughened to provide key. The surfaces shall be scrubbed clean of all loose materials and soaked with water and kept dampen for 2 hours before plastering. A neat cement coat shall be applied on all concrete surfaces before application of plaster.

**Application of plaster**

Plaster shall consist of a grout application and a finish coat, when applied direct to concrete surface. The under and finish coats shall be applied without an interval.

All edges and corners, unless otherwise shown on the Plans, shall be rounded or chamfered as directed by the Engineer. All moulds shall be neat, clean and true to template.

Plaster shall be floated and troweled to a true surface and tested frequently during the progress of the work with a straight edge sufficiently long. There shall be no overlaps or construction joints in single unbroken surface unless the area is over 28 square meter or prior permission is taken from the Engineer for a deviation. Plaster shall be stopped only at corners, construction or expansion joints.

If any crack appears in the plaster or any part sound hollow when tapped or found to be soft or otherwise defective after the plaster has dried, the defect shall be mended by cutting out and re-plastering at the Contractor’s own costs.

The methods and equipment used for transporting and placing mortar shall be such, as not to damage or delay the use of mixed mortar. All equipment and tools used for mixing or transporting mortar shall be kept clean and free from set mortar, dirt or other deleterious foreign substances.

All plastering works shall be placed only after all R.C.C surfaces have been prepared satisfactorily in accordance with the specifications and the Engineer’s instructions.

The plaster shall not be applied during rain sufficiently heavy or prolonged to wash the mortar when the works are carried out under open sky. Mortar already applied, but becomes diluted by rain, shall be removed and replaced before continuing the work at the expenses of the Contractor.

18.2.4 **Scaffolding**

The scaffolding shall be sound and strong enough to withstand all loads likely to be imposed upon it and subject to the Engineer’s approval. If any place is left out or the plaster gets damaged by resting of poles, the places shall be made plastered or repaired before/on removal of the scaffolding at the expenses of the Contractor.

18.2.5 **Protection and curing**

All plaster shall be kept moist throughout the progress of work and protected for a minimum 10 days immediately following completion against harmful effects of weather by suitable covering when the
location is exposed under the open sky. During hot weather, all finished or partly completed works shall be covered or wetted in such a manner as will prevent rapid drying of the plaster.

On completion of works, all visible surfaces shall be free from damage or debris and shall look clean. All cares shall be taken that the plaster surfaces are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.

18.2.6 Measurement

Measurement shall be taken for payment in square meter of the surface of the finished plaster works completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

18.2.7 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter of plaster which price shall constitute full compensation for furnishing all materials including their transportation and storage, mixing of mortar, plastering surfaces and corners, rounding or chamfering preparing, cleaning and watering the surface to be plastered, watering and protecting the plaster after completion, providing scaffolding and its erection and removal, all other works and all incidentals necessary to complete the Work as per requirements described under this item of work, the Bill of Quantities, as shown on the Drawings and/or as directed by the Engineer.

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18.3 12mm thick cement - sand skirting/dado

18.3.1 Description
This item of work shall consist of providing 12mm thick cement-sand plaster with neat cement finishing in skirting/dado on brick masonry wall surfaces and at any other locations where necessary in cement mortar with specified proportion. The Work shall include supply of all labour, materials, tools and equipment, carriage and the performance of all necessary works. All works shall be carried out in accordance with these specifications and conforming to the levels, dimensions and designs as shown on the Drawings, provisions of the BOQ and/or to carry out the Work following the directions of the Engineer.

18.3.2 Materials

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BS 12 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate
Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of plastering and conform to the requirements of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section(s) of the Section on ‘Construction Materials’ of this Specification. Sand, to be used for plastering work, will be normally of F.M. 1.2 or as directed by the Engineer.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

18.3.3 Construction methods

Preparation of mortar
Unless otherwise specified on the Drawings or in the BOQ or instructed by the Engineer, cement mortar for skirting/dado works on brick masonry walls shall consists by volume of one part Ordinary Portland cement and four parts screened sand of specified F.M.. In each mortar, requisite quantity of water shall be added and the components mixed and thoroughly incorporated together to give a workability, appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding mixing water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform, Cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only the minimum necessary to produce a workable
mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as
directed by the Engineer.

Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30
minutes after addition of water. The adding of additional water to and re-tempering (cement mortar that
stiffened because of evaporation of water), shall be permitted only within 30 minutes from the time of
addition of water at the time of initial mixing.

Mortar that has taken initial set shall not be used in the work with or without addition of fresh materials.

**Preparation of surface**

Before application of skirting/dado, wall plaster, if there be any, shall be removed along the floor to the
required height and the joints in brick-walls shall be adequately raked out to provide key. The surfaces
shall be scrubbed clean of all loose materials and soaked with water and kept dampened for 24 hours
before skirting/dado works start.

**Application of skirting/dado**

Skirting/dado shall consist of two coats i.e under and finish. The under and finish coats shall be applied
without an interval to permit the undercoat to set.

The undercoat shall have a minimum thickness of 6mm and the total built-up thickness will be same as
that of the plaster on the wall. A 3mm groove shall be formed where skirting/dado meets wall plaster.

The skirting/dado shall be installed flushed with the finished wall surface. The intersection with the floor
shall be right-angled and the top of the skirting/dado shall be straight and sharp.

The under-bed shall be laid as uniformly as possible and allowed to become firm before scratching for
key and subsequently allowed to become thoroughly dry before applying the second under-coat. A neat
cement paste of 3mm thickness shall be spread evenly over the second coat and shall be steel troweled
under firm pressure to produce a dense uniform smooth surface free from trowel marks.

The finish coat shall be troweled over with care and leveled with a straight-edge to obtain a flat smooth
surface including neat cement finishing. All edges and corners unless otherwise shown on the Drawings
shall be rounded or chamfered as directed by the Engineer. All moulds shall be neat clean and true to
template.

Skirting/dado shall be floated and troweled to a true and plumbed surface and tested frequently during
the progress of the work with a straight edge sufficiently long. There shall be no overlaps or construction
joints in single unbroken surface unless the area is over 28 square meter or prior permission is taken
from the Engineer. Skirting/dado shall be stopped only at cornsers, sieves, construction or expansion
joints.

If any crack appears in the skirting/dado, or any part sound hollow when tapped or found to be soft or
otherwise defective after the skirting/dado has dried, the defect shall be made good by cutting out and re-
plastering at the Contractor's own costs.

When the skirting/dado is applied on the plinth wall, it shall be plastered up to 150mm below the ground
level.
The methods and equipment used for transporting and placing mortar shall be such, as not to damage or delay the use of mixed mortar. All equipment and tools used for mixing or transporting mortar shall be kept clean and free from set mortar, dirt or other deleterious foreign substances.

All skirting/dado works shall be placed only after all brick-wall surfaces have been prepared satisfactorily in accordance with the specifications and the Engineer’s instructions.

The skirting/dado shall not be applied during rain sufficiently heavy or prolonged to wash the mortar. Mortar already applied, but becomes diluted by rain shall be removed and replaced before continuing the work at the expenses of the Contractor.

18.3.4 Protection and curing

All skirting/dado shall be kept moist throughout the progress of work and protected for a minimum 10 days immediately following completion against harmful effects of weather by suitable covering. During hot weather, all finished or partly completed works shall be covered or wetted in such a manner as will prevent rapid drying of the skirting/dado.

On completion of works, all visible surfaces shall be free from damage or debris and shall look clean. All cares shall be taken that the skirting/dado surfaces are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.

18.3.5 Measurement

Measurement shall be taken for payment in square meter of the surface of the finished skirting/dado works completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

18.3.6 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter of skirting/dado which price shall constitute full compensation for furnishing all materials, mixing of mortar, plastering surfaces and corners, rounding and/or chamfering, preparing, cleaning and watering the surface to be skirted, watering and protecting the skirting/dado after completion, all other works and all incidentals necessary to complete the Work as per requirements described under this item of Work, the BOQ, as shown on the Drawings and/or as directed by the Engineer.

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<tr>
<th>Item of payment</th>
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<tbody>
<tr>
<td>12mm thick skirting/dado</td>
<td>Square meter</td>
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</table>
18.4 Rule pointing on brick masonry wall joints

18.4.1 Description

This item of work shall consist of making rule points in the joints of the brick masonry wall surfaces and at any other locations where necessary in cement mortar with specified proportion. The Work shall include supply of all labour, materials, tools and equipment, carriage and the performance of all necessary works. All works shall be carried out in accordance with these specifications and conforming to the levels, dimensions and designs as shown on the Drawings, provisions of the BOQ and/or to carry out the Work following the directions of the Engineer.

18.4.2 Materials

Cement

Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C 150 Type 1 or BS 12 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Fine aggregate

Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of rule pointing and conform to the requirements of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section(s) of the Section on ‘Construction Materials’ of this Specification. Sand, to be used for plastering work, will be normally of F.M. 1.2 or as directed by the Engineer.

Water

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

18.4.3 Construction methods

Preparation of mortar

Unless otherwise specified on the Drawings or in the BOQ or instructed by the Engineer, cement mortar for rule pointing works on brick masonry wall joints shall consists by volume of one part Ordinary Portland cement and two parts screened sand unless otherwise required by the Drawings or instructed by the Engineer. In each mortar, requisite quantity of water shall be added and the components mixed and thoroughly incorporated together to give a workability, appropriate to its use. Mortar shall be used whilst freshly mixed and no softening or re-tampering will be allowed.

Mortar shall be mixed in an approved mechanical mixer unless hand-mixing is specifically permitted by the Engineer and in a manner as to accurately determine and control the quantity of each ingredient in the mortar. The cement and sand shall be first mixed dry until thoroughly mixed before adding mixing water. If hand mixing is permitted, the operation shall be carried out on a clean watertight platform. Cement and sand shall be first mixed dry in the required proportion to obtain a uniform colour of the mixture. Water shall then be added sparingly, only the minimum necessary to produce a workable mixture of normal consistency. The water cement ratio in no case shall exceed 0.50 by weight, or as directed by the Engineer.
Only a sufficient quantity of sand and cement shall be mixed with water that can be used within 30 minutes after addition of water. The adding of additional water to and re-tempering (cement mortar that stiffened because of evaporation of water), shall be permitted only within 30 minutes from the time of addition of water at the time of initial mixing.

Mortar that has taken initial set shall not be used in the work with or without addition of fresh materials.

**Preparation of surface**

Before rule pointing, the joints in brick-walls shall be adequately roughened. The surfaces shall be scrubbed clean of all loose materials and soaked with water and kept damp for 24 hours.

**Making rule points**

Unless otherwise specified, mortar for rule pointing shall be prepared with one part of Portland cement and two parts of sand generally of F.M. 1.2. Lime in powder form passing 100 mesh in the proportion of 2% by weight of cement shall also constitute an ingredient of the mortar.

The methods and equipment used for transporting and placing mortar shall be such, as not to damage or delay the use of mixed mortar. All equipment and tools used for mixing or transporting mortar shall be kept clean and free from set mortar, dirt or other deleterious foreign substances.

When all brick wall surfaces including the joints are well prepared as described earlier, mortar of specified proportion shall be applied at the joints and finished in rule or concave pointing as mentioned in the 'BOQ' or indicated on the Drawings or directed by the Engineer. The concave pointing shall be done with the help of rebars wooden template to prepare semi-circular pointing intruding inside the brick wall joints. The Contractor shall remain very careful in maintaining the type of pointing as asked for in the Contract.

All rule-pointing works shall only start when all brick-wall surfaces have been prepared satisfactorily in accordance with the specifications and the Engineer's instructions.

The rule pointing works shall not be undertaken during rain sufficiently heavy or prolonged to wash the mortar. Mortar already applied, which becomes diluted by rain shall be removed and replaced before continuing the work at the expenses of the Contractor.

**18.4.4 Scaffolding**

The scaffolding shall be sound and strong enough to withstand all loads likely to be imposed upon it and subject to the Engineer’s approval. Pole, going into the masonry should be at a place, which can be filled with a header brick. The holes, which provide resting, space for horizontal members shall not be left in masonry under 1m in width or immediately near the skewbacks of arches. The holes left in the masonry work for supporting the scaffolding shall be filled and made good.

**18.4.5 Protection and curing**

All works shall be kept moist throughout the progress of work and protected for minimum 10 days immediately following completion against harmful effects of weather by suitable covering. During hot weather, all finished or partly completed works shall be covered or wetted in such a manner as will prevent rapid drying of the plaster.
On completion of works, all visible surfaces shall be free from damage or debris and shall look clean. All cares shall be taken that the plaster surfaces are not stained or coated as the work proceeds. No rubbing of the faces to remove coating shall be allowed.

### 18.4.6 Re-pointing of existing brick masonry work joints

The extent of re-pointing of existing brick masonry works shall be jointly surveyed by the Contractor and the Engineer at the start of the work and the location of all repairs needed shall be recorded and permanently marked in paint.

The defective mortar shall be carefully removed from the joints and the joints shall be cleaned immediately prior to re-pointing. The re-pointing shall be done with cement mortar of specified proportion to full depth, penetration and trimmed flush with the face of the brick masonry works.

Cracks in the existing brick masonry works shall be treated in the same way. Defective materials shall be carefully removed and the cracks shall be filled with cement mortar of specified proportion.

### 18.4.7 Measurement

Measurement shall be taken for payment in square meter of the surface of the brick masonry works with all joints have been finished by rule pointing in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 18.4.8 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter of brick masonry wall which price shall constitute full compensation for furnishing all materials, mixing of mortar, rule pointing of joints, cleaning and watering the surface to be rule pointed, watering and protecting the work after completion, providing scaffolding and its erection and removal, all other works and all incidentals necessary to complete the Work as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Rule pointing of joints of the brick masonry wall</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
18.5 Terrazzo Skirting

18.5.1 Description
Works covered under this item shall consist of providing 10mm or 12mm thick in-situ terrazzo skirting over a cement sand mortar under-bed on walls or other locations as required in the BOQ and in accordance with these Specifications and/or as per the direction of the Engineer.

18.5.2 Materials

Marble chips
Best quality machine crushed marble chips of Chinese, Pakistani or Indian origin as specified.

White cement
White cement shall be ‘Onoda’ brand or of an equivalent standard approved by the Engineer.

Grey cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C150 Type 1 or BS 12 or BDS 232 or of an equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid materials. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer. All other requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

18.5.3 Construction methods
Terrazzo or mosaic floor shall be prepared with best quality Chinese/Pakistani/Indian origin marble chips of No. 2 size and colour as required (generally 90% white and 10% coloured), gray cement and white cement. The cement and marble mixture shall be in the proportion of one part marble chips and one part cement as specified in the BOQ or as directed by the Engineer. The cement mix shall consist of nine parts white cement and one part gray cement. The marble chips and the cement mix shall be thoroughly mixed in specified proportion. However, the required proportion of the mixes shall be determined at Site by the Engineer.

White cement shall be ‘Onoda’ brand or an approved equal. Colouring materials shall be of best quality of mineral pigment of high purity and shall be finely grounded, sun-proof and with specific gravity not exceeding 5% by weight of cement used.

The Contractor shall submit three sets of samples of all type of marble chips to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

The terrazzo work shall be polished by machine and carborundum stone of 80 grit followed by 120 and 200 grit and polishing stone until the finish becomes acceptable to the Engineer.
The Contractor shall prepare sample terrazzo work, which must receive the approval of the Engineer. Before such approval is received, no full-scale work shall start.

The thickness of terrazzo topping shall be at least 20mm. The setting bed shall be minimum 12mm thick using Portland cement and sand in proportion as mentioned in the BOQ.

The surface receiving situ terrazzo work shall be cleaned thoroughly and if necessary the joints will be racked. The surface shall be scarified properly. It shall then be moistened with water adequately to make the surface damp.

Following preparatory work, the setting bed as specified above shall be applied in such away that it provides a plumb and true surface, which will be scratched with broom and allowed to be cured for at least 24 hours.

A coat of bonding paste of neat cement (Portland Cement) shall then be trowelled on the setting bed. The setting bed shall be moistened before application of bonding slurry in such a way that it is damp only and not glister-wet.

The next step is to well trowell the terrazzo topping layer in 20mm thickness over the live bonding paste. The ingredients of terrazzo topping layer shall be mixed in proportion and in manner as specified under the Sub-section on ‘Mosaic Floor’ and approved by the Engineer.

Layer of same cement paste of same composition as used in topping shall be well trowelled next leaving a smooth surface.

The curing must be done after laying the terrazzo work by wet sacking.

After terrazzo topping has hardened enough to withstand any dislodgment, polishing works shall commence, which shall be first started with grinding the surface by an approved type of grinding machine with carborundum stone of 80 grits to expose the marble chips. The wall shall be kept wet during grinding. Sweeping and flushing with clean water shall be done to remove all grind-off materials.

Air holes, pits and other blemishes shall than be filled with a thin grout of cement paste of same composition as used in topping.

On hardening of patch fillers, the wall shall receive a successive grinding with carborundum stone up to 240 grit in a final finish. It shall then be cleaned and washed of all surplus materials.

The wall shall be kept undisturbed for a period of 2 weeks on even exposure of marble chips. On expiry of this period the wall shall be cleaned of dirt and dust by rubbing gently with pumice stone (No. 40, No. 80 and No. 120) using sufficient water. Stains shall be removed by moistening with Oxalic Acid and rubbing with warm water, if required. No acid solutions shall be used.

The surface shall receive bee wax polishing on drying.

All materials shall be mixed in dry state and shall be protected from harmful effects of moisture. Water shall be added to only such amounts as may be consumed in less than 30 minutes, in quantities required to produce workability. Mixing shall be done on watertight platform.
18.5.4 Measurement

Measurement shall be taken for payment in square meter of the finished terrazzo skirting in place completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

18.5.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which shall constitute the full compensation for furnishing all materials, equipment and labour, including transport, storage and handling of materials, mixing, placing, compacting, curing, grinding, polishing and all other works and all incidentals as would be necessary to fully complete the Work in all respect as per requirements described under this item of work, the BOQ, as shown on the Drawings and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Terrazzo skirting</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
18.6 Glazed wall tiles

18.6.1 Description

Works covered under this item shall consist of supplying, fitting and fixing approved glazed ceramic tiles laid on walls or any other places as specified on a cement mortar base of proportion in accordance with the applicable Drawings and these Specifications. The tiles may be of local or foreign origin as would be specified in the BOQ.

18.6.2 Materials

Glazed ceramic tiles

Glazed ceramic tiles, unless otherwise specified, shall be of the standard size of 150mm x 150mm x 6mm or as approved by the Engineer, white or coloured, free from war-page, blemishes and dimensional defects and conforming to the standards of Federal Specification SS-T-308p. In case of foreign made tiles, the places of origin are normally Srilanka/Thailand/England/Italy.

Mortar

Mortar for installation shall consist of 1 part cement and 3 parts sand (FM 1.2). The specification for cement and sand shall conform to those stated under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification and/or as directed by the Engineer.

Grout

All grout for tile joints shall be prepared with white cement, coloured cement and with inert pigments, where specified. The specification for white cement shall conform to those stated under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification and/or as directed by the Engineer.

Water

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid materials. The Contractor shall get the water tested by comparing with water of known satisfactory quality, if requested by the Engineer. All other requirements shall be similar to what have been stated under the relevant Sub-section of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

18.6.3 Construction methods

The Contractor shall submit three sets of samples of all types of tiles to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

The Contractor shall prepare a sample work, which must receive the approval of the Engineer. Before such approval is received, no full-scale work for tile setting shall start.

Preparation of wall surface and application of mortar bed shall comply with the provisions of the item on ‘Terrazzo Skirting’ as stated in the preceding Sub-section of this Specification.
The tiles are to be fitted and fixed on wall on a base of 20mm thick cement mortar prepared with 1 part ordinary Portland cement and 3 parts sand of FM 1.2. The mortar bed shall be cut through horizontally and vertically every 425mm to 600mm.

If the surface needs leveling, a scratch cost of plaster shall be applied, leveled and scratched for key and be allowed to dry out for 12 hours before installing tiles. The setting mortar shall be applied evenly and a neat cement paste to a thickness of about 2mm shall be trowelled to the back of the tiles and the tiles to be set on firmly tapped in to place to ensure full contact. The joints shall be in specified pattern and shall not exceed 2mm in width.

The tiles shall be soaked in water for at least 6 hours before setting. Installation shall be controlled by strings, pages, spacers, levels or other suitable methods that will ensure correct layout and uniform leveled joints.

The joints shall be grouted with white cement, cleaned and damp-cured for at least 3 days.

18.6.4 Measurement

Measurement shall be taken for payment in square meter of the finished tiled surface completed in accordance with the Specifications stated herein and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

18.6.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which shall constitute the full compensation for furnishing all materials, equipment and labour, including transport, storage and handling of materials, cleaning, preparing and laying bed and cutting and laying the tiles, grouting, curing and all other works and all incidentals as would be necessary to fully complete the Work in all respect as per requirements described under this item of work, the BOQ, as shown on the Drawings and/or as directed by the Engineer.

<table>
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<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
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<td>Glazed wall tiles</td>
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SECTION-19
LIME TERRACING AND
DAMP PROOF COURSE
# LIME TERRACING AND DAMP PROOF COURSE

## CONTENTS

### SECTION-19

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Materials</th>
<th>Construction methods</th>
<th>Measurement</th>
<th>Payment</th>
</tr>
</thead>
<tbody>
<tr>
<td>19.1</td>
<td>Lime terracing</td>
<td>Lime, Surki, Brick chips, Water</td>
<td>Mixing, Installation</td>
<td>211-212</td>
<td>212</td>
</tr>
<tr>
<td>19.1.1</td>
<td>Description</td>
<td>Lime</td>
<td></td>
<td>211</td>
<td></td>
</tr>
<tr>
<td>19.1.2</td>
<td>Materials</td>
<td>Surki</td>
<td></td>
<td>211</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>Brick chips</td>
<td></td>
<td>211</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>Water</td>
<td></td>
<td>211</td>
<td></td>
</tr>
<tr>
<td>19.1.3</td>
<td>Construction methods</td>
<td>Mixing</td>
<td></td>
<td>211</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>Installation</td>
<td></td>
<td>212</td>
<td></td>
</tr>
<tr>
<td>19.1.4</td>
<td>Measurement</td>
<td></td>
<td></td>
<td>212</td>
<td></td>
</tr>
<tr>
<td>19.1.5</td>
<td>Payment</td>
<td></td>
<td></td>
<td>213</td>
<td></td>
</tr>
<tr>
<td>19.2</td>
<td>Damp proof course (DPC)</td>
<td>Cement, Coarse aggregate, Fine aggregate, Water, Asphalt</td>
<td></td>
<td>214-215</td>
<td></td>
</tr>
<tr>
<td>19.2.1</td>
<td>Description</td>
<td>Cement</td>
<td></td>
<td>214</td>
<td></td>
</tr>
<tr>
<td>19.2.2</td>
<td>Materials</td>
<td>Coarse aggregate</td>
<td></td>
<td>214</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>Fine aggregate</td>
<td></td>
<td>214</td>
<td></td>
</tr>
<tr>
<td></td>
<td></td>
<td>Water</td>
<td></td>
<td>214</td>
<td></td>
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<tr>
<td></td>
<td></td>
<td>Asphalt</td>
<td></td>
<td>214</td>
<td></td>
</tr>
<tr>
<td>19.2.3</td>
<td>Construction methods</td>
<td></td>
<td></td>
<td>214-215</td>
<td></td>
</tr>
<tr>
<td>19.2.4</td>
<td>Measurement</td>
<td></td>
<td></td>
<td>215</td>
<td></td>
</tr>
<tr>
<td>19.2.5</td>
<td>Payment</td>
<td></td>
<td></td>
<td>215</td>
<td></td>
</tr>
</tbody>
</table>
SECTION-19
LIME TERRACING AND DAMP PROOF COURSE

19.1 Lime Terracing

19.1.1 Description

This item of work shall consist of placing a waterproofing and drainage course on roof slabs by providing lime concrete of specified thickness and proportion in accordance with the requirements as shown on the Drawings, as stated in the BOQ and these Specifications or and as directed by the Engineer.

19.1.2 Materials

Lime

Lime to be used for lime terracing shall be slaked lime and shall conform to the requirements of ASTM C 5 and ASTM C 207.

Lime shall not contain more than 5% of impurities. It shall dissolve in soft water when water is added in sufficient quantity. Stone lime available in Sylhet, meeting the above requirements, may be used.

A good hydraulic lime shall have an ultimate tensile strength of at least 7kg/cm² for a mortar prepared with 1 part of lime and 3 parts of sand. A pure surki mortar gives a breaking strength of about 5.6 to 6.3 kg/cm² when left in dry air and 21 to 25 kg/cm² when left immersed under water.

Surki

Surki shall be made only by grinding 1st class well-burnt (but not vitrified) bricks or brickbats. Surki shall not be made from under-burnt or over-burnt bricks nor bricks containing high proportion of sand. Surki shall be perfectly clean and free from dust, sand or any other particles and shall be ground to such a fineness as would pass through 3.35mm sieve with at least 50% of it passing through 1.7mm sieve.

Brick chips

Brick chips (khoa) shall consist of 20mm down-graded angular fragments of broken or crushed well burnt (but not vitrified) 1st class bricks. Porous brick chips or showing signs of salt petre shall not be used for lime terracing.

Water

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

19.1.3 Construction methods

Mixing

The approximate proportion of the mixture shall be 2 parts lime to 2 parts surki to 7 parts brick chips, all having specifications as stated above and as stated under the relevant Sub-sections of the Section on ‘Construction Materials’ of this Specification.
The mixing shall be done on a clean platform. The lime and surki, in specified quantities, shall be first mixed dry till it takes a uniform colour. The mixture then shall be laid on the top of stack of specified quantity of previously wetted brick chips. The whole shall be turned over once without adding water and twice by gradually adding small quantities of water for tempering itself for 24 hours. The mix shall then be allowed to age for at least 7 days. During this period the mixture shall be turned by spading twice a day by adding further lime water to prevent drying up. The lime water shall be prepared with 1 part lime and 10 parts water. The process shall be repeated throughout the period of aging until the mix attains the desirable consistency.

The mixing shall be done invariably on the ground on a brick platform at the designated place.

The mixing shall not be allowed on the roof deck whatever may the case arises.

Installation

The roof slab, on which the mixture will be laid, shall be cleaned and washed accompanied by scrubbing, if necessary. The prepared mixture shall then be laid evenly on the cleaned roof slab to proper slope and grade in thickness 25mm more than the required compacted thickness as shown on the Drawings, as required by the Bill of Quantities or as directed by the Engineer.

Beating of the laid mixture shall then be undertaken which shall be done by two rows of workers sitting in rows that will traverse the length of the roof backward and forward beating with wooden mallets. Beating shall continue until the mixture has almost set and the mallets rebound from the surface. Beating shall usually be continued for 5 or 6 days. Before beating starts, a lime slurry shall be sprinkled on the top and allowed to soak well. The lime slurry shall be continually sprinkled on the concrete to keep it wet while being beaten. The surface shall never be allowed to dry. The mortar, which comes to the surface during the beating is to be rendered smooth and finished off with a grout prepared with 1 part lime and 2 parts surki, but no plaster shall be given to the surface.

The work shall be cured for 3 weeks by covering with a 50mm layer of moist earth mixed with 3% straw or hay. This layer shall be moistened from time to time as required. On expiry of the curing period, the layer of earth shall be removed and the entire roof area shall be swept clean.

Good care shall be taken not to clog roof drains.

Where lime concrete roofing cannot be placed all in one day, each day's work shall be terminated on a straight line with 1:2 slope. Joining of new work to previous day's work shall be accomplished by applying a bonding paste of lime-surki mortar to the slope before placing the new lime concrete. The composition of the lime-surki mortar shall be 1 part of lime and 1 part of surki mixed with requisite quantity of water.

Turn-up along parapet shall be provided as per the requirements of the Drawings and finished in a manner similar to roof.

19.1.4 Measurement

Measurement shall be taken for payment in cubic meter of volume actually built by computing the actual area multiplied by the average thickness of the lime concrete installed in position as per specifications of this item and/or as per the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.
19.1.5 Payment

The amount of completed and accepted work as measured above shall be paid for at the Contract unit price per cubic meter. The payment shall constitute the full compensation for furnishing all materials, equipment and labour including storage, handling and transport of all materials, slaking of lime, making platform, mixing, laying and consolidating of lime concrete, making “ghoondi” neat finishing with lime mortar and all incidentals necessary to complete the Work as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Finished lime terracing on the roof slab</td>
<td>Cubic meter</td>
</tr>
</tbody>
</table>
19.2 Damp proof course (DPC)

19.2.1 Description
Works covered under this item shall consist of constructing a layer of cement concrete with specified proportion and in required thickness with top surface painted with Asphalt/Coal tar.

19.2.2 Materials

Cement
Cement used in the works shall be ordinary Portland cement complying with the requirements of ASTM C150 Type 1 or BS 12 or BDS 232 or equivalent standard and those stated under the Section on ‘Construction Materials’ of this Specification.

Coarse aggregate
Coarse aggregate shall conform to the requirements of ASTM C 330.

Coarse aggregate shall be hard, durable, clean, free from dust and other deleterious material to be obtained by crushing 1st class/picked jhama bricks. The grading of the coarse aggregate shall be such that when combined with the approved fine aggregate and cement, it shall produce a workable concrete of maximum density which has been considered to be 10mm down graded in this case.

Materials shall also conform to the requirements specified in the relevant Sub-section of the Section titled ‘Construction Materials’ of this Specification.

Fine aggregate
Fine aggregates shall be non-saline clean natural sand and have a specific gravity not less than 2.6, a Fineness Modulus not less than what will be specified for a particular type of DPC and conform to the requirement of ASTM C 33 or BDS 243 and those stated under the relevant Sub-section(s) of the Section on ‘Construction Materials’ of this Specification. Sand, to be used for Damp Proof Course, will be of FM normally not below 1.8 or as directed by the Engineer.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

Asphalt
Asphalt shall conform to the requirements of ASTM D 312. Type-1 shall be used below ground and Type-2 shall be used above ground.

19.2.3 Construction methods
25mm to below 40mm thick cement concrete mixture prepared with 1 part cement, 2 parts sand and 4 parts brick chips is to be installed following the procedures stated under the Section on ‘Concrete Work’ of this Specification. In case of DPC designed with a 40mm thickness, the cement concrete mixture may be prepared with 1 part cement, 1½ parts sand and 3 parts brick chips. Two coats of hot asphalt should be applied over the cement concrete when the concrete has been fully cured and dried. The surface to
be damp-proofed shall be primed and thoroughly mopped with asphalt. When the first mopping of asphalt has set sufficiently, the entire surface shall be mopped with second coating of hot asphalt. Special care shall be taken to see that there are no skips in the coatings and that all surfaces are thoroughly covered. The asphalt used should not melt or soften in the hottest days and should not get squeezed due to pressure of the masonry over it.

All concrete surfaces, which are to be damp-proofed shall be reasonably smooth and free from foreign material that would prevent bond. The surface shall be dry and immediately before the application of the primer, the surface shall be thoroughly cleaned of dust and loose materials.

The damp-proof course should be laid flush with the floor surface and should not be carried across doorways or other openings. The upper layer of cement concrete floors should be continued over such openings and should be laid at the same time as the floors. The asphalt or tar layer should be laid under the concrete at the openings. Where concrete is laid on bitumen or tar, the surface of the bitumen or tar must be sprinkled with dry sand.

The position of the damp proof course is also an important factor and it should be laid at such a height that it is above the normal level to which water splashes from the ground when it is raining. A damp proof course should not be less than 15cm above the highest level of the ground.

19.2.4 Measurement

Damp proofing shall be measured in square meter of the works completed in place, in accordance with the Specifications stated herein and/or with the provisions of the Bill of Quantities and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

19.2.5 Payment

The amount of completed and accepted work as measured above shall be paid for at the Contract unit price per square meter. The payment shall constitute the full compensation for the cost of furnishing all equipment, materials, labour for preparation of concrete mixture and its casting, compacting, curing, including, asphalt painting including all storage, handling and transport and all incidentals necessary for the satisfactory completion of the damp-proofing as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Damp-proofing</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
SECTION-20
PAINTING AND WHITEWASH
### Contents

#### Section 20

<table>
<thead>
<tr>
<th>Subsection</th>
<th>Description</th>
<th>Pages</th>
</tr>
</thead>
<tbody>
<tr>
<td>20.1</td>
<td>Synthetic enamel paint</td>
<td>216-219</td>
</tr>
<tr>
<td>20.1.1</td>
<td>Description</td>
<td>216</td>
</tr>
<tr>
<td>20.1.2</td>
<td>Materials</td>
<td>216</td>
</tr>
<tr>
<td>20.1.3</td>
<td>Construction methods</td>
<td>216-219</td>
</tr>
<tr>
<td>20.1.3.1</td>
<td>General</td>
<td>216</td>
</tr>
<tr>
<td>20.1.3.2</td>
<td>Colours and samples</td>
<td>216-217</td>
</tr>
<tr>
<td>20.1.3.3</td>
<td>Protection</td>
<td>217</td>
</tr>
<tr>
<td>20.1.3.4</td>
<td>Surface preparation</td>
<td>217</td>
</tr>
<tr>
<td>20.1.3.5</td>
<td>For metallic surface</td>
<td>217</td>
</tr>
<tr>
<td>20.1.3.6</td>
<td>For wooden surface</td>
<td>217</td>
</tr>
<tr>
<td>20.1.3.7</td>
<td>Application of paint</td>
<td>217-218</td>
</tr>
<tr>
<td>20.1.3.8</td>
<td>Wooden and ferrous metallic surfaces</td>
<td>217-218</td>
</tr>
<tr>
<td>20.1.3.9</td>
<td>Galvanized iron surfaces</td>
<td>218</td>
</tr>
<tr>
<td>20.1.3.10</td>
<td>Completion</td>
<td>218-219</td>
</tr>
<tr>
<td>20.1.4</td>
<td>Measurement</td>
<td>219</td>
</tr>
<tr>
<td>20.1.5</td>
<td>Payment</td>
<td>219</td>
</tr>
<tr>
<td>20.2</td>
<td>Plastic painting</td>
<td>220-222</td>
</tr>
<tr>
<td>20.2.1</td>
<td>Description</td>
<td>220</td>
</tr>
<tr>
<td>20.2.2</td>
<td>Materials</td>
<td>220</td>
</tr>
<tr>
<td>20.2.3</td>
<td>Construction methods</td>
<td>220-222</td>
</tr>
<tr>
<td>20.2.3.1</td>
<td>General</td>
<td>220</td>
</tr>
<tr>
<td>20.2.3.2</td>
<td>Colours and samples</td>
<td>220</td>
</tr>
<tr>
<td>20.2.3.3</td>
<td>Protection</td>
<td>221</td>
</tr>
<tr>
<td>20.2.3.4</td>
<td>Surface preparation</td>
<td>221</td>
</tr>
<tr>
<td>20.2.3.5</td>
<td>Application of paint</td>
<td>221-222</td>
</tr>
<tr>
<td>20.2.3.6</td>
<td>Completion</td>
<td>222</td>
</tr>
<tr>
<td>20.2.4</td>
<td>Measurement</td>
<td>222</td>
</tr>
<tr>
<td>20.2.5</td>
<td>Payment</td>
<td>222</td>
</tr>
<tr>
<td>20.3</td>
<td>Distempering</td>
<td>223-225</td>
</tr>
<tr>
<td>20.3.1</td>
<td>Description</td>
<td>223</td>
</tr>
<tr>
<td>20.3.2</td>
<td>Materials</td>
<td>223</td>
</tr>
<tr>
<td>20.3.3</td>
<td>Construction methods</td>
<td>223-225</td>
</tr>
<tr>
<td>20.3.3.1</td>
<td>General</td>
<td>223</td>
</tr>
<tr>
<td>20.3.3.2</td>
<td>Colours and samples</td>
<td>223</td>
</tr>
<tr>
<td>20.3.3.3</td>
<td>Protection</td>
<td>224</td>
</tr>
<tr>
<td>20.3.3.4</td>
<td>Surface preparation</td>
<td>224</td>
</tr>
<tr>
<td>20.3.3.5</td>
<td>Application of distemper</td>
<td>224-225</td>
</tr>
<tr>
<td>20.3.3.6</td>
<td>Completion</td>
<td>225</td>
</tr>
<tr>
<td>20.3.4</td>
<td>Measurement</td>
<td>225</td>
</tr>
<tr>
<td>20.3.5</td>
<td>Payment</td>
<td>225</td>
</tr>
</tbody>
</table>
### 20.4 White washing

**20.4.1 Description**

226

**20.4.2 Materials**

- Lime stone 226
- Shell lime 226
- Gum arabic 226
- Robin blue 226
- Water 226

**20.4.3 Construction methods**

- Samples 226
- Protection 227
- Surface preparation 227
- Preparation of white wash 227
- Application of white wash 227-228
- Completion 228-229

**20.4.4 Measurement**

229

**20.4.5 Payment**

229

### 20.5 Colour washing

**20.5.1 Description**

230

**20.5.2 Materials**

- Lime stone 230
- Shell lime 230
- Gum arabic 230
- Colour 230
- Water 230

**20.5.3 Construction methods**

- Samples 230
- Protection 230
- Surface preparation 231
- Preparation of colour wash 231
- Application of colour wash 231-232
- Completion 232-233

**20.5.4 Measurement**

233

**20.5.5 Payment**

233

### 20.6 Coloured cement painting

**20.6.1 Description**

234

**20.6.2 Materials**

- Cement powder 234
- Water 234

**20.6.3 Construction methods**

- General 234
- Samples 234
- Protection 234
- Surface preparation 234-235
- Preparation of pain 235
- Application of paint 235-236
- Completion 236
20.6.4 Measurement 236
20.6.5 Payment 236

20.7 Water repellent painting 237-239
20.7.1 Description 237
20.7.2 Materials 237
20.7.3 Construction methods 237-238
   General 237
   Samples 237
   Protection 237
   Surface preparation 237
   Application of silicon water repellent paint 237-238
   Completion 238
20.7.4 Measurement 238
20.7.5 Payment 239

20.8 Varnishing 240-242
20.8.1 Description 240
20.8.2 Materials 240
   Oil varnish 240
   Storage of materials 240
20.8.3 Construction Methods 240-242
20.8.3.1 Samples 240
20.8.3.2 Surface preparation 240-241
20.8.3.3 Application 241-242
   Type of finish 241
   Finishing materials 241
   Application method 241-242
   Completion 242
20.8.4 Measurement 242
20.8.5 Payment 242

20.9 French polishing 243-244
20.9.1 Description 243
20.9.2 Materials 243
   Spirit varnish 243
   Storage of materials 243
20.9.3 Construction methods 243-244
   Samples 243
   Surface preparation 243
   Application 243-244
   Completion 244
20.9.4 Measurement 244
20.9.5 Payment 244
SECTION-20
PAINTING AND WHITEWASH

20.1 Synthetic enamel paint

20.1.1 Description
Works covered under this item shall consist of painting metallic surface or wooden surface where water
proofing is required or the places as directed by the Engineer. Synthetic enamel paint of any approved
brand and colour shall be used in minimum 3 coats in accordance with these specifications, as per the
provisions in the BOQ and/or as directed by the Engineer or as specified by the manufacturers. Painting
shall be done in a manner to obtain an even, smooth finish of confirm shade and without any mark of
brush and joint.

20.1.2 Materials
Paint shall consist of ready-mixed synthetic enamel of approved type supplied in original sealed
containers bearing the name of the manufacturer of the paint.

All painting materials shall be of the best quality and be delivered at the Site in sealed original containers
bearing manufacturer’s labels and seals.

Materials to be used in the work shall conform to the reputed manufacturer’s specifications and to the
satisfaction of the Engineer.

Storage of materials
Materials and tools shall be stored in a single place at the Site as designated by the Engineer.

Storage area shall be maintained in a neat and clean condition with surroundings protected from
damage.

Inflammable materials shall be stored in sealed containers. Waste shall be removed from the premises at
the end of each day’s work. Every precaution shall be taken to prevent fire.

Storage area shall be all time accessible to the Engineer.

20.1.3 Construction methods

General
Before purchasing materials, the Contractor shall submit to the Engineer a list showing the brand and
type of paints proposed for this item of work. Manufacturer’s catalogue or specification sheets, in
triplicate, for materials selected shall be submitted to the Engineer with the list of brands and types. No
material shall be used without the approval of the Engineer.

Colours and samples
Colour scheme shall conform the Finish Schedule and as directed by the Engineer. All tinting and
matching shall be to the satisfaction of the Engineer.
For all painted finishes, samples shall be prepared as per direction of the Engineer on pieces of the same kind of material surface at least on an area of 150mm by 300mm. The finished sample shall be approved by the Engineer.

**Protection**

Drop cloths or other approved protection materials shall be furnished and laid on all areas where painting and finishing is being done so as to adequately protect floor and other places from all damages caused during the execution of the painting work.

**Surface preparation**

**For metallic surface**

All metallic surfaces shall be prepared before application of paint.

For ferrous metal, the surfaces shall be cleaned by brushing with wire brush or sand paper to remove all rust, weld spatter an other foreign particles. Any grease and oil film shall be removed with a solvent, using a fine steel wood pad or a coarse cloth. All damages to shop coat caused by erection, repairing and cleaning shall be spot primed with the same materials used for the shop coat.

In case of galvanized metal, the surfaces shall be cleaned and dried. Any grease and oil film shall also be removed with a solvent, using a fine steel wood pad or a coarse cloth. It is considered that paint will adhere to galvanized iron if the surface is washed with vinegar or slaked lime and washing soda before painting.

In all cases manufacturer’s instructions are to be strictly followed in preparing the surfaces to be painted.

**For wooden surface**

Wood, the surfaces, which are to be painted, shall be well seasoned and the surface to be painted shall be perfectly dry.

The surfaces of woodwork to be painted or polished should be rubbed down perfectly smooth with medium and fine grade sandpaper. All rubbing to be done with the grain. Worked timber should be primed as soon as possible particularly on the cut end grain. New woodwork shall be knotted, primed and stopped before given coats of paint.

**Application of paint**

**Wooden and ferrous metallic surfaces**

The workmanship for painting shall be of high quality and experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before painting or finishing is started.

All paint shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. Paint shall be worked into all corners and crevices.

Materials shall be applied in strict accordance with the manufacturer’s directions. In particular, no prepared paint shall be thinned by any methods except as directed by the manufacturer. All paint shall be thoroughly mixed before being applied.
Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.

No exterior painting shall be done in rainy and damp weather until the surface is thoroughly dry. No interior painting shall be done on damp surfaces.

Drying time for every coat shall not be less than 72 hours and 48 hours for exterior paints and interior paints respectively. Each coat shall be thoroughly dry before application of subsequent coat.

All natural finished woodwork, painted woodwork and painted metal shall be slightly sanded between coats using No.'00’ sandpaper. The finished surface must be smooth, evenly leveled and free from brush marks.

Natural finished woodwork only shall be rubbed with fine sandpaper after the last coat has received the desired finish as per approved sample.

All woodwork for natural finish shall be sealed on the back and all surfaces, which will be concealed after erection with two coats of an approved transparent sealer prior to installation.

After being fitted by the Carpenter, all edges of the doors and windows shall be finished in the same way as the faces.

All exposed piping (except PVC), if specified shall be painted to match the adjoining wall surfaces where such wall surfaces are either glazed tile or painted.

Painting around finish hardware of other removable items already in place shall not be allowed.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during painting.

**Galvanized Iron surfaces**

Galvanized iron should not be painted until it has been exposed to the weather for a year as paint adheres badly to new galvanized iron. If necessary to paint sooner, a coat composed of about 200 grams of copper acetate added to 5 litres of water, or 60 grams of muriatic acid added to a mixture of 60 grams each of copper chloride, copper nitrate and sal-ammoniac, dissolved in 5 litres of soft water, to which a small quantity of hydrochloric acid has been added, should be given. This will be sufficient for about a surface area of 250 square meter.

**Completion**

At completion of painting work, the Contractor shall remove any paint spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall repaint or retouch, as directed by the Engineer, any surface which do not comply with the requirements of these specifications or which have been damaged during performing
works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.

20.1.4 Measurement
Measurement shall be taken for payment in square meter of the surface area actually painted with required quality in accordance with the provisions of the BOQ and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

20.1.5 Payment
The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and paint and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Synthetic enamel painting</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
20.2 Plastic painting

20.2.1 Description

Works covered under this item shall consist of painting plastered wall or ceiling surfaces or the places as directed by the Engineer. Synthetic resin emulsion paint of any approved brand and colour shall be used in minimum 3 coats in accordance with these specifications, as per the provisions in the BOQ and/or as directed by the Engineer or as specified by the manufacturers. Painting shall be done in a manner to obtain an even, smooth finish of confirm shade and without any mark of brush and joint.

20.2.2 Materials

Paint shall consist of ready-mixed synthetic emulsion resin base water-thinned approved products supplied in original sealed containers bearing the manufacturer’s trade mark.

All painting materials shall be of the best quality and be delivered at the Site in sealed original containers bearing manufacturer’s labels and seals.

Materials to be used in the work shall conform to the reputed manufacturer’s specifications and to the satisfaction of the Engineer.

Storage of materials

Materials and tools shall be stored in a single place at the Site as designated by the Engineer.

Storage area shall be maintained in a neat and clean condition with surroundings protected from damage.

Inflammable materials shall be stored in sealed containers. Waste shall be removed from the premises at the end of each day’s work. Every precaution shall be taken to prevent fire.

Storage area shall be all time accessible to the Engineer.

20.2.3 Construction methods

General

Before purchasing materials, the Contractor shall submit to the Engineer a list showing the brand and type of paints proposed for this item of work. Manufacturer’s catalogue or specification sheets, in triplicate, for materials selected shall be submitted to the Engineer with the list of brands and types. No material shall be used without the approval of the Engineer.

Colours and samples

Colour scheme shall conform the Finish Schedule and as directed by the Engineer. All tinting and matching shall be to the satisfaction of the Engineer.

For all finished painting on plastered masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be painted. The finished samples shall be approved by the Engineer.
Protection

Drop cloths or other approved protection materials shall be furnished and laid on all areas where painting and finishing is being done so as to adequately protect floor and other places from all damages caused during the execution of the painting work.

Surface preparation

All surfaces to be painted shall be thoroughly cleaned of all grit, grease, dirt, loose materials, mortar drippings and the like. It is better that some soap is added in the wash.

The surface shall be given a thorough rub down to remove all loose materials and all cracks and surface irregularities shall be prepared with patching plaster and filler to obtain a smooth and even surface to the satisfaction of the Engineer. The filler shall always be spread from the same face of the filling knife, the other face shall be kept clean and free. Brush shall always be cleaned after use. Filler shall be applied before priming and the surface shall be rubbed before the application to ensure clean work and again after application on allowing 12 hours to dry.

The plastered surfaces shall be made smooth by sand papering and made free from marks before applying the first coat.

Voids and holes shall be filled after first the coat becomes dry by using filler compatible with the finishing specified and tinted, if required to camouflage repairs.

In the case of new cement plaster walls, a solution of 2 kg of zinc sulphate in 4 litres of water should be applied to the surface and when dry should be given a coat of pure raw linseed oil; or the surface can be treated with dilute sulphuric or hydrochloric acid (1 part acid to 50 parts water) and then washed down with water. Cares shall be taken to ensure that acids are added to the water and not water to the acids.

Application of paint

The workmanship for painting shall be of high quality and experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before painting or finishing is started.

All paints shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. Paint shall be worked into all corners and crevices.

Materials shall be applied in strict accordance with the manufacturer’s directions. In particular, no prepared paint shall be thinned by any method except as directed by the manufacturer. All paint shall be thoroughly mixed before being applied.

Each coat shall be thoroughly dry before application of subsequent coat.

Drying time for every coat shall not be less than 72 hours and 48 hours for exterior and interior painting respectively.

Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.
No exterior painting shall be done in rainy and damp weather until the surface is thoroughly dry. No interior painting shall be done on damp surfaces.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface. The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during painting.

**Completion**

At completion of painting work, the Contractor shall remove any paint spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall repaint or retouch, as directed by the Engineer, any surface which do not comply with the requirements of these specifications or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.

### 20.2.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually painted with required quality in accordance with the provisions of the BOQ and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 20.2.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and paint and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
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<th>Unit</th>
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<tbody>
<tr>
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<td>Square meter</td>
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</table>
20.3 Distempering

20.3.1 Description

Works covered under this item shall consist of distempering plastered wall or ceiling surfaces or the places as directed by the Engineer. Distemper of any approved brand and colour shall be used in minimum 3 coats in accordance with these specifications, as per the provisions in the BOQ and/or as directed by the Engineer or as specified by the manufacturers. Distempering shall be done in a manner to obtain an even, smooth finish of confirm shade and without any mark of brush and joint.

20.3.2 Materials

Paint shall consist of approved ready-mixed distemper supplied in original sealed containers bearing the manufacturer’s trademark.

All distempering materials shall be of the best quality and be delivered at the Site in sealed original containers bearing manufacturer’s labels and seals.

Materials to be used in the work shall conform to the reputed manufacturer’s specifications and to the satisfaction of the Engineer.

Storage of materials

Materials and tools shall be stored in a single place at the Site as designated by the Engineer.

Storage area shall be maintained in a neat and clean condition with surroundings protected from damage.

Inflammable materials shall be stored in sealed containers. Waste shall be removed from the premises at the end of each day’s work. Every precaution shall be taken to prevent fire.

Storage area shall be all time accessible to the Engineer.

20.3.3 Construction methods

General

Before purchasing materials, the Contractor shall submit to the Engineer a list showing the brand and type of distemper proposed for this item of work. Manufacturer’s catalogue or specification sheets, in triplicate, for materials selected shall be submitted to the Engineer with the list of brands and types. No material shall be used without the approval of the Engineer.

Colours and samples

Colour scheme shall conform the Finish Schedule and as directed by the Engineer. All tinting and matching shall be to the satisfaction of the Engineer.

For all finished distempering on plastered masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be painted. The finished samples shall be approved by the Engineer.
Protection

Drop cloths or other approved protection materials shall be furnished and laid on all areas where distempering and finishing is being done so as to adequately protect floor and other places from all damages caused during the execution of the distempering work.

Surface preparation

All surfaces to be distempered shall be thoroughly cleaned of all grit, grease, dirt, loose materials, mortar drippings and the like.

The surface shall be given a thorough rub down to remove all loose materials and all cracks and surface irregularities shall be prepared with patching plaster and filler to obtain a smooth and even surface to the satisfaction of the Engineer. The filler shall always be spread from the same face of the filling knife, the other face shall be kept clean and free. Brush shall always be cleaned after use. Filler shall be applied before priming and the surface shall be rubbed before the application to ensure clean work and again after application on allowing 12 hours to dry.

The plastered surfaces shall be made smooth by sand papering and made free from of marks before applying the first coat.

Voids and holes shall be filled after first the coat becomes dry by using filler compatible with the finishing specified and tinted, if required to camouflage repairs.

In the case of new cement plaster walls, a solution of 2 kg of zinc sulphate in 4 litres of water should be applied to the plastered surface and when dry should be given a coat of pure raw linseed oil; or the surface may be treated with dilute sulphuric or hydrochloric acid (1 part acid to 50 parts water) and then washed down with water. Cares shall be taken to ensure that acids are added to the water and not water to the acids.

Application of distemper

The workmanship for painting shall be of high quality and experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before distempering or finishing is started.

All distemper shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. Distemper shall be worked into all corners and crevices.

Materials shall be applied in strict accordance with the manufacturer's directions. In particular, no prepared distemper shall be thinned by any method except as directed by the manufacturer. All distemper shall be thoroughly mixed before being applied.

Each coat shall be thoroughly dry before application of subsequent coat.

Drying time for every coat shall not be less than 72 hours and 48 hours for exterior and interior painting respectively.

Each coat applied, must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to
repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.

No exterior distempering shall be done in rainy and damp weather until the surface is thoroughly dry. No interior painting shall be done on damp surfaces.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during distempering.

**Completion**

At completion of distempering work, the Contractor shall remove any distemper spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall repaint with distemper or retouch, as directed by the Engineer, any surface which do not comply with the requirements of these specifications or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.

**20.3.4 Measurement**

Measurement shall be taken for payment in square meter of the surface area actually distempered with required quality in accordance with the provisions of the BOQ and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

**20.3.5 Payment**

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and distemper and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Distempering plastered surface</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
20.4 White washing

20.4.1 Description
Works covered under this item shall consist of white washing on the plastered wall or ceiling surfaces or at locations as directed by the Engineer in minimum 3 coats in accordance with these specifications, as per the provisions in the BOQ and/or as directed by the Engineer. White washing shall be done in a manner to obtain an even, smooth finish without any mark of brush and joint.

20.4.2 Materials

Limestone
Limestone is high calcium lime with about 6% material insoluble in acid, obtained by burning pure limestone or chalk in a kiln. Limestone shall be slaked as early as possible after it is burnt in a kiln. Stone lime should be stored in an enclosed space in large heaps and air excluded as far as possible. Un-slaked lime weighs 640 kilogram per cubic meter when fresh, increasing to about 800 kilogram per cubic meter after 10 days.

Shell lime
Shell lime is also high calcium lime with about 6% material insoluble in acid, obtained by burning seashells in a kiln.

Gum arabic
This is a kind of glue used as a binding agent between the white wash and the plaster surfaces.

Robin blue
Robin blue is a kind of manufactured ready-made blue available in packets from reputed manufacturer. This is required for maintaining the whiteness of the wash.

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

20.4.3 Construction methods

Samples
White washing scheme shall conform the Finish Schedule and as directed by the Engineer. All matching shall be to the satisfaction of the Engineer.

For all finished white washing on plastered masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be washed. The finished samples shall be approved by the Engineer.
Protection

Drop cloths or other approved protection materials shall be furnished and laid on all areas where white washing is being done so as to adequately protect floor and other places from all damage caused during the execution of the distempering work.

Surface preparation

All surfaces to be white washed shall essentially be thoroughly cleaned by removing all grit, grease, dirt, loose materials, mortar drippings and the like. It is better that some soap is added in the wash.

The surface shall be given a thorough rub down with a brush or by rubbing with an old gunny bag to remove all loose materials. All holes, cracks, surface irregularities and minor repairs shall be made good with patching plaster and lime putty to obtain a smooth and even surface to the satisfaction of the Engineer. Lime putty is obtained by slaking lime with water and sifting it. The filler shall be let dry for 24 hours before white washing. The filler shall always be spread from the same face of the filling knife, the other face shall be kept clean and free. Filler shall be applied before priming and the surface shall be rubbed before the application to ensure clean work and again after application on allowing 12 hours to dry.

The plastered surfaces shall be made smooth by sand papering and made free from marks before applying the prime coat.

In the case of new cement plaster walls, a solution of 2 kg of zinc sulphate in 4 litres of water should be applied to the plastered surface and when dry given a coat of pure raw linseed oil; or the surface may be treated with dilute sulphuric or hydrochloric acid (1 part acid to 50 parts water) and then washed down with water. Cares shall be taken to ensure that acids are added to the water and not water to the acids.

Preparation of white wash

The lime shall be brought to the Site in an un-slaaked condition and thoroughly slaked on the spot, mixed and stirred with sufficient water and requisite amount of blue and gum to make a thin cream and allowed to stand for 24 hours. If 4 grams of gum (or shellac) and 50 grams of common salt dissolved in hot water are added to 1 kilogram of limestone for the last coat, the white wash will not easily rub off. Indigo (blue) upto 3 grams per kilogram of lime dissolved in water is added and the wash stirred well.

The lime is placed 30cm deep in a drum or a tub with about 90cm of water and allowed to stand for about 24 hours or such longer period as may be necessary to slake the lime completely. It is better to add lime to the water and not water to the lime. The mixture should be well stirred.

Lime is considered to be completely slaked when the temperature of the lime and the water ceases to rise and any further addition of water also produces no further chemical action or heat. As a precaution, water should be allowed to stand on for 12 hours or more. A vigorous slaking with heat and noise indicates a high calcium content. After slaking, the lime should be screened through a 3.35mm sieve or kept in excess of water to meet the requirements. Limes must be thoroughly slaked which is also ground very fine. Any un-slaked particles left will produce “blisters”.

Application of white wash

The workmanship for white washing shall be of high quality. Experienced and skilled painters shall be engaged for the work.
No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before washing or finishing is started.

The lime wash should be strained through a course cloth or sieved through a fine wire gauge before applying.

All white washing shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. White washing shall be worked in to all corners and crevices.

The coats shall be applied alternatively vertically and horizontally. One stroke is given from the top downwards and the other from the bottom upwards over the first stroke and similarly, one stroke from the right and another from the left over the first brush before it dries. Each coat shall be let to dry before applying the next coat.

White wash shall be applied on surfaces in two coats over a priming coat. The final coat shall be applied vertically and finished surface shall be free of dust, dirt and must be free from brush marks. The finished dry surface shall not readily come off on the hand when rubbed.

Brush shall always be cleaned after use.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

Adequate precautionary measures shall be taken so as not to damage or stain floors, walls or any other works while applying white wash. Any damage, stains or spots caused by white washing shall be rectified and removed at the expenses of the Contractor.

Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.

No exterior washing shall be done in rainy and damp weather until the surfaces are thoroughly dry. No interior washing shall be done on damp surfaces.

Each coat shall be thoroughly dry before application of the subsequent coat.

Drying time for every coat shall not be less than 72 hours and 48 hours for exterior and interior washing respectively.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during white washing.

Completion

At completion of white washing, the Contractor shall remove any wash spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall rewash or retouch any surface, which do not comply with the requirements of these specifications or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.
All rubbish and accumulated painting materials shall be removed from the premises.

20.4.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually white washed with required quality in accordance with the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

20.4.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and all white washing materials and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>White washing</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
20.5 Colour washing

20.5.1 Description
Works covered under this item shall consist of colour washing on the plastered wall or ceiling surfaces or at locations as directed by the Engineer in minimum 3 coats in accordance with these specifications, as per the provisions in the BOQ and/or as directed by the Engineer. Colour washing shall be done in a manner to obtain an even, smooth finish without any mark of brush and joint.

20.5.2 Materials

Lime stone
Limestone is high calcium lime with about 6% material insoluble in acid, obtained by burning pure limestone or chalk in a kiln. Limestone shall be slaked as early as possible after it is burnt in a kiln. Stone lime should be stored in an enclosed space in large heaps and air excluded as far as possible. Un-slaked lime weighs 640 kilogram per cubic meter when fresh, increasing to about 800 kilogram per cubic meter after 10 days.

Shell lime
Shell lime is also high calcium lime with about 6% material insoluble in acid, obtained by burning seashells in a kiln.

Gum arabic
This is a kind of glue used as a binding agent between the white wash and the plaster surfaces.

Colour
Mineral colours, not affected by lime, shall be added to white wash instead of indigo (blue).

Water
Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

20.5.3 Construction methods

Samples
Colour washing scheme shall conform the Finish Schedule and as directed by the Engineer. All tinting and matching shall be to the satisfaction of the Engineer.

For all finished colour washing on plastered masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be washed. The finished samples shall be approved by the Engineer.

Protection
Drop cloths or other approved protection materials shall be furnished and laid on all areas where colour washing is being done so as to adequately protect floor and other places from all damage caused during the execution of the distempering work.
**Surface preparation**

All surfaces to be colour washed shall essentially be thoroughly cleaned through removing all grit, grease, dirt, loose materials, mortar drippings and the like. It is better that some soap is added in the wash.

The surfaces shall be given a thorough rub down with a brush or by rubbing with an old gunny bag to remove all loose materials. All holes, cracks, surface irregularities and minor repairs shall be made good with patching plaster and lime putty to obtain a smooth and even surface to the satisfaction of the Engineer. Lime putty is obtained by slaking lime with water and sifting it. The filler shall be let dry for 24 hours before colour washing. The filler shall always be spread from the same face of the filling knife, the other face shall be kept clean and free. Filler shall be applied before priming and the surface shall be rubbed before the application to ensure clean work and again after application on allowing 12 hours to dry.

The plastered surfaces shall be made smooth by sand papering and made free from marks before applying the prime coat.

In the case of new cement plaster walls, a solution of 2 kg of zinc sulphate in 4 litres of water should be applied to the plastered surface and when dry given a coat of pure raw linseed oil; or the surface may be treated with dilute sulphuric or hydrochloric acid (1 part acid to 50 parts water) and then washed down with water. Cares shall be taken to ensure that acids are added to the water and not water to the acids.

**Preparation of colour wash**

The lime shall be brought to the Site in an un-slaked condition and thoroughly slaked on the spot, mixed and stirred with sufficient water and requisite amount of colour and gum to make a thin cream and allowed to stand for 24 hours. If 4 grams of gum (or shellac) and 50 grams of common salt dissolved in hot water are added to 1 kilogram of limestone for the last coat, the colour wash will not easily rub off. Mineral colour in requisite quantity per kilogram of lime dissolved in water is added and the wash stirred well.

The lime is placed 30cm deep in a drum or a tub with about 90cm of water and allowed to stand for about 24 hours or such longer period as may be necessary to slake the lime completely. It is better to add lime to the water and not water to the lime. The mixture should be well stirred.

Lime is considered to be completely slaked when the temperature of the lime and the water ceases to rise and any further addition of water also produces no further chemical action or heat. As a precaution, water should be allowed to stand on for 12 hours or more. A vigorous slaking with heat and noise indicates high calcium content. After slaking, the lime should be screened through a 3.35mm sieve or kept in excess of water to meet the requirements. Limes must be thoroughly slaked which is also ground very fine. Any un-slaked particles left will produce “blisters”.

**Application of colour wash**

The workmanship for colour washing shall be of high quality. Experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before washing or finishing is started.

The colour wash should be strained through a course cloth or sieved through a fine wire gauge before applying.
All colour washing shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. Colour washing shall be worked into all corners and crevices.

The coats shall be applied alternatively vertically and horizontally. One stroke is given from the top downwards and the other from the bottom up-wards over the first stroke and similarly, one stroke from the right and another from the left over the first brush before it dries. Each coat shall be let to dry before applying the next coat.

Colour wash shall be applied on the surfaces in two coats over a prime coat. The prime coat for the colour wash shall be of white wash with lime or with whiting. The final coat shall be applied vertically and finished surface shall be free of dust, dirt and must be free from brush marks. The finished dry surface shall not readily come off on the hand when rubbed.

In replacing one colour with another, a coat of white wash shall be given or the old paint scraped off, before the new colour is given. Gum or rice water shall be added as for white washing.

Brush shall always be cleaned after use.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

Adequate precautionary measures shall be taken so as not to damage or stain floors, walls or any other work while applying white wash. Any damage, stains or spots caused by colour washing shall be rectified and removed at the expenses of the Contractor.

Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.

No exterior washing shall be done in rainy and damp weather until the surfaces are thoroughly dry.

No interior washing shall be done on damp surfaces.

Each coat shall be thoroughly dry before application of subsequent coat.

Drying time for every coat shall not be less than 72 hours and 48 hours for exterior and interior washing respectively.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during application of colour wash.

**Completion**

At completion of colour washing, the Contractor shall remove any wash spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.
The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall rewash or retouch any surface, which do not comply with the requirements of these specifications or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.

**20.5.4 Measurement**

Measurement shall be taken for payment in square meter of the surface area actually colour washed with required quality in accordance with the provisions of the BOQ and/or shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

**20.5.5 Payment**

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and all colour washing materials and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Colour washing</td>
<td>Square meter</td>
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</tbody>
</table>
20.6 Coloured cement painting

20.6.1 Description

Works covered under this item shall consist of applying coloured cement paint on the wall or ceiling plaster or at any other locations as directed by the Engineer. Cement paint of any approved brand and colour shall be used in minimum 2 coats over a coat of priming in accordance with these specifications as per the provisions in the BOQ and/or as directed by the Engineer or as specified by the manufacturers. Painting shall be done in a manner to obtain an even, smooth finish of confirm shade and without any mark of brush and joint.

20.6.2 Materials

Cement powder

Cement powder shall be manufactured of the best quality and of approved colour supplied in original sealed containers bearing the manufacturers labels and seals and be delivered at the Site.

Materials to be used in the work shall conform to the reputed manufacturer’s specifications and to the satisfaction of the Engineer.

Water

Water shall be clean, free from injurious quantities of oil, alkali, salts and organic materials or other substances that may be deleterious to concrete or reinforcement and shall not contain any visibly solid material. If requested by the Engineer, water shall be tested by comparing with water of known satisfactory quality. All other requirements shall be similar to what have been stated under the relevant Sub-sections of the Sections on ‘Concrete Work’ and ‘Construction Materials’ of this Specification.

20.6.3 Construction methods

General

Before purchasing materials, the Contractor shall submit to the Engineer a list showing the brand and type of cement powder proposed for this item of work. Manufacturer’s catalogue or specification sheets, in triplicate, for materials selected shall be submitted to the Engineer with the list of brands and types. No material shall be used without the approval of the Engineer.

Samples

Coloured cement painting scheme shall conform the Finish Schedule and as directed by the Engineer. All tinting and matching shall be to the satisfaction of the Engineer.

For all finished coloured cement painting on plastered masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be painted. The finished samples shall be approved by the Engineer.

Protection

Drop cloths or other approved protection materials shall be furnished and laid on all areas where painting is being done so as to adequately protect floor and other places from all damages caused during the execution of the coloured cement painting.

Surface preparation

All surfaces to be painted shall essentially be thoroughly cleaned through removing all grit, grease, dirt, loose materials, mortar drippings and the like. It is better that some soap is added in the wash.
The surfaces shall be given a thorough rub down with a brush or by rubbing with an old gunny bag to remove all loose materials. All holes, cracks, surface irregularities and minor repairs shall be made good with patching plaster and lime putty to obtain a smooth and even surface to the satisfaction of the Engineer. Lime putty is obtained by slaking lime with water and sifting it. The filler shall be let dry for 24 hours before colour painting. The filler shall always be spread from the same face of the filling knife, the other face shall be kept clean and free. Filler shall be applied before priming and the surface shall be rubbed before the application to ensure clean work and again after application on allowing 12 hours to dry.

The plastered surfaces shall be made smooth by sand papering and made free from marks before applying the prime coat.

In case of any inconsistency with the manufacturer’s instructions, the manufacturer’s instructions shall prevail.

**Preparation of paint**

The paint shall be prepared by mixing and stirring coloured cement powder, sand/lime as per manufacturer’s specifications and water in such quantities as will produce a mixture of the consistency of thin cream. When sufficiently mixed, the mixture shall be strained through a clean coarse cloth.

**Application of paint**

The workmanship for coloured cement painting shall be of high quality and experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be broom cleaned before washing or finishing is started.

All painting shall be applied with brushes under adequate illumination, evenly spread, smoothly flowed on without runs or sags. Painting shall be worked into all corners and crevices.

Painting shall be applied on the surfaces in two coats over a prime coat. The prime coat shall be of white wash with lime or with whiting. The coats shall be applied alternately vertically and horizontally. The final coat shall be applied vertically and finished surface shall be free from dust, dirt and must be free of brush marks. The finished dry surface shall not readily come off on the hand when rubbed.

In replacing one colour with another, a coat of white wash shall be given or the old paint scraped off, before the new colour is given. Gum or rice water shall be added as for white washing.

Brush shall always be cleaned after use.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

Adequate precautionary measures shall be taken so as not to damage or stain floors, walls or any other work while applying paint. Any damage, stains or spots caused by coloured cement painting shall be rectified and removed at the expenses of the Contractor.

Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.
No exterior painting shall be done in rainy and damp weather until the surface is thoroughly dry. No interior painting shall be done on damp surfaces.

Each coat shall be thoroughly dry before application of subsequent coat.

The washing shall be done with good hairbrush and not with brush made of jute.

Proper curing shall be done at least for 7 days on application of the final coat and/or as per instructions of the manufacturer.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during application of paint.

In case of any inconsistency with the manufacturer’s instructions, the manufacturer’s instructions shall prevail.

**Completion**

At completion of coloured cement painting, the Contractor shall remove any paint spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and the Contractor shall repaint or retouch, as directed by the Engineer, any surface which do not comply with the requirements of these specifications or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.

### 20.6.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually painted with required quality in accordance with the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 20.6.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and all painting materials and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities and/or as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Coloured cement painting</td>
<td>Square meter</td>
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</tbody>
</table>
20.7  Water repellent painting

20.7.1 Description

Works covered under this item shall consist of applying two coats of clean silicon water repellent on exposed brick or concrete surfaces and cement rendered on wall, ceiling and at any other locations in accordance with these specifications, as per the provisions in the BOQ and/or as directed by the Engineer or as specified by the manufacturers.

20.7.2 Materials

Silicon water repellent shall consist of sodium silicate or other alkaline silicates based clear approved product supplied in original sealed containers bearing the manufacturer’s trade mark.

All silicon water repellent painting materials shall be of the best quality and be delivered at the Site in sealed original containers bearing manufacturer’s labels and seals.

Materials to be used in the work shall conform to the reputed manufacturer’s specifications and to the satisfaction of the Engineer.

20.7.3 Construction methods

General

Before purchasing materials, the Contractor shall submit to the Engineer a list showing the brand and type of silicon water repellent proposed for this item of work. Manufacturer’s catalogue or specification sheets, in triplicate, for materials selected shall be submitted to the Engineer with the list of brands and types. No material shall be used without the approval of the Engineer.

Samples

For all finished silicon water repellent painting on masonry and concrete surfaces, samples shall be prepared as per direction of the Engineer on the surfaces to be painted. The finished samples shall be approved by the Engineer.

Protection

Drop cloths or other approved protection materials shall be furnished and laid on all areas where colour washing is being done so as to adequately protect floor and other places from all damages during the execution of the painting.

Surface preparation

All surfaces to be silicon water repellent painted shall essentially be thoroughly cleaned by removing all grit, grease, dirt, loose materials, mortar drippings and the like.

The surfaces shall be given a thorough rub down with a brush or by rubbing with an old gunny bag to remove all loose materials. All holes, cracks, surface irregularities and minor repairs shall be prepared in such a manner so as to provide a smooth and even surface to the satisfaction of the Engineer.

Application of silicon water repellent paint

The workmanship for silicon water repellent paint shall be of high quality and experienced and skilled painters shall be engaged for the work.
No work shall be done under conditions, which are not suitable for the production of good results. All spaces shall be neatly cleaned before painting or finishing starts.

All silicon water repellent paints shall be applied with brushes under adequate illumination, evenly spread and smoothly flowed on. Silicon water repellent paint shall be worked into all corners and crevices.

The application of water repellent coat shall strictly comply with the manufacturer’s instruction. The application shall preferably be carried out after a period of dry weather and before application, the surface shall be thoroughly cleaned and dried. A heavy coat shall be applied evenly direct from the container by flooding the surface with a wide brush so that at least 6mm penetration is achieved. A second coat shall be applied in the similar manner, which shall follow after 24 hours.

Brush shall always be cleaned after use.

Wherever scaffolding is necessary, it shall be free standing so as not to damage or scratch the painted surface.

Adequate precautionary measures shall be taken so as not to damage or stain floors, walls or any other work while applying the paint. Any damage, stains or spots caused by painting shall be rectified and removed at the expenses of the Contractor.

Each coat applied must be inspected and approved by the Engineer before the application of the succeeding coat. Otherwise, no credit for the coat applied will be given and the Contractor may require to repeat the work at his own expenses. The Contractor shall notify the Engineer when each coat is ready for inspection.

No exterior painting shall be done in rainy and damp weather until the surface is thoroughly dry. No interior painting shall be done on damp surfaces.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during painting.

Completion

At completion of silicon water repellent paint the Contractor shall remove any wash spot and stain caused during the whole process of works as stated under this Sub-section from floors, walls, glass, hardware, equipment and other surfaces leaving these surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section, and/or as per provision of the BOQ and the Contractor shall re-paint or retouch, as directed by the Engineer, any surface which do not comply with the requirements of these specifications and/or the provisions of the BOQ or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from all defects and blemishes.

All rubbish and accumulated painting materials shall be removed from the premises.
20.7.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually painted with silicon water repellent of required quality in accordance with the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

20.7.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport all silicon water repellent materials and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
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<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Water repellent paint</td>
<td>Square meter</td>
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</tbody>
</table>
20.8 Varnishing

20.8.1 Description
Works covered under this item shall consist of varnishing to the surfaces of wood works in three coats in accordance with these specifications, as per the provisions of the BOQ and/or as directed by the Engineer.

20.8.2 Materials
Oil varnish
This is a kind of varnish belonging to the class ‘Oil Varnish’. The essential constituent is ‘resin’ or rosin which is dissolved in oils, turpentine, or alcohol. Shellac, Gum Arabic, Rosin and Amber. Resins are most commonly used for preparation of varnishes. Various types of varnishes are obtainable in the market, each suited to a specific work. Preparation of varnishes is a difficult matter and it is best to purchase ready-made.

Storage of materials
Materials and tools shall be stored in a single place at the Site as designated by the Engineer.

Storage area shall be maintained in a neat and clean condition with surroundings protected from damage.

Inflammable materials shall be stored in sealed containers. Waste shall be removed from the premises at the end of each day's work. Every precaution shall be taken to prevent fire.

Storage area shall be all time accessible to the Engineer.

20.8.3 Construction methods

20.8.3.1 Samples
Before starting full scale finishing work, sample finishing shall be done on similar wooden surfaces on an area of 300mm x 300mm and shall receive the approval of the Engineer.

Modifications, if required, in the above specimen shall be done depending on the outcome of sample work.

No payment will be made unless samples are made beforehand and approval of the Engineer is received for the same.

20.8.3.2 Surface preparation
Wood, surfaces that would be varnished, shall be well seasoned. The surfaces to be varnished shall be perfectly dry.

The process of preparing the surfaces shall include removal of all machine and plain marks and defects that will make an imperfect surface. Unless the surface is perfectly smooth and free from defects, varnish shall not be applied.

The following rules shall be maintained in preparing the wood surfaces.

- Before assembling the work, all marks shall be removed from the visible parts with a plane or cabinet scraper.
- All traces of glue from around the joints shall be removed.
Defects, such as cracks and holes that can not be removed, shall be filled with stick shellac or its equivalent.

After the shellac or its equivalent hardens, it shall be placed down until it is nearly leveled with the adjoining surfaces.

The surfaces then shall be scraped and sanded thoroughly. Emery paper shall be used as abrasive. Four grades of abrasive paper shall be used successively in the order of No. 2, No. 1, No. 0 and No. 00. Sand papering shall be done with the grain. When thoroughly sand papered, the dust shall be brushed off with a stiff brush and inspected to see if the surfaces are free from all blemishes. It shall then be rubbed with a clean woolen rag.

Before application of varnish or painting, all articles shall receive inspection and approval of the Engineer.

20.8.3.3 Application

Type of finish

Unless otherwise specified wooden surfaces shall receive clear shellac varnish.

Finishing materials

Fillers shall be White Zinc or natural paste fillers.

Sealers shall be of shellac wash coat. This is a mixture of seven parts alcohol to one part shellac, using Two-Pound-Cut shellac. Two-Pound-Cut shellac means that there are 1.8 kg of shellac mixed to 4.5 liters of alcohol.

Finish shall be done with a mixture of equal amount of alcohol and Four-Pound-Cut shellac.

Benzene shall be used as a cleaning fluid.

Application method

Fillers made in the form of heavy paste by adding desired amount of turpentine shall be applied with a stiff brush, brushing first with the grain and then across it, covering only a small area at a time. It shall be allowed to dry for a few minutes until it loses its glossy appearance.

Excess fillers shall be wiped off across the grain with rough cloth. The surface shall then be rubbed down with the grain lightly with soft cloth to remove the excess. It should be pressed in such hardness so that the filler is not wiped off the pores.

Finishing shall be applied only after the filler has dried up.

Clean shellac varnish shall be applied with a good quality brush, 40mm to 75mm wide. Varnishing shall start near the center and top of a vertical surface or the middle of a horizontal surface, quickly brushing out in long sweeping strokes without going over the same area several times as shellac dries out very rapidly.

Brushing should be done towards the edges and care should be taken not to allow the shellac to run over the edges and pile up. It shall then be allowed to dry for 3 to 4 hours. The surface shall be lightly rubbed down with No.00 dry abrasive paper along the grain.
Grit and dust shall be removed with soft cloth before applying the second coat with slightly reduced alcohol mixture. It shall then be allowed to dry and rubbed down lightly with No.00 dry abrasive paper along the grain.

Grit and dust shall be removed again before applying the third coat with 25 percent alcohol mixture.

After the last coat dries up, the surface shall be wiped out lightly with Benzene.

The workmanship for varnishing shall be of high quality for this purpose, experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not congenial for the production of good results. All spaces shall be broom cleaned before varnishing or finishing starts.

All varnish shall be applied under adequate illumination. Varnish shall be worked into all corners and crevices.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during varnishing.

**Completion**

At completion of varnishing work, the Contractor shall remove any varnished spot and stain caused during the whole process of works as stated under this Sub-section leaving the surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and as per provisions of the BOQ. The Contractor shall re-varnish or retouch, as directed by the Engineer, any surface which does not comply with the requirements of these specifications, as per provisions of the BOQ or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated varnishing materials shall be removed from the premises.

### 20.8.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually varnished with required quality in accordance with the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 20.8.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and varnish and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Varnishing</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
20.9 French polishing

20.9.1 Description
Works covered under this item shall consist of French polishing to the surfaces of wood works in three coats over a coat of priming in accordance with these specifications, as per the provisions of the BOQ and/or and/or as directed by the Engineer.

20.9.2 Materials
Spirit varnish
It belongs to the class of Spirit Varnish. It is prepared by dissolving ¼ kilo-gram of shellac in ½ litre of methylated spirit or naptha and straining the solution through a double thickness of coarse muslin. A number of other recipes are also in use.

Storage of materials
Materials and tools shall be stored in a single place at the site as designated by the Engineer.

Storage area shall be maintained in a neat and clean condition with surroundings protected from damage.

Inflammable materials shall be stored in sealed containers. Waste shall be removed from the premises at the end of each day’s work. Every precaution shall be taken to prevent fire.

Storage area shall be all time accessible to the Engineer.

20.9.3 Construction methods
Samples
Before starting full scale finishing work, sample finishing shall be done on similar wooden surfaces that shall receive the approval of the Engineer.

Modifications, if required, in the above specimen shall be done depending on the outcome of sample work.

No payment will be made unless samples are made beforehand and approval of the Engineer is received for the same.

Surface preparation
Wood, surfaces that are to be polished, shall be well seasoned. The surfaces to be polished shall be perfectly dry.

The surfaces of woodwork to be polished should be rubbed down perfectly smooth with medium and fine grade sandpaper. All rubbing shall be done with the grain.

Application
The workmanship for polishing shall be of high quality and experienced and skilled painters shall be engaged for the work.

No work shall be done under conditions, which are not congenial for the production of good results. All spaces shall be broom cleaned before polishing or finishing is started.

All polish shall be applied under adequate illumination. Polishing shall be worked into all corners and crevices.
When wood becomes dry, all cracks and holes are to be filled up with putty and the whole surfaces are rubbed down with sandpaper and allowed to be hard. The surfaces shall be painted first with a filler composed of 3 kilo-gram of whiting mixed in 3 litres of methylated spirit and then sand papered when dried. The polish then be applied to the wood surfaces with a polishing pad of soft cloth containing absorbent cotton filling. Application shall be made by light strokes along the grain. Several coats may be necessary before the desired shine and finish is achieved. The pad shall be dabbed with a drop of olive or mustard oil after each coat in order to allow a smooth working and finish.

Fillers can also be made in other ways as mentioned below:

- Whiting mixed with water
- Three parts linseed oil and one part bees wax boiled
- Plaster of Paris either in water or raw linseed oil

Frequent applications of raw linseed oil rubbed in well rags will give a very fine polish to the wood surfaces.

A good furniture polish can be made of equal parts of vinegar and linseed oil or better vinegar and olive oil in the same proportions as this mixture is less sticky than the former.

The Contractor shall rectify at his own expenses any damage that may be caused to the adjacent works during polishing.

**Completion**

At completion of polishing work, the Contractor shall remove any polished spot and stain caused during the whole process of works as stated under this Sub-section leaving the surfaces in perfect condition.

The Engineer shall conduct a final inspection of all works completed in accordance with this Sub-section and as per provisions of the BOQ. The Contractor shall re-polish or retouch, as directed by the Engineer, any surface which does not comply with the requirements of these specifications, as per provisions of the BOQ or which have been damaged during performing works. All surfaces finished under this Sub-section shall be left in perfect condition, free from defects and blemishes.

All rubbish and accumulated polishing materials shall be removed from the premises.

### 20.9.4 Measurement

Measurement shall be taken for payment in square meter of the surface area actually French polished with required quality in accordance with the provisions of the BOQ and/or as shown on the Drawings and/or as directed by the Engineer. Only the completed works as accepted by the Engineer will be eligible for payment.

### 20.9.5 Payment

The amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter which payment shall constitute the full compensation for furnishing all materials, equipment, appliances and labour including storage, transport, preparing, mixing and applying putty, primer and French polish and providing scaffolding as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>French polishing</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
SECTION-21
DOORS, WINDOWS AND CEILINGS
## DOORS, WINDOWS AND CEILINGS

### CONTENTS

#### SECTION-21

<table>
<thead>
<tr>
<th>21.1</th>
<th>Wood work for door/window frames and shutters</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>21.1.1</td>
<td>Description</td>
<td>245-251</td>
</tr>
<tr>
<td>21.1.2</td>
<td>General requirements</td>
<td>245-246</td>
</tr>
<tr>
<td>21.1.3</td>
<td>Other requirements</td>
<td>246-247</td>
</tr>
<tr>
<td>21.1.4</td>
<td>Materials</td>
<td>246-247</td>
</tr>
<tr>
<td>21.1.4.1</td>
<td>Timber</td>
<td>246-248</td>
</tr>
<tr>
<td>21.1.4.2</td>
<td>Finish hardware</td>
<td>247-248</td>
</tr>
<tr>
<td>21.1.5</td>
<td>Joinery</td>
<td>248-249</td>
</tr>
<tr>
<td>21.1.6</td>
<td>Gluing up</td>
<td>249</td>
</tr>
<tr>
<td>21.1.7</td>
<td>Fabrication and installation</td>
<td>249-250</td>
</tr>
<tr>
<td>21.1.8</td>
<td>Glazing</td>
<td>250-251</td>
</tr>
<tr>
<td>21.1.9</td>
<td>Measurement</td>
<td>251</td>
</tr>
<tr>
<td>21.1.10</td>
<td>Payment</td>
<td>251</td>
</tr>
</tbody>
</table>

| 21.2 | Veneered partex fixed panel door/window | 252 |
| 21.2.1 | Description | 252 |
| 21.2.2 | General requirements | 252 |
| 21.2.3 | Other requirements | 252 |
| 21.2.4 | Materials | 252 |
| 21.2.5 | Joinery | 252 |
21.2.6 Gluing up
21.2.7 Finish hardware
21.2.8 Fabrication and installation
21.2.9 Measurement
21.2.10 Payment

21.3 Aluminium doors, windows and curtain walls
21.3.1 Description
21.3.2 Materials and products
   Doors, windows and curtain walls
   Fasteners, hardware and anchors
   Glazing beads
   Weather stripping
   Joints
   Surface finish
   Accessories
   Sealant
21.3.3 Shop drawings
21.3.4 Installation
21.3.5 Measurement
21.3.6 Payment

21.4 Gypsum board false ceiling
21.4.1 Description
21.4.2 Materials and products
21.4.3 Shop drawings
21.4.4 Installation
21.4.5 Measurement
21.4.6 Payment

21.5 Aluminium false ceiling
21.5.1 Description
21.5.2 Materials and products
21.5.3 Shop drawing
21.5.4 Installation
21.5.5 Measurement
21.5.6 Payment

21.6 Polycarbonate sheet
21.6.1 Description
21.6.2 Materials
   Polycarbonate sheet
   Other properties
21.6.3 Installation
21.6.4 Measurement
21.6.5 Payment
SECTION-21
DOORS, WINDOWS AND CEILINGS

21.1 Wood work for door/window frames and shutters

21.1.1 Description

Works covered under this item shall consist of furnishing, finishing and installing of wooden door/window frames and shutters of the size and shape shown on the Drawings and/or as specified in the Bill of Quantities and/or as directed by the Engineer including supplying and fixing of all finished hardware and glazing.

21.1.2 General requirements

Design drawings

Design Drawings shall be thoroughly studied by the Contractor before the Work is commenced. Detail of joints as shown on the Drawings must be specifically checked. If any detail description or specification is found missing or in the opinion of the Contractor inadequate, inconsistent or otherwise, the Contractor shall draw the same to the attention of the Engineer who may make necessary arrangements as deemed fit. On no account, the Contractor shall use his own judgement when any discrepancy is noticed in the Drawings, details and descriptions.

Shop drawings

Detailed Shop Drawings of doors and windows and other wooden works including glazing and installation details, when required, shall be submitted to the Engineer for approval.

Shop Drawings shall include the submission of manufacturer's literature, brochures and delivery date for all finish hardware and manufacturer's literature or specification for glass.

Fabrication of wooden doors/windows shall not start until the Engineer approves the Shop Drawings.

Samples

Within 35 days (or as may be specified otherwise) following the Contract is awarded, the Contractor shall furnish the Engineer, for his approval, a complete list in 4 copies of all hardware proposed for use under the Contract, scheduling all hardware for every door/window.

The Contractor shall submit to the Engineer two pieces of wood specimens, each of size 150mm x 250mm x 25mm, for his approval.

The Contractor shall submit to the Engineer one sample of each type of finished hardware for doors and windows for his approval. However, the hardware shall be of the quality equal to or better than the samples, if available in the Engineer's office.

The Contractor shall submit to the Engineer two pieces of glass sample, each of size 150mm x 200mm and of required thickness proposed for glazing for his approval. The samples shall bear the name of the manufacturer, thickness and the type of glass.

All approved samples may be kept at Site for comparing the materials supplied by the Contractor.
21.1.3 Other requirements

Dimensions shown on the Drawings are finished dimensions. In sizing rough components, necessary allowance, therefore, must be kept for the working loss arising from planing, smoothening and finishing.

Requisite precautionary measures against fire, denting, breakage or loss must be ensured while the articles are in transit and till the supply is completed.

Polishing or painting, as the case may be, shall be done at Site on receiving approval of the woodwork, carpentry etc. by the Engineer. The working area shall be cleaned properly before the finishing works start and subsequently before each day’s work to ensure reasonably dust-free surroundings.

Particulars of the workshop, working area and storage space must be furnished to the Engineer, which shall be checked by him. If required, modifications shall be made as instructed by the Engineer to ensure proper atmosphere and amenities.

The Contractor shall provide adequate locked-up storage space. The Contractor shall replenish all lost or damaged hardware at his own expenses.

In case of inflicting injury to any part of the building/other works while installing, the Contractor shall rectify the same employing proper workers of the trade and furnishing all requisite materials at his own expenses.

The Contractor shall keep the Employer indemnified against all charges, which may arise out of this Contract in case of procurement of timbers from local sources.

21.1.4 Materials

21.1.4.1 Timber

General

Timber, only as specified on the Drawings and Bill of Quantities, shall be used. Timber used for woodwork shall be well seasoned, kiln dry containing not more than 8% to 12% moisture so as to ensure minimum tendency towards warping, shrinking and swellings. It shall be free from all defects, such as large or loose knots, saps, shakes, upsets, wane edge and twisted fibre. It shall also be free from all diseases such as decay, wet rot, dry rot, woodworms and white ant. Timber shall be finished to the exact dimensions shown on the Drawings or as directed by the Engineer. The pieces of wood shall be properly finished by planer and other tools before joining and the completed wood works shall be accepted by the Engineer before fixing those in position.

Wood for frame

High quality, well-seasoned Garjan, Jarul, Local Sal, Shilkarai/Chikrashi, Telsu and Teak Chambol or any other equivalent type of wood as approved by the Engineer, shall be used for frame work or any other related works required.

Wood veneered flash door shutter

Jack wood, Gamari and Chapalish wood, Teak Chambol wood and Chittagong Teak wood veneered flash door shall be used for door shutter or sash or any other related works required. Any other equivalent type of wood may be used when it is required by the Engineer.
Wood for solid door shutter

High quality Jack wood, Chittagong Teak wood, Gammmari and Chapalish wood and Teak Chambol wood or any other equivalent type of wood, as approved by the Engineer, shall be used for solid door shutter or any other related works required.

Wood for panel door shutter

High quality Jack wood, Chittagong Teak wood Gammmari and Chapalish wood and Teak Chambol wood or any other equivalent type of wood, as approved by the Engineer, shall be used for solid door shutter or any other related works required.

21.1.4.2 Finish hardware

Materials and finish

Two sets of complete list indicating the manufacturer’s name, brand name, type, size and location of all hardware to be installed shall be submitted to the Engineer for his approval. No hardware shall be ordered until the Engineer has approved this list.

Unless otherwise specified on the Drawings and in the Bill of Quantities or directed by the Engineer, specifications of the finish hardware shall be as follows:

Door hinges

Hinges shall be locally available, best quality iron or brass hinge of 100mm size and attached with No. 8 steel screw 32mm long, as approved by the Engineer or as directed by him.

Door stopper

Door stopper shall be made of timber as specified previously in the relevant portion of this Sub-section and shall be 100mm long to act as a stopper to keep the door in open position. Cleat with 65mm steel hinge, “Lion” brand or approved equal shall be fixed with No. 6 steel screws 20mm long, “Diamond” or “Star” brand or equal standard approved by the Engineer or as directed by him. One cleat to be furnished for each leaf as approved by the Engineer.

PVC buffer block

PVC buffer block shall be locally available best quality PVC block 38mm diameter and attached to wall with No.8 steel screw 50mm long, “Diamond” or “Star” brand or approved equal or as directed by the Engineer. One block to be furnished for each leaf as approved by the Engineer.

Door handle

Door handle shall be locally available best quality chromium or nickel plated 150mm long, attached with No.7 steel screw 25mm long or as directed by the Engineer. Two handles to be furnished for each door, one inside and one outside as approved by the Engineer.

Door tower bolts

Door tower bolts shall be locally available best quality 250mm and 200mm brass or iron bolts fitted on the interior side of the door leaf. 250mm long bolts shall be fitted at the top of the leaf vertically and the 200mm long bolts shall be fitted at the bottom of the door leaf horizontally. The 250mm and 200mm tower bolts shall be fitted with No. 6 brass screw 20mm long and as approved by the Engineer.
Hatch bolt

Hatch bolt shall be locally available best quality brass or iron bolt 300mm long fixed with approved bolts and nuts fitted on the outside of the door for locking arrangement as per direction of the Engineer.

Window hinge

Window hinge shall be locally available best quality 100mm iron hinge, “Lion” brand or approved equal, 75mm x 50mm in size and fixed with No. 7 steel screw 25mm long “Diamond” or “Star” brand or approved equal or as directed by the Engineer. Three hinges shall be furnished per window.

Window handles

Window handles shall be locally available best quality Chromium plated 100mm long, “Lion” brand or approved equal or as directed by the Engineer. One handle shall be furnished for each leaf at the centre on the inside.

Windows catch hook

Window catch hook shall be locally available best quality, “Diamond” brand or approved equal, galvanized iron 225mm long or as directed by the Engineer. One catch hook shall be furnished for each leaf.

Window tower bolt

Window tower bolt shall be locally available best quality iron, “Diamond” brand or approved equal. Tower bolts 150mm long be fitted on the interior side of the window leaf. Tower bolts shall be fixed with No. 5 steel screw 20mm long, “Diamond” or “Star” brand or approved equal or as directed by the Engineer and fixed vertically one each at the top and bottom on the interior side of the leaf to close the window from inside.

Glass

All glass shall be the approved best quality locally available sheet glass unless otherwise specified and shall be of the various sizes and thickness as shown on the Drawings and Bill of Quantities. All glass shall be free from bubbles, distortion and flaws of every kind with even surface and free from all other imperfections. Each piece of glass shall bear a label indicating the name of the manufacturer, the thickness and the type of glass. Label shall remain on the glass till they are cleaned finally on completion of work.

Lock sets

All Lock sets shall be of the best quality ‘Yale’ brand door lock or approved equal in perfect operating conditions or as directed by the Engineer, if not shown on the Drawings or mentioned elsewhere. Strikes shall be used where required to protect trim from being marred by hatch bolt. Three keys shall be furnished with proper identification for each lock set.

21.1.5 Joinery

Joints shall be made according to the sizes and profiles as shown on the Drawings.

No extra nails or screws, other than those used in the approved samples, shall be used. Nails and screws to be used on finished surfaces shall be fitted slightly below the surface. Polyvinyl acetate adhesive of ‘Aica Aibon’ brand or other equivalent brand shall be used where use of adhesive is required.
or instructed. Wooden pins, where required or instructed, shall be of the same specimen, perfectly round and pressure fitted in the holes which shall be circular.

All joints shall be of such true-fit that they will not be discernible from a distance of one meter.

Layout of the joints shall be made by using accurate instruments. A knife can be used whenever a line is to be cut. A sharp pencil can be used for all layouts and specially when part of the area is to be cut away. All measurements shall be made from a common starting point, edge or surface. All identical joints shall be laid out at the same time. Two members of each joint shall always be identified with a pencil mark for quick identification during assembling.

Right instrument or machine shall be used for each cut. The cut shall always be made just inside or outside the layout line. The joints shall be trimmed out with a router plane or chisel when necessary.

Proper type of clamping devices shall be used for assembling the joint. A trial assembly shall be made to make sure that each joint shall fit properly. A correct method of fastening shall have to be determined. The same shall be completely square and aligned when properly assembled.

21.1.6 Gluing up

The stock shall be glued together where necessary. The glue shall be of ‘Aica Aibon’ brand Polyvinyl-acetate emulsion adhesive or its equivalent quality. The grain of all the pieces shall run in the same direction. The edge of gluing stock shall be of the same maturity and strength.

Good glue must develop the full strength of the wood under all condition stress. To obtain this result, it is necessary to control the gluing operation as well as the condition of the material. The moisture content before gluing shall not be too low or too high.

The average moisture content of wood is about twelve percent. High quality glue joints can not be made on wet wood. The moisture content of timber shall be checked through each step of manufacturing preferably with an electrical Moisture Meter.

The following points must be checked in order to ensure proper gluing:

- Parts of the assembly are at proper moisture content and temperature.
- Gluing surfaces have been made smooth, free from irregularities and even as much as possible.
- All joints have been placed under equal pressure.
- Excess glue has been removed before machining.

21.1.7 Fabrication and installation

All materials and finish shall get the approval of the Engineer before they are installed. All hardware and accessories shall be purchased in the manufacturer’s original packages complete with all required trimmings. They shall conform to the requirements of the specifications and no substitution shall be made for the samples submitted without prior approval of the Engineer. Required templates shall be submitted for proper installation.

All wood works shall be fabricated and installed in a way to conform to the details and dimensions indicated on the Drawings in the Bill of Quantities and as directed by the Engineer.

All hardware and accessories shall be of best stainless and non-corroding variety of screws, bolts, nuts and other fastenings and approved by the Engineer before attaching them. These shall be of the same finish as the material, which they attach and shall be of the type and standard of the manufacturer.
All items of finish hardware and accessories shall be carefully fitted and adjusted to ensure smooth operation. All items of finish hardware and accessories shall be in perfect operating condition and undamaged while installing.

Door/window frames shall be properly cut, housed together and jointed with ‘mortice’ and ‘tenon’ joints. The frames shall be rabatted on one side by a cut measuring 13mm in one direction and the full thickness of the shutter in the other. Frames shall be plumbed and leveled with corners at right angles. All exposed surfaces shall be smoothened with sandpaper. Back faces of wood, remaining in contact with or against concrete or masonry, shall be treated with a minimum of two coats of wood preservation paint, such as creosote or solignum. Wood preservatives, to be used, shall be approved by the Engineer.

The frame shall be fitted vertically in position true and plumb and fixed with clamps made with M.S. angle iron of size 375mm x 40mm x 6mm set in cement concrete within the masonry wall. There shall be six clamps for door frames and four clamps for window frames in general. However, the number and the size of the clamps shall conform to the requirements of the Drawings and as specified in the Bill of Quantities. The clamps shall be fitted by filling the recess properly so as to fit the frame nicely with the wall.

Door sash shall be of panel design and made as shown on the Drawings. Panel-sash frame shall be housed and jointed with mortise and ‘tenon’ joints. Panels for doors shall be solid wood, placed properly in retaining grooves with 4mm gap between adjacent panels on all sides or as indicated/shown on the Drawings.

Door/window frames and door sash shall be approved by the Engineer on assembling and before installing in position.

All hardware shall be installed and all door/window assemblies shall be fitted properly with minimum clearances. Hinges shall be recessed flush with surrounding wood surfaces. All sash shall be tested for proper and smooth operation without hinge bind.

On completion of door/window installation, all wood surfaces shall be French polished or painted.

21.1.8 Glazing

General requirements

All glazing work shall be performed in accordance with the typical glazing details shown on the Drawings. Joints and spaces, to be sealed, shall be thoroughly dried and made free from dust and other foreign materials before glazing. All glass shall be set with proper clearance as recommended by the manufacturer at all edges. Glass with nipped or damaged edges shall not be installed. Adjacent materials, which are solid, shall be cleaned immediately before the sealant and compound harden or stain the adjoining surfaces.

Glazing process

Glass to be cut to provide a clearance of 1.7mm to 3mm on all sides. A thin layer of sealant made of chalk, double boiled linseed oil and resin is to be applied to the frame surfaces coming in contact with the glass. The glass panes shall be fitted in to the rabbet not less than 20mm wide taking care to centre with equal clearance of jambs between glass and frame. The glass is next to be pressed firmly in to the place against the sealant. A bead of sealant is then to be laid in to the spaces between the glass and the frame. Sufficient sealant shall be applied so that when the stop is put in place, the sealant will be forced in to the gap between the glass and the stop and completely fill the space between the frame, glass and
stop. The removal stop is then to be installed. The remaining space between the face of glass and stop shall be completely filled with sealant.

Cleaning

No glazing shall be considered complete until and unless paints and other stains have been removed from the surface of the glass. Glass must be cleaned and polished with pads of damp cloth and then with clean dry soft cloths. It will have to be finally finished with appropriate glass cleaning fluid and made absolutely free of foreign particles.

Defects and breakage

The Contractor shall replace any glass not conforming these specifications or having defects not admissible under the manufacturer’s grading rules. The Contractor shall replace all glass, broken, cracked or chipped by his workers or by faulty installation or from any other cause. All glasses shall remain in perfect condition at the time of handing over of the building to the Employer.

21.1.9 Measurement

All wood door/window frames and assemblies completed, including all hardware installed in place, shall be measured in cubic meter for the specified section of the installed frame and accepted by the Engineer.

All wood door/window leaf and assemblies completed, including glass panes and all hardware installed in place shall be measured in square meter of the installed area and accepted by the Engineer.

21.1.10 Payment

For all wood door/window frames, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per linear meter, which payment shall constitute the full compensation for furnishing all materials and assemblies, fitting and fixing the frames, filling the recess, painting, all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

For all wood door/window shutters including window glass panes, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials, hardware and assemblies, fitting and fixing the shutters/panes, all tools and appliances and labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

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<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Wood door/window frame</td>
<td>Cubic meter</td>
</tr>
<tr>
<td>Wood door/window shutters/panes</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
21.2 **Veneered partex fixed panel door/window**

21.2.1 **Description**
Works covered by this item shall consist of supplying and fitting fixing Gammari Veneered Partex Panel of the size and shape as shown on the Drawing or as directed by the Engineer including supplying and fixing of all necessary hardware and meeting all requirements as described under the Sub-section on the ‘Wood Work for Door/Window Frames and Shutters’.

21.2.2 **General requirements**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.3 **Other requirements**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.4 **Materials**
Gammari Veneered Partex door>window panel.

21.2.5 **Joinery**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.6 **Gluing up**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.7 **Finish hardware**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.8 **Fabrication and installation**
Same as stated under the Sub-section on ‘Wood Work for Door/Window Frames and Shutters’.

21.2.9 **Measurement**
All Veneered Partex panel door/window and assemblies completed and all hardware installed in place shall be measured in square meter of the installed area. Only the works completed in accordance with the provisions of the BOQ, and/or as shown on the Drawings and/or as directed by the Engineer and accepted by the Engineer will be eligible for payment.

21.2.10 **Payment**
For all Veneered Partex panel door/window and assemblies, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials, hardware and assemblies, fitting and fixing the panel, all tools and appliances and labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Veneered Partex fixed panel door/window</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
21.3 Aluminium doors, windows and curtain walls

21.3.1 Description

Works covered under this item shall consist of supplying and fixing aluminum products of various types and uses such as doors, windows, curtain wall, curtain rail, cladding/flushing of sills, window grills, etc. fitted with necessary hardware, glass (where required) and finished in accordance with applicable Drawings and specifications.

21.3.2 Materials and products

Doors, windows and curtain walls

Doors, windows, curtain walls etc. shall be of approved standard conforming to the U.S. Architectural Aluminum Manufacturing Association (AAMA) or approved equivalent specifications. The frames and such members shall be of extruded shape made of 6063-T5 high quality aluminum alloy having a minimum section thickness of 1.8mm unless otherwise shown on the Drawings or indicated in the Bill of Quantities and shall conform to the U.S. Architectural Aluminum Manufacturing Association or approved equivalent standard.

Fasteners, hardware and anchors

Fasteners, hardware and anchors shall be of aluminum or non-magnetic, non-corrosive material compatible with aluminum. All windows shall be provided with non-jamming latches of rocker type designed to be locked from inside. Window locks shall be 'flush type' as manufactured by the Adams Rite Manufacturing Company of Glendale, California or any approved equivalent. The doors shall be provided with cylinder lock and suitable built-in-non-jamming latches and bolts.

Security locks shall be pin type ‘Mortice’ lock, 6 or 7 pins and adaptable to Master, Grand Master and Great Grand Master keys. Sliding windows and doors shall be fitted with adjustable sealed bearing sheaves of durable hydrated nyons or approved equivalent. Closer, push/pull and kick plates shall have to match with the frames. Any other necessary hardware to be incorporated in the works shall also match with the frame. Assembly and installation screws shall be of stainless steel. Doors, windows, curtain walls etc., to be installed with ‘Teflon’ injected expanding bolts and sills, shall contain adequate provisions for drainage. Head, sills and jamb members shall be comprised as single unit. Aluminum to aluminum contact between hardware parts or moving members shall not be permitted. Such contacts shall be properly insulated.

Glazing beads

Glazing beads shall be aluminum shape-in-interchangeable type.

Weather stripping

Weather stripping shall be of neoprene or silicon treated woven wood or any approved equivalent.

Joints

All joints shall be mechanically done square (telescopic) joints. No ‘Mitral’ joints and forced fitting shall be accepted. All units shall be fabricated at the factory with high dimensional accuracy. It shall be rigid and designed to permit complete weather stripping. In principle, the parts should be put together by self-tapping screws.
**Surface finish**

All exposed surfaces of aluminum members shall be factory finish and of substantially uniform appearance maintaining the “Architectural” standard.

All exposed surfaces shall be given a natural Anodic Oxide Hardcore coating of 15 micron in thickness and a density of 4 mg per square centimeter and a uniform colour tone conforming to the U.S. Architectural Aluminum Manufacturing Association or any other approved equivalent standard. The colour spectrum shall be an-lock. Finish of hardware shall match closely with the door/window/curtain wall finish.

**Accessories**

All accessories necessary for proper fixing and operation such as anchors, clips, fins, sub-frames, metal sills, mullion, covers, other trim, cleaning anchors, glazing beads, weathering and glazing strips, hardware and mechanical operators, etc. shall be supplied ready to set in place with the door, window, curtain wall units.

Steel or wood sub-frames shall be painted with Zinc Chromatic primer in case of steel and with wood preservative in case wood. Steel anchor shall be properly insulated from aluminum frame.

**Sealant**

Sealant shall be one part elastic compound of “Architectural” grade caulk and shall be in matching colour.

21.3.3 **Shop drawings**

The Contractor shall prepare detailed design of all works involved in line with the Employers design and prepare Shop Drawings for the total works and submit to the Engineer for approval before factory fabrication starts. All exterior doors, windows and curtain walls shall be designed to withstand a wind pressure of 180 kg/cm².

21.3.4 **Installation**

All units shall be assembled at Site under proper conditions, erected, fixed and glazed in place in strict conformity with the manufacturer’s instruction. All cut-out operations for hardware preparation shall be made accurately and reinforced as required.

All doors, windows, curtain walls, etc. shall be set plumb, square, level and in exact alignment with surrounding works and shall be securely anchored ready for operation. All joints between the masonry opening and frames shall be caulked and sealed after installation of the frames. All installation works shall be done and finished in such a way as to ensure a free and smooth operation.

Abrasion or other injuries to the finished surfaces shall be carefully avoided. Cleaning should be accomplished with plain water or a petroleum type cleaning agent or with the manufacturer’s recommended cleaning reagent. No corrosive reagent shall be used.

21.3.5 **Measurement**

All aluminium door and window frame shutter with glass assemblies complex including all hardware, installed in place shall be measured in square meter of the installed frame. Only the works completed in accordance with the provisions of the BOQ, and/or as shown on the Drawings and/or as directed by the Engineer and accepted by the Engineer will be eligible for payment.
21.3.6 Payment

For all aluminium door and window frame shutter with glass, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials and assemblies and hardware, fitting and fixing, all tools, accessories and appliances and labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Aluminium door, windows and curtain walls</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
21.4 Gypsum board false ceiling

21.4.1 Description
Works covered under this item shall consist of supplying, fitting and fixing thermal acoustical insulation of ceiling finished in accordance with the specifications of Drawings and Bill of Quantities.

21.4.2 Materials and products
Materials for false ceiling shall be made of noncombustible extruded mineral fibre or gypsum board tiles of thickness 12mm and size 600mm x 1200mm or as otherwise required and suspended from the roof by means of adjustable height suspension system and on an appropriate frame work of galvanized steel or aluminium cross rails, furring channels, furring channel joineries, locking keys fasteners, renal plugs etc. and aluminium ceiling tees or any other appropriate and approved material, section and quality.

The ceiling tiles shall be supplied with adjustable height, corrosion resistant metal suspension systems with necessary accessories, all in adequate quantities as recommended by the manufactures.

The Contractor shall submit three sets of samples of all types of materials and products to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

21.4.3 Shop drawings
The Contractor shall prepare detailed design of all works involved in line with the Employers design and prepare Shop Drawings for the total works and submit to the Engineer for approval before any work starts.

21.4.4 Installation
All units shall be assembled under proper conditions, erected and fixed in place in strict conformity with the manufacturer's instruction. All cut-out operations shall be made accurately and reinforced as required.

Abrasion or other injuries to the finished surfaces shall be carefully avoided. Cleaning should be accomplished with appropriate type cleaning agent as recommended by the manufacturer or as directed by the Engineer. No corrosive reagent shall be used.

21.4.5 Measurement
All false ceiling including all aluminium channel, mineral fibre or gypsum board and including all other hardware, installed in place will be measured in square meter of the installed frame and accepted by the Engineer.

21.4.6 Payment
For all false ceiling, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials and assemblies, fitting and fixing, all tools, accessories and appliances and all labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Gypsum board false ceiling</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
21.5 Aluminium false ceiling

21.5.1 Description
Works covered under this item shall consist of supplying, fitting and fixing non-combustible aluminium channel false ceiling finished in accordance with the specifications of Drawings and Bill of Quantities.

21.5.2 Materials and products
Materials for false ceiling shall be made of non-combustible extruded aluminium channel, aluminium board tiles of 2mm thickness and suitable size and suspended from ceiling by means of adjustable height suspension system and on a frame work of galvanized steel or aluminium cross rails, furring channels, furring channel joineries, locking keys fasteners, royal plugs etc. and aluminium ceiling tees or any other approved section of approved quality.

The ceiling tiles shall be supplied with adjustable height, corrosion resistant metal, suspension systems with necessary accessories, all in adequate quantities as recommended by the manufacturer.

The Contractor shall submit three sets of samples of all types of materials and products to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

21.5.3 Shop drawing
The Contractor shall prepare detail design of the works involved in line with the Employer's design and prepare Shop Drawing for the Work and submit to the Engineer for approval before starting the work. No work shall start before obtaining such approval.

21.5.4 Installation
All units shall be assembled under proper conditions, erected and fixed in place in strict conformity with the manufacturer's instruction. All cut-out operations shall be made accurately and reinforced as required.

Abrasion or other injuries to the finished surfaces shall be carefully avoided. Cleaning should be accomplished with appropriate type cleaning agent as recommended by the manufacturer or as directed by the Engineer. No corrosive reagent shall be used.

21.5.5 Measurement
All false ceiling completed with noncombustible aluminium channel and including all other hardware, installed in place will be measured in square meter of the installed ceiling area and accepted by the Engineer.

21.5.6 Payment
For all false ceiling, completed with noncombustible aluminium channel, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials and assemblies, fitting and fixing, all tools, accessories and appliances and labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
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<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Aluminium false ceiling</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
21.6 Polycarbonate sheet

21.6.1 Description

The work covered under this item shall consist of supplying and spreading polycarbonate sheet on roof, covering the opening or over “skywalk”/“sky-bridge” connecting the proposed building with the old one in accordance with the applicable plans, Bill of Quantities and these specifications.

21.6.2 Materials

Polycarbonate sheet

Polycarbonate sheet has excellent resistance when compared to other transparent glazing material. The sheet is available with thickness of 4mm, 6mm, 8mm, 10mm and 16mm. The colours are also various like clear, opal, gray, brown, green and blue. Other properties are as follows:

Polycarbonate sheet can be cold with a minimum radius of 150 times the thickness.

<table>
<thead>
<tr>
<th>Thickness</th>
<th>4mm</th>
<th>6mm</th>
<th>8mm</th>
<th>10mm</th>
<th>16mm</th>
</tr>
</thead>
<tbody>
<tr>
<td>Radius</td>
<td>10cm</td>
<td>90cm</td>
<td>120cm</td>
<td>150cm</td>
<td>cm</td>
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Other Properties

<table>
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<tr>
<td>Density</td>
<td>DIN 53479</td>
<td>g/cm³</td>
<td>1.2</td>
</tr>
<tr>
<td>Modulus of Elasticity</td>
<td>DIN 53457</td>
<td>N/mm²</td>
<td>2000-2200</td>
</tr>
<tr>
<td>Tensile Strength at Break</td>
<td>DIN 53455</td>
<td>N/mm²</td>
<td>&gt;60</td>
</tr>
<tr>
<td>Tensile Strength at Yield</td>
<td>DIN 53455</td>
<td>N/mm²</td>
<td>&gt;70</td>
</tr>
<tr>
<td>Coefficient of Thermal Expansion</td>
<td>VDE 0304/1</td>
<td>K₁, 10⁻⁶</td>
<td>60-70</td>
</tr>
<tr>
<td>Thermal Conductivity</td>
<td>DIN 52612</td>
<td>W/k.m</td>
<td>0.2</td>
</tr>
<tr>
<td>Heat Resistant Temperature</td>
<td>DIN 53460</td>
<td>°C</td>
<td>145-150</td>
</tr>
<tr>
<td>Moisture Absorption</td>
<td>DIN 53495</td>
<td>%</td>
<td>0.36</td>
</tr>
<tr>
<td>Vapour Permeability</td>
<td>DIN 53122</td>
<td>g/m²d</td>
<td>2.28 (1mm)</td>
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</tbody>
</table>

Thermal Transmission Coefficient (K).

<table>
<thead>
<tr>
<th>Material</th>
<th>K [kcal/(h.m²·°C)]</th>
<th>K [W/(m²·°C)]</th>
</tr>
</thead>
<tbody>
<tr>
<td>Glass 4mm</td>
<td>5.5</td>
<td>5.6</td>
</tr>
<tr>
<td>Double glass 4/12/16</td>
<td>2.6</td>
<td>3.0</td>
</tr>
<tr>
<td>Acrylic Sheet 4mm</td>
<td>0.46</td>
<td>5.3</td>
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<tr>
<td>Fiber glass 1.2mm</td>
<td>5.5</td>
<td>6.4</td>
</tr>
<tr>
<td>PC 4mm</td>
<td>3.4</td>
<td>3.9</td>
</tr>
<tr>
<td>PC 6mm</td>
<td>3.2</td>
<td>3.7</td>
</tr>
<tr>
<td>PC 8mm</td>
<td>3.1</td>
<td>3.6</td>
</tr>
<tr>
<td>PC 10mm</td>
<td>2.9</td>
<td>3.4</td>
</tr>
<tr>
<td>PC 16mm</td>
<td>2.0</td>
<td>2.3</td>
</tr>
</tbody>
</table>
21.6.3 Installation

The material for the polycarbonate sheet shall be selected by the Engineer as regards to its thickness, colour and other properties unless otherwise those have been specified on the Drawings or in the Bill of Quantities.

The Contractor shall submit three sets of samples of all types of materials and products to the Engineer for his approval before procuring the materials. One set will be kept at the office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

Manufacturer’s literature or specifications shall be strictly followed for installation and laying of all polycarbonate sheet.

21.6.4 Measurement

All polycarbonate sheet as laid and installed in place will be measured in square meter of the installed roof area and accepted by the Engineer.

21.6.5 Payment

For all polycarbonate sheet roofing, completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing and installation of all materials and assemblies, all tools, accessories and appliances and labour including storage, transport, and providing scaffolding and all other works as well as all incidentals necessary for satisfactory completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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</thead>
<tbody>
<tr>
<td>Polycarbonate sheet</td>
<td>Square meter</td>
</tr>
</tbody>
</table>
SECTION-22
STAIR RAILINGS
## STAIR RAILINGS

### CONTENTS

#### SECTION-22

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Pages</th>
</tr>
</thead>
<tbody>
<tr>
<td>22.1</td>
<td>Wooden handrail for stairs</td>
<td>260-261</td>
</tr>
<tr>
<td>22.1.1</td>
<td>Description</td>
<td>260</td>
</tr>
<tr>
<td>22.1.2</td>
<td>Design drawings</td>
<td>260</td>
</tr>
<tr>
<td>22.1.3</td>
<td>Samples</td>
<td>260</td>
</tr>
<tr>
<td>22.1.4</td>
<td>Other requirements</td>
<td>260</td>
</tr>
<tr>
<td>22.1.5</td>
<td>Materials</td>
<td>260-261</td>
</tr>
<tr>
<td></td>
<td>Timber</td>
<td>260-261</td>
</tr>
<tr>
<td>22.1.6</td>
<td>Fabrication and installation</td>
<td>261</td>
</tr>
<tr>
<td>22.1.7</td>
<td>Measurement</td>
<td>261</td>
</tr>
<tr>
<td>22.1.8</td>
<td>Payment</td>
<td>261</td>
</tr>
<tr>
<td>22.2</td>
<td>Brass pipe for stairs</td>
<td>262-263</td>
</tr>
<tr>
<td>22.2.1</td>
<td>Description</td>
<td>262</td>
</tr>
<tr>
<td>22.2.2</td>
<td>Design drawings</td>
<td>262</td>
</tr>
<tr>
<td>22.2.3</td>
<td>Shop drawings</td>
<td>262</td>
</tr>
<tr>
<td>22.2.4</td>
<td>Materials</td>
<td>262</td>
</tr>
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<td>22.2.5</td>
<td>Samples</td>
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<td>262</td>
</tr>
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<td>Other requirements</td>
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</tr>
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<td>22.2.8</td>
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</tr>
<tr>
<td>22.2.9</td>
<td>Payment</td>
<td>263</td>
</tr>
<tr>
<td>22.3</td>
<td>Stainless steel pipe for stairs</td>
<td>264-265</td>
</tr>
<tr>
<td>22.3.1</td>
<td>Description</td>
<td>264</td>
</tr>
<tr>
<td>22.3.2</td>
<td>Design drawings</td>
<td>264</td>
</tr>
<tr>
<td>22.3.3</td>
<td>Shop drawings</td>
<td>264</td>
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<tr>
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</tr>
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</tr>
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<td>264-265</td>
</tr>
<tr>
<td>22.3.9</td>
<td>Payment</td>
<td>265</td>
</tr>
<tr>
<td>22.4</td>
<td>Brass nosing for stairs steps</td>
<td>266</td>
</tr>
<tr>
<td>22.4.1</td>
<td>Description</td>
<td>266</td>
</tr>
<tr>
<td>22.4.2</td>
<td>Construction method</td>
<td>266</td>
</tr>
<tr>
<td>22.4.3</td>
<td>Measurement</td>
<td>266</td>
</tr>
<tr>
<td>22.4.4</td>
<td>Payment</td>
<td>266</td>
</tr>
</tbody>
</table>
SECTION-22
STAIR RAILINGS

22.1 Wooden handrail for stairs

22.1.1 Description

Works covered under this item shall consist of furnishing, finishing and installing of wooden handrails fixed on balustrade of brass square bar for stair landing in accordance with the applicable Drawings, design and these specifications or as directed by the Engineer.

22.1.2 Design drawings

Design Drawings shall be thoroughly studied by the Contractor before any work is commenced. If any detail description or specification is found missing or in the opinion of the Contractor inadequate, inconsistent or otherwise, the Contractor shall draw the same to the attention of the Engineer who may make necessary arrangements as deemed fit. On no account, the Contractor shall use his own judgement when any discrepancy is noticed in the design, Drawing, details and description.

22.1.3 Samples

Within 35 days following the Contract is awarded, the Contractor shall furnish, for approval of the Engineer, two pieces of samples of wood specimens 150mm x 250mm in area of 25mm thick for the type of wood to be used in the work for approval of the Engineer.

All approved samples may be kept at Site for comparing the materials supplied by the Contractor.

No work for this item shall start until the samples get approval of the Engineer.

22.1.4 Other requirements

Dimensions shown on the Drawings are finished dimensions. In sizing rough components, necessary allowance, therefore, must be kept for the working loss arising from planing, smoothening and finishing.

Polishing or painting, as the case may be, shall be done at Site on receiving approval of the woodwork, carpentry etc. by the Engineer. The working area shall be cleaned properly before starting finishing work and subsequently before each day’s work to ensure reasonably dust-free surroundings.

In case of inflicting injury to any part of the building/other works while installing, the Contractor shall rectify the same employing appropriate skilled workers of the trade and materials required at his own expense.

The Contractor shall keep the Employer indemnified against all charges, which may arise out of this Contract in case of procurement of timbers from local sources.

22.1.5 Materials

Timber

Type of timber, only as specified on the Drawings or in the Bill of Quantities, shall be used. Timber used for woodwork shall be well seasoned, kiln dry containing not more than 8% to 12% moisture so as to ensure minimum tendency towards warping, shrinking and swellings. It shall be free from all defects such as large or loose knots, saps, shakes, upsets, wane edge and twisted fibre. It shall also be free
from all diseases such as decay, wet rot, dry rot, woodworms and white ant. Timber shall be finished to the exact dimensions shown on the Drawings or as directed by the Engineer. The pieces of wood shall be properly finished by planer and other tools and the completed wood works shall be accepted by the Engineer before fixing those in position.

Unless otherwise specified timber used for handrail shall be Chittagong Teak meeting the requirements stated under the relevant portion of the Sub-section on ‘Wood Work for Door/ Window Frames and Shutters’ of this Specification. The size should be in accordance with the Drawings and the Bill of Quantities or as directed by the Engineer.

22.1.6 Fabrication and installation

Railing shall be fabricated of materials indicated on the Drawings or in the Bill of Quantities and fixed to accurate form and position. The Contractor shall perform this job in true perfection.

All woodwork in handrail shall be carried out, finished and varnished and waxed in accordance with the provisions of the relevant Sub-sections of this Specification.

22.1.7 Measurement

All wooden handrail finished, varnished, completed and installed in place, shall be measured in cubic meter for the specified section of the installed handrail and accepted by the Engineer.

22.1.8 Payment

For all wooden handrail, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per cubic meter, which payment shall constitute the full compensation for furnishing all materials, fitting and fixing the handrail with varnishing, waxing and finishing it, including all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>Wooden handrail</td>
<td>Cubic meter</td>
</tr>
</tbody>
</table>
22.2 Brass pipe for stairs

22.2.1 Description
Works covered under this item shall consist of furnishing, fitting, fixing brass pipe of required diameter for stair and any other location in accordance with the provisions of the BOQ, the applicable design Drawings and these specifications and/or as directed by the Engineer.

22.2.2 Design drawings
Design Drawings shall be thoroughly studied by the Contractor before the Work is commenced. If any detail description or specification is found missing or in the opinion of the Contractor the descriptions are inadequate, inconsistent or otherwise, the Contractor shall draw the same to the attention of the Engineer who may make necessary arrangements as deemed fit. On no account, the Contractor shall apply his own judgement when any discrepancy is noticed in the design Drawing, details and description.

22.2.3 Shop drawings
The Contractor shall prepare detail design of the works involved in line with the Employer’s design and prepare Shop Drawing for the Work and submit to the Engineer for approval before starting the work. No work shall start before obtaining such approval.

22.2.4 Materials
The materials shall consist of brass pipe of required diameter and thickness as shown on the Drawings and/or specified in the BOQ and/or as directed by the Engineer.

22.2.5 Samples
The Contractor shall submit three sets of samples of all types of materials and products to the Engineer for his approval before procuring the materials. One set will be kept at the Office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

The works of this Sub-section shall not commence until the samples get approval of the Engineer.

22.2.6 Fabrication and installation
The brass pipe shall be accurately bent, cut and welded as required and fixed with the vertical post or wall by means of brass tray, anchor bolt or welding. The weld must be smoothen by using grinder.

The Contractor shall carryout this work in true perfection in accordance with the Shop Drawings approved by the Engineer earlier, the Design Drawings and the Bill of Quantities.

22.2.7 Other requirements
In case of inflicting damages to any part of the building/other works while fitting and fixing, the Contractor shall rectify the same employing workers of appropriate skill of the trade and materials required at his own expenses.

22.2.8 Measurement
All brass pipe completed and installed in place, shall be measured in linear meter for the specified section and actually installed in accordance with the provisions of the BOQ and/or the design Drawings and/or as directed by the Engineer. Only the works completed and accepted by the Engineer will be eligible for payment.
### 22.2.9 Payment

For all brass pipe, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per linear meter, which payment shall constitute the full compensation for furnishing all materials, fitting and fixing the pipe, all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
<thead>
<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Brass pipe</td>
<td>Linear meter</td>
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</tbody>
</table>
22.3 Stainless steel pipe for stairs

22.3.1 Description

Works covered under this item shall consist of furnishing, fitting, fixing stainless steel pipe of required diameter for stair and any other location in accordance with the provisions of the BOQ, the applicable design Drawings and these specifications and/or as directed by the Engineer.

22.3.2 Design drawings

Design Drawings shall be thoroughly studied by the Contractor before the Work is commenced. If any detail description or specification is found missing or in the opinion of the Contractor the descriptions are inadequate, inconsistent or otherwise, the Contractor shall draw the same to the attention of the Engineer who may make necessary arrangements as deemed fit. On no account, the Contractor shall apply his own judgement when any discrepancy is noticed in the design Drawing, details and description.

22.3.3 Shop drawings

The Contractor shall prepare detail design of the works involved in line with the Employer’s design and prepare Shop Drawing for the Work and submit to the Engineer for approval before starting the work. No work shall start before obtaining such approval.

22.3.4 Materials

The materials shall consist of brass pipe of required diameter and thickness as shown on the Drawings or specified in the BOQ or as directed by the Engineer.

22.3.5 Samples

The Contractor shall submit three sets of samples of all types of materials and products to the Engineer for his approval before procuring the materials. One set will be kept at the Office of the Engineer, one set at the Site office and the remaining set will be returned to the Contractor.

The works of this Sub-section shall not commence until the samples get approval of the Engineer.

22.3.6 Fabrication and installation

The stainless steel pipe shall be accurately bent, cut and welded as required and fixed with the vertical post or wall by means of stainless steel tray, anchor bolt or welding. The weld must be smoothen by using grinder.

The Contractor shall carryout this work in true perfection in accordance with the Shop Drawings approved by the Engineer earlier, the Design Drawings and the Bill of Quantities.

22.3.7 Other requirements

In case of inflicting damages to any part of the building/other works while fitting and fixing, the Contractor shall rectify the same employing workers of appropriate skill of the trade and materials required at his own expenses.

22.3.8 Measurement

All stainless steel pipe completed and installed in place, shall be measured in linear meter for the specified section and actually installed in accordance with the provisions of the BOQ and/or the design
Drawings and/or as directed by the Engineer. Only the works completed and accepted by the Engineer will be eligible for payment.

### 22.3.9 Payment

For all stainless steel pipe, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per linear meter, which payment shall constitute the full compensation for furnishing all materials, fitting and fixing the pipe, all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section, the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

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<th>Item of payment</th>
<th>Unit</th>
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<tbody>
<tr>
<td>Stainless steel pipe</td>
<td>Linear meter</td>
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</tbody>
</table>
22.4 **Brass nosing for stairs steps**

22.4.1 **Description**

Works covered under this item shall consist of furnishing, fitting, fixing 3mm thick 30mm x 30mm brass plate nosing in stair steps including 87mm long anchor bar of section 3mm x 25mm M.S. plate embedded in the concrete in accordance with the applicable Drawings, design and these specifications or as directed by the Engineer.

22.4.2 **Construction method**

The brass plate shall be accurately cut, formed, fitted and finished true to the form and dimension as indicated on the Drawings. The brass plate shall be fixed to the anchor bar by 3 Nos. of rivet or as shown on the Drawings.

22.4.3 **Measurement**

Measurement for payment shall be made in running meter of finished visible brass nosing fitted to the stair steps, installed and finished as shown on the Drawing and accepted by the Engineer.

22.4.4 **Payment**

For all brass nosing, the amount of complete and accepted work measured as provided above shall be paid at the Contract unit price per running meter which payment shall constitute full compensation for furnishing all materials including anchor bar, equipment and labour in assembling, fixing, fitting as well as all incidentals necessary to complete the work as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

<table>
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<th>Item of payment</th>
<th>Unit</th>
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</thead>
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<td>Running meter</td>
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</tbody>
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SECTION-23
MISCELLANEOUS METAL WORK
## MISCELLANEOUS METAL WORK

### CONTENTS

#### SECTION – 23

<table>
<thead>
<tr>
<th>Section</th>
<th>Description</th>
<th>Page</th>
</tr>
</thead>
<tbody>
<tr>
<td>23.1</td>
<td>Metal works</td>
<td>267-271</td>
</tr>
<tr>
<td>23.1.1</td>
<td>Description</td>
<td>267</td>
</tr>
<tr>
<td>23.1.2</td>
<td>Shop drawings</td>
<td>267</td>
</tr>
<tr>
<td>23.1.3</td>
<td>Materials</td>
<td>267-269</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>267</td>
</tr>
<tr>
<td></td>
<td>Steel materials</td>
<td>267-268</td>
</tr>
<tr>
<td></td>
<td>Brass</td>
<td>268</td>
</tr>
<tr>
<td></td>
<td>Anchors, bolts and fastening</td>
<td>268</td>
</tr>
<tr>
<td></td>
<td>Pipe sleeves</td>
<td>268</td>
</tr>
<tr>
<td></td>
<td>Pipe</td>
<td>268</td>
</tr>
<tr>
<td></td>
<td>Electrodes</td>
<td>268</td>
</tr>
<tr>
<td></td>
<td>Concrete and masonry anchors</td>
<td>269</td>
</tr>
<tr>
<td></td>
<td>Grout</td>
<td>269</td>
</tr>
<tr>
<td>23.1.4</td>
<td>Miscellaneous items</td>
<td>269</td>
</tr>
<tr>
<td>23.1.5</td>
<td>Fabrication</td>
<td>269</td>
</tr>
<tr>
<td>23.1.6</td>
<td>Welded connections</td>
<td>269-270</td>
</tr>
<tr>
<td>23.1.7</td>
<td>Shop coating</td>
<td>270</td>
</tr>
<tr>
<td>23.1.8</td>
<td>Installation</td>
<td>270-271</td>
</tr>
<tr>
<td></td>
<td>General</td>
<td>270</td>
</tr>
<tr>
<td></td>
<td>Anchors, bolts, studs and fasteners</td>
<td>270-271</td>
</tr>
<tr>
<td></td>
<td>Base and bearing plates</td>
<td>271</td>
</tr>
<tr>
<td></td>
<td>Doors, windows and gates</td>
<td>271</td>
</tr>
<tr>
<td>23.1.9</td>
<td>Painting</td>
<td>271</td>
</tr>
<tr>
<td>23.1.10</td>
<td>Measurement</td>
<td>271</td>
</tr>
<tr>
<td>23.2</td>
<td>Barbed wire fencing with angle iron posts</td>
<td>272</td>
</tr>
<tr>
<td>23.2.1</td>
<td>Description</td>
<td>272</td>
</tr>
<tr>
<td>23.2.2</td>
<td>Materials</td>
<td>272</td>
</tr>
<tr>
<td>23.2.3</td>
<td>Construction methods</td>
<td>272</td>
</tr>
<tr>
<td>23.2.4</td>
<td>Measurement</td>
<td>272</td>
</tr>
<tr>
<td>23.2.5</td>
<td>Payment</td>
<td>272</td>
</tr>
<tr>
<td>23.3</td>
<td>Barbed wire fencing with RCC posts</td>
<td>273</td>
</tr>
<tr>
<td>23.3.1</td>
<td>Description</td>
<td>273</td>
</tr>
<tr>
<td>23.3.2</td>
<td>Materials</td>
<td>273</td>
</tr>
<tr>
<td></td>
<td>Barbed wire</td>
<td>273</td>
</tr>
<tr>
<td></td>
<td>RCC posts</td>
<td>273</td>
</tr>
<tr>
<td>23.3.3</td>
<td>Construction methods</td>
<td>273</td>
</tr>
<tr>
<td>23.3.4</td>
<td>Measurement</td>
<td>273</td>
</tr>
<tr>
<td>23.3.5</td>
<td>Payment</td>
<td>273</td>
</tr>
</tbody>
</table>
 SECTION-23  
MISCELLANEOUS METAL WORK

23.1 Metal works

23.1.1 Description

Works to be performed under this Section include, but is not limited to, the furnishing and installing of all structural steel and miscellaneous metal work, anchors, bolts and fastenings, and metal fabrications as shown and noted on the Drawings and as specified herein.

Metal work includes steel doors, steel window and glazing thereof, security grilles, stair railing, collapsible gate. M.S. pipe handrails, rolling shutter, boundary gates and other metal works shown on the Drawings or as directed by the Engineer.

The materials, fabrication, workmanship and installation of the entrance gates shall conform to the requirements of this Section.

23.1.2 Shop drawings

The Contractor shall submit full details, large scale Shop Drawings of all structural steel and metal work, showing sizes, details of fabrication and construction, methods of assembly and installation details to the Engineer for review and approval. The Contractor shall also provide manufacturer’s literature for fabricated items, where necessary.

All shop and field weld shall be indicated by standard welding symbols of the American Welding Society. Drawings shall show the size, length and type of each weld.

23.1.3 Materials

General

All materials shall be of new stock and of the best commercial quality for the indicated purpose. Stock items shall be well manufactured by concerns having specialization in the particular item free from defects impairing strength, durability or appearance and subjected to the approval of the Engineer.

Manufactured steel clips and angles will require to be accepted where such is the obvious intent of the Drawings.

All anchor bolts and other parts required for securing each item of work to the construction shall be new.

The Contractor shall take and verify all measurements as may be necessary or required. He shall be responsible for all field dimensions, all fittings and the proper attachment of all works included herein.

Steel materials

- **Structural Shapes:** All structural and miscellaneous steel members and components shall be standard structural sections, shapes and plates, as indicated on the Drawings, conforming to ASTM A 6, ASTM A 36 (minimum yield point of 2530 kg/cm²) of standard specifications for ‘Structural Steel for Building’ or approved equal.
Structural Tubing: Steel tubes, where indicated, shall be welded steel tubing, conforming to BS 1387:1967 (medium), of size and shape indicated on the Drawings.

Bars: Steel bars shall be made from billet steel and shall conform to ASTM A 616 or approved equal.

Brass
Brass shall be Red Brass, conforming to ASTM Specifications Designation B 36 as amended to date.

Anchors, bolts and fastening
Bolts and studs, nuts and washers shall conform to ASTM A-307, A-449 and A-563 as applicable or approved equal.

Anchors, Bolts and Fastening: Bolts and nuts, other than those with self locking screw thread, shall be coarse-thread series. Bolts and nuts shall be the regular hexagon head types.

Washers: Round washers shall be of American National Standard B 27.2 Type, or approved equal. Cut washers shall be placed under all bolts and nuts, bearing on steel and at other locations indicated on the Drawings. Beveled washers shall be square, smooth and sloped so that contact surfaces of bolt head and nuts are parallel.

Galvanizing: All exposed bolts, studs, nuts and washers shall be hot-dip galvanized in accordance with ASTM A 153.

Pipe sleeves
Pipe sleeves through concrete or masonry walls and footings shall be standard weight, wrought iron, mild steel, or cast iron sleeves with not less than 6mm space all around between the sleeve and pipe or conduit.

Pipe
Pipe for handrails, railings, and pipe guards, as shown on the Drawings, shall be welded steel pipe conforming to ASTM A 53, Type E or S, grade A or approved equal. All pipes shall be galvanized. Pipes for gate frames and other locations, indicated on the Drawings, shall be welded steel pipes, conforming to BS 1387 (medium). Pipe fitting, where indicated or if any required shall conform to ASTM A 420. Standard manufacturer’s weldable steel pipe, conforming generally to the foregoing requirements, will be acceptable. All pipes and pipe fittings shall be galvanized.

Electrodes
All arc-welding electrodes shall conform to AWS standards for steel Arc-welding Electrodes. Electrodes shall be rods or wire of size and classification number as recommended by their manufacturers for the positions and other conditions of actual use.
Concrete and masonry anchors

Where anchors are not included in the concrete or masonry construction, anchors shall be Phillips “Read Head” Concrete or Masonry Anchors manufactured by Phillips Drill Co. “Wej-It” Expansion Products, Inc., or approved equal of the sizes and types indicated on Drawings or as required.

Grout

Master Builders, “Embcco” Pro-mix Grout, Conrad Sovin’s “Metal-mix Grout”, Sonneborn’s “Ferrolith G Redi-mixed Grout” or approved equal, high strength, non-shrink grout shall be used.

23.1.4 Miscellaneous items

Structural steel and miscellaneous metal items and their related components are not necessarily to be described individually. The most important and those requiring details will be described. Structural and miscellaneous items, not mentioned or described, shall be furnished and installed in accordance with the intent of the Drawings and specifications and as required to complete the specific work.

23.1.5 Fabrication

Steel works and firms or shops, skilled and experienced in the fabrication of structural steel and miscellaneous metal work, shall perform metal fabrication.

All foreign materials shall be removed and deformations are rectified prior to sizing for fabrication.

Cutting shall be done in a neat and workmanlike manner without damaging the sections. All cuts shall be made square, precise and true to dimensions shown on the Drawings.

The ends of all members shall be carefully reamed out free from burrs before fabrication and shall be checked carefully against deformations and if required such defects shall be rectified.

Tubular sections shall be bent true to profile without any deformation using bending equipment.

Fabrication work shall be neat, true to plumb, square, true to dimension and profile, accurately fitted with tight joints and intersections.

All exposed welded joints shall be grounded smooth without impairing the strength of the joint. Necessary procedures must be adopted prior to making such joints.

Finished surfaces of exposed members shall be smooth and free from markings, burrs or other defects.

23.1.6 Welded connections

Welders shall be experienced in the type of welding work to be performed. If at any time the welder’s performance quality is found not satisfactory, as determined by the Engineer, the welder shall be replaced.

Welded connections shall be made in accordance with AWS D 1.1.

Welds exposed in the finished work shall be ground and dressed smooth so that the shape and profile of the welded item is preserved. Embedded metal frames for floors and deck openings shall be with continuous welds at corners and ground smooth where exposed to view.
Railing shall be fabricated in straight run sections with top and intermediate rails, posts and flange connections, welded ready for installation in the field. Splices, where required, shall be reinforced with sleeve within the pipe and secured with set screws on the underside of the pipe.

23.1.7 Shop coating

All works shall conform to the details of the Drawings and except for galvanized metal brass or bronze, be furnished at the Site with one prime coat of red lead oxide unless otherwise required by the Engineer. The Engineer shall approve the anti-rust prime coat.

Before applying anti-rust prime coat, all rusts, loose milled scales, dirt, welding flux, spatter and other foreign materials shall be removed with wire brushes or steel scrapers. All grease and oil shall be removed by solvent recommended by paint manufacturer. Surfaces shall be dry when painted.

Dissimilar metals shall be insulated from each other with one heavy coat of asphalt paint on contact surfaces in addition to the prime coat specified above.

Prime coat shall be thoroughly and evenly applied and shall be well worked in to corners and joints taking care to avoid sags and runs. Bolts, which are to remain permanently in the work, shall be dipped in paint to cover the entire bolt.

Prime coat shall be omitted from surfaces to be embedded in concrete or masonry. Prime coat shall also be omitted from surfaces to be welded in the field, except where the primer used can be conclusively shown to have no adverse effect on the weld.

The Contractor shall submit the manufacturer’s specification to the Engineer for his approval prior to applying primer.

23.1.7 Installation

General

Installation of miscellaneous metal works and metal fabrication shall be in accordance with the Drawings and approved Shop Drawings, true and horizontal and perpendicular, as the case may be, plumb, level and square, with angles and edges parallel with related lines of the work.

Shop fabricated items, subjected to damage, shall be braced and carefully handled to prevent distortions or other damage.

Before concrete is placed, items to be installed shall be properly braced to prevent distortion by pressure of concrete.

Field welding, where required, shall conform to the requirements specified for shop fabrication.

Anchors, bolts, studs and fasteners

All anchors, bolts and washers, inserts etc. as required for the installation and completion of the work and other miscellaneous steel or iron fastenings to be installed in forms before concrete placement, or built in to concrete, shall be provided as indicated on Drawings, details, and schedules at the time schedule for the concrete work. Bolts and anchors shall be present by the use of templates or such other methods as may be required to locate the anchors and anchor bolts accurately.

Expansion bolts, where approved, shall be carefully installed in snug-fitting, smoothly drilled holes, all in accordance with the manufacturer’s instructions. Expansion bolts shall be installed so that the load acts
on the bolt in shear and withdrawal. Expansion bolts shall be carefully located in order to eliminate the risk of damage to concrete reinforcement and other embedded items. Expansion bolts shall not be used except where indicated on Drawings or where first approved by the Engineer in writing.

**Base and bearing plates**

Bases and plates and pipe posts, where shown in the Drawings, shall be set accurately using a high strength, non-shrink grouting mortar as herein before specified to obtain uniform bearing. Grouting mortar shall be mixed and installed in accordance with the manufacturer’s specifications and instructions. Surfaces to receive the grout shall be cleaned and moistened thoroughly immediately before the grout is placed. Exposed surfaces of grout shall be water-cured with wet burlap or rags for 7 days.

**Doors, windows and gates**

Doors, window units and entrance gates shall be properly and correctly installed by expert persons with all hardware and accessories furnished and installed as shown on the Drawings and as required for complete and finished installations. Doors, windows and gates shall operate freely, properly and smoothly at completion. Glazing of windows shall be performed in accordance with the requirements and procedures as stated previously in the relevant Sub-section.

23.1.8 **Painting**

After installation of steel works and metal fabrication, abraded areas, field areas, field bolts and welds to be touched up and spot shall be painted with the same type of corrosion inhibitor primers as were used for shop painting. Field welds shall be thoroughly wire brushed or sanded prior to painting touch up.

All metal surfaces shall then be painted with three coats of an approved synthetic exterior enamel paint for metal as manufactured by “Berger” or approved equal.

23.1.9 **Measurement**

The different items on structural and miscellaneous metalwork, as indicated on the Drawings and specified herein, will be measured by the various units listed in the Bill of Quantities.

23.1.10 **Payment**

Structural and miscellaneous metalwork will be paid for at the relevant Contract unit prices quoted in the Bill of Quantities for the installed work accepted by the Engineer measured in place.
23.2 Barbed wire fencing with angle iron posts

23.2.1 Description
Works covered by this item shall consist of supplying and providing barbed wire fencing on the top of boundary wall and where required in accordance with the applicable plans and Drawings.

23.2.2 Materials
The barbed wire shall be GI 12 BWG 2-ply with 4 points barbs.

23.2.3 Construction methods
The barbed wire shall be fitted and fixed @ 150mm centre to centre in both horizontally and vertically supported by iron angle posts of section 38mm x 38mm x 6mm. The angle iron posts shall be embedded in the RCC or in the brick work upto 300mm. The angles shall be fabricated in a way as to provide in length of 600mm in vertical position and 450mm in inclined position or as shown in the Drawing or as required by the Engineer. The angle posts shall be fixed @ 2.4m centre to centre. At the joints the binding and tightening shall be done with 18 BWG wire. No sagging in the barbed wires, after fixing, shall be allowed.

23.2.4 Measurement
All barbed wire fencing completed and installed in place, shall be measured in square meter for the length fenced and accepted by the Engineer.

23.2.5 Payment
For all barbed wire fencing, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per square meter, which payment shall constitute the full compensation for furnishing all materials like barbed wire, angle iron posts, binding wires, fitting and fixing the angle iron posts inside RCC or masonry, making holes in the angle posts, straightening, tightening, binding the joints, all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

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<tr>
<th>Item of payment</th>
<th>Unit</th>
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<tr>
<td>Barbed wire fencing with angle iron posts</td>
<td>Square meter</td>
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</tbody>
</table>
23.3  **Barbed wire fencing with RCC posts**

23.3.1  **Description**

Works covered by this item shall consist of supplying and providing barbed wire fencing supported with RCC posts of required sections in accordance with the applicable plans and Drawings.

23.3.2  **Materials**

**Barbed wire**

The barbed wire shall be GI 12 SWG 2-ply with 4 points barbs.

**RCC posts**

Each RCC posts shall have a section 150mm x 150mm having a total length of 2.13m with a spread footing for an area of 450 square millimeter or as shown on the Drawings or as directed by the Engineer.

The specifications for cement, sand, brick chips, water and reinforcement shall conform to the specifications described under the Section on ‘Construction Materials’ of this Specification.

23.3.3  **Construction methods**

The RCC posts shall be constructed with cement concrete with one part of cement, two parts of sand and 4 parts of brick chips. Reinforcement for the posts shall comprise 4 nos. 10mm diameter main rods and 6mm diameter stirrups @ 150mm centre to centre. Reinforcement for footing shall comprise 5 nos. 10mm diameter rod in both ways. The posts shall bear 8 loops made with 6mm diameter rods embedded in to the posts. The posts shall be plastered with 6mm thick cement plaster with 1 part cement and 4 parts sand up to a length of 500mm.

Each RCC post shall be embedded within the ground up to a depth of 0.76m or as shown on the Drawing or as directed by the Engineer.

The barbed wire shall be fitted and fixed in 8 lines drawn through the loops of the RCC posts horizontally @ at least 112mm centre to centre and two lines diagonally from posts to posts. The barbed wire shall be straighten, tighten and shall be bound with 18 BWG wire with the loops of the posts. No sagging in the barbed wires, after fixing, shall be allowed.

23.3.4  **Measurement**

All barbed wire fencing with RCC posts completed and installed in place, shall be measured in running meter for the length fenced and accepted by the Engineer.

23.3.5  **Payment**

For all barbed wire fencing with RCC posts, the amount of completed and accepted work measured as provided above shall be paid at the Contract unit price per running meter, which payment shall constitute the full compensation for furnishing all materials like barbed wire, RCC posts, binding wires, fixing of the RCC posts making loops in the RCC posts, straightening, tightening, binding the joints, all tools and appliances and labour including storage, transport, providing scaffolding and other works as well as all incidentals necessary for completion of all works as per specifications and requirements described under this Sub-section the Bill of Quantities, as shown on the Drawings and as directed by the Engineer.

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<tr>
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<th>Unit</th>
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<tr>
<td>Barbed wire fencing with RCC posts</td>
<td>Running meter</td>
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